Project No. 85-154 May 1986

Paul C. Rizzo Associates, Inc.

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### Report

## Groundwater Assessment Westinghouse Plant, Area A-9

Beaver, Pennsylvania

Westinghouse Electric Corp. Beaver, Pennsylvania

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REPORT GROUNDWATER ASSESSMENT WESTINGHOUSE PLANT, AREA A-9 BEAVER, PENNSYLVANIA

> May 28, 1986 Project No. 85-154

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#### Description

WESTINGHOUSE BEAVER PLANT LOCATION MAP OF EXISTING AND PROPOSED BORINGS

WESTINGHOUSE BEAVER PLANT SUBSURFACE CROSS SECTIONS



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#### DRAFT REPORT GROUNDWATER ASSESSMENT WESTINGHOUSE PLANT, AREA A-9 BEAVER, PENNSYLVANIA

#### 1.0 INTRODUCTION

This report summarizes the results of a survey by Rizzo Associates to assess groundwater conditions in the vicinity of Area A-9 at the Westinghouse Plant in Beaver, Pennsylvania. Existing conditions were described in our report dated September 17, 1985. In this previous study, the presence of both high-pH and low-pH fluids in the ground was documented. Based on chemical testing from existing wells and geophysical measurements, it was concluded that high-pH liquid had leaked from the old cyanide waste tank, and probably from the collection sump for the cyanide rinsewater. This leak was suspected to be inactive. The groundwater containing cyanide is referred to as the cyanide or high-pH plume in the text. Low-pH groundwater, suspected to be from an active unidentified leak, was also documented. This acidic groundwater is referred to as the acid or low-pH plume.

The subsequent investigations, documented in this report, essentially confirm and refine previous observations. The scope of work for this study was consistent with the October 15, 1985 Work Plan and included drilling, sampling, and chemical testing of soil samples and groundwater from four borings located on Figure 1: one deep boring to bedrock (B-5) and three shallow borings (B-6, B-7, and B-8). Monitoring wells were installed in all of these borings to allow for the sampling of groundwater and to monitor the water levels during a pumping test performed as part of the study.

Drilling operations for the shallow borings were conducted from October 21-24, 1985 by Lambert, Inc., of Bridgeville, Pennsylvania with a rotary rig. The deep well was drilled October 28-30 by

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James C. Toward of McDonald, Pennsylvania using a cable tool rig. All of the drilling operations were supervised by William C. Smith of Rizzo Associates. The pumping test was conducted from Boring B-4 by Messrs. Mark Zatezalo and William Baughman of Rizzo Associates on November 8, 1985.

The results of these additional studies lead to the following conclusions, discussed in more detail in Section 5.0:

- High-pH groundwater containing cyanide probably originated from leaks in the cyanide waste tank and the cyanide sump. These leaks have been stopped and only residual contamination persists. Ion concentrations are much reduced from the time prior to the installation of a new cyanide sump and removal of the waste tank. Although previously found only in a clayey fill, high-pH groundwater has been encountered in a permeable sand and gravel unit. The lateral extent of contaminant migration in this unit attenuates by an order of magnitude over a distance of about 20 feet. Available evidence indicates the leakage has been contained within a layer of fill found to a depth of between 10 and 20 feet from ground surface.
- Low-pH groundwater persists only in Boring B-4, even though borings have now been completed to surround this location. The results of the pumping test, as well as the water quality testing, indicate that an active leak may exist from an unknown source.
- Groundwater encountered at a depth of approximately 61 feet below ground surface and perched on a shale bedrock found at a depth of 63.5 feet was not found to be contaminated. This indicates that the near-surface leaks have not impacted the main alluvial aquifer.

Subsequent sections of this report provide a description of the investigations performed (Section 2.0); a re-evaluation of the hydrogeological setting (Section 3.0); a summary of geochemical

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conditions (Section 4.0); and an assessment of soil and groundwater impacts (Section 5.0). It is noted that this report is complementary to the September 17 study and that emphasis has been made to present only the newly acquired data. Older data and background information have been repeated only to the degree necessary to facilitate an understanding of the current results.



#### 2.0 DESCRIPTION OF INVESTIGATIONS PERFORMED

The field operations conducted at the Westinghouse Beaver Plant consisted of drilling and soil sampling, geophysical well logging, groundwater sampling, and a pumping test. Soil and groundwater samples were subsequently delivered to the IT Corporation laboratory in Export, Pennsylvania for testing. All of the work was conducted consistent with the Health and Safety, Sampling and Testing, and Decontamination and Waste Disposal procedures presented in the document "Proposed Work Procedures" submitted to Westinghouse on October 15, 1985. These operations are discussed separately.

#### 2.1 HEALTH AND SAFETY

All site operations were conducted following health and safety procedures as defined in the October 15 "Proposed Work Procedures" document. At the commencement of the drilling activities, a health and safety briefing was provided by the Health and Safety Coordinator (HSC), Mr. Kenneth J. Bird. Site personnel were provided liquid-resistant coveralls, chemical-resistant boots or boot covers, gloves, eye protection and hard hats. The protective gear was cleaned with soap and water or properly disposed of before leaving the site. Disposables were double bagged and placed in a sanitary landfill. Disposable floor mats were used with all vehicles on site.

Air was continuously monitored in the work area for hydrogen cyanide gas (HCN). If HCN had been detected in concentrations greater than five parts per million, work would have stopped and the area evacuated. No HCN gas was encountered during the actual subsurface investigation.

The responsible individual for executing the health and safety program during the field investigations was Mr. William C. Smith. Mr. Smith maintained a daily log documenting the names of site personnel involved, protective equipment used, activities, and any unusual occurrences. This log is available to Westinghouse.

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#### 2.2 DRILLING, SOIL SAMPLING, AND WELL INSTALLATIONS

Two different procedures were used to drill the site borings. Boring B-5 was drilled using a cable tool rig, which functions by raising and dropping a hammer device which breaks the ground and bails up the cuttings. As the hammer is lowered, casing is driven downward allowing for a flush contact with the soil/rock formations. Samples representing a composite of what was drilled were retrieved at five-foot intervals and stored in jars after being classified by visual inspection.

Borings B-6, B-7, and B-8 were drilled by a conventional rotary rig. Drilling was accomplished using a hollow-stem auger and sampling performed using a split-spoon sampler at three-foot intervals. Results are provided in the boring logs in Appendix A. Samples were stored in clean jars after being visually classified. The jar samples were then placed on ice for shipment to the testing laboratory. Chain-of-custody forms were initiated for each of the samples as they were obtained.

As wet zones were encountered at each drill site, well screens were installed in all of the borings in order to attempt to determine the water table under open-hole conditions. In the case of Boring B-5, water was encountered at the top of bedrock; consequently, bentonite was placed from the bottom of the boring to the top of bedrock then 4-inch I.D. threaded, flush-joint mechanically slotted PVC pipe was placed in the bottom nine feet of the well above the bentonite.

In Borings B-6, B-7, and B-8, wet zones were identified from the soil samples. Bentonite pellets were placed in the hole up to the base of the zone desired to be monitored and a 2-inch I.D. threaded flush-joint PVC pipe with 10 feet of mechanically slotted well screen at the bottom was lowered to the top of the bentonite seal. Ottawa sand was placed in the annular space to a depth at least one foot above the top of the screened section. A layer of bentonite pellets were placed over the sand. These pellets were then hydrated to allow for expansion. A cement-bentonite grout slurry was then tremied from the top of the

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bentonite seal up to the surface. At the surface, a 4-inch I.D. protective steel casing with a locking cap was grouted in place. The details of the well installation are provided on the boring logs in Appendix A.

#### 2.3 GEOPHYSICAL WELL LOGGING

In order to provide for a means to compare the conditions encountered in the four new borings with those from the existing wells, geophysical well logs were run in all open borings. The survey was conducted by Appalachian Coal Surveys on November 8, 1985. Three types of logs were run:

- Natural Gamma Increased natural gamma response usually reflects an increase in clay content of the soil penetrated.
- Neutron This log responds to moisture content, where the lower the response, the higher the moisture content; i.e., completely saturated material would have a response of zero, whereas completely dry material would have a response of 100.
- Density This log responds to density variations.

Copies of the logs are included in Appendix B.

#### 2.4 GROUNDWATER SAMPLING AND TESTING

Groundwater was sampled from all wells where a sufficient quantity of groundwater was present. Sampling took place in November and December. Prior to the water sampling, the wells were purged by evacuating a minimum of three times the volume of fluid in the well or until wells were evacuated to dryness. Sample preservation was accomplished as documented in Table 1 to enable proper analysis for cyanide, copper, silver, sulfate, nitrate, ammonia, chloride, and pH. Chain-of-custody records were completed for each sample and are maintained in Rizzo Associates' project files.

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Testing of all water and soil samples was conducted by the laboratory of IT Corporation in Export, Pennsylvania. The procedures followed for analytical testing are defined in Table 2.

#### 2.5 DECONTAMINATION AND WASTE DISPOSAL

Decontamination procedures were required to clean the sampling equipment at the beginning of the project and between borings to avoid cross contamination. This was accomplished with a steam cleaner. Water from this operation was contained on a sheet of plastic and then pumped into a drum for subsequent disposal at the on-site wastewater treatment plant. Cuttings from the drilling operations were also drummed for subsequent testing and appropriate disposal by Westinghouse site personnel. Personnel decontamination was accomplished consistent with the health and safety criteria discussed in Section 2.1.

#### 2.6 PUMPING TESTING

To determine the hydrogeologic characteristics of the shallow saturated zone, a pumping test was performed using Well B-4 as the pumping well. During the test, Well B-4 was pumped in seven five-gallon increments and one 15-gallon increment. During pumping, water levels were recorded in Wells B-6 and B-1B. After each pumping increment, the recovery of water level with time was monitored in Well B-4 to aid in determining hydraulic characteristics of the saturated zone. Actual data collected in the field are summarized in Appendix C. The results of the pumping test are summarized in Section 3.3.

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#### 3.0 HYDROGEOLOGIC CONDITIONS

The drilling of the four new borings, as well as information and drawings provided by Westinghouse from previous geotechnical investigations at the plant, have allowed for a more refined interpretation of subsurface conditions than presented in the September 17 report. The revised descriptions of the overall hydrogeological setting and the hydrogeologic conditions specific to Area A-9 of the plant are discussed separately.

#### 3.1 HYDROGEOLOGICAL SETTING

The Westinghouse plant in Beaver, Pennsylvania is founded on a large alluvial terrace overlooking the Ohio River. As the glacial ice sheets terminated their southern movement about 10 miles north of the site, glacial till is not a constituent of the terrace and all of the sediments are alluvial in nature. Borings to bedrock in different parts of the plant have encountered either sandstone or shale of the Pennsylvanian age Allegheny Group. Bedrock is frequently within 20 feet of the surface along the northwestern side of the plant. Sporadic borings in other parts of the plant indicate the bedrock surface is irregular, but generally deepens toward the Ohio River. The depth to bedrock in Area A-9 based on the results of Boring B-5 (Figure 1) is 63.5 feet.

Regional information on terraces along the Ohio River indicates that the terrace deposits consist of sand and gravel capped with about a 50-foot thickness comprised predominantly of silt. Silt and clay to a thickness of 10-15 feet appear to be present irregularly across much of the plant. Results of the subsurface investigation indicate that in Area A-9 this material is fill and it appears likely that much of the plant may be constructed on similar fill. Although some thin silt and/or clay layers may be present, undisturbed alluvial terrace deposits under most of the plant appear to be mainly sand or sand and gravel.

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The water table across the plant, as well as in Area A-9, is erratic. This appears to be because the alluvium under the site is generally unsaturated, except in a few areas where conditions have allowed groundwater to be locally perched. The main alluvial aquifer does not appear to be under the plant, but is reportedly present farther towards the Ohio River, where the alluvium may be in excess of 150 feet thick. Boring B-5, which encountered water at a depth of approximately 61 feet, appears to have intersected just the fringe of the main aquifer.

#### 3.2 HYDROGEOLOGIC CONDITIONS IN AREA A-9

The four borings added as part of this study allow for a more refined interpretation of site conditions from that presented in the September 17 report. Based on the visual classification of soil samples taken during this study, the presence of rock fragments, cinders, brick fragments, etc., indicate that much of the surficial material is fill. With this in mind, the Michael Baker boring logs drilled in 1983 were re-interpreted. Revised cross sections are presented on Figure 2. The clayey silt/silty clay material previously identified in the September 17 report is fill. Sand and gravel beneath this material is also probably fill. A layer of fine to medium sand found in all of the borings appears to represent the top of natural ground, although some of the sand and gravel may also be natural. Sand, with occasional layers containing gravel, is present to the top of a shale bedrock.

The presence of between 10 and 20 feet of fill in Area A-9 helps explain the highly variable soil conditions and complex groundwater flow regime encountered. The most recently obtained water levels (12/85) are plotted on Figure 2. Most of the groundwater encountered appears to be present within the sand and gravel fill or sand and gravel that is interpreted as fill. Although the situation is unusual, the underlying fine to medium sand appears to be able to perch, at least somewhat, the water encountered in the sand and gravel. It is possible that a very

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thin layer of low permeability separates these two zones, although such a layer was not disclosed during this investitation. In Boring B-7, wet sand and gravel was observed to directly overlie essentially dry fine to medium sand.

Boring B-4 continues to represent the most anomalous groundwater conditions. This boring is now essentially surrounded by dry wells, except for Boring B-2B, which does not appear to have a hydraulic connection with B-4 based on the stratigraphy, as well as the results of the pump test presented in Section 3.3 and the geochemical differences between the two borings discussed in Section 4.0. This is evidence that fluid from an unknown source may be infiltrating the ground directly in the vicinity of B-4.

#### 3.3 PUMPING TEST RESULTS

Results of the pumping test indicate that sustained yield from B-4 is less than one gallon per minute. The yield during the extended pumping test was 15 gallons per hour (0.25 gallons per minute). The maximum drawdown recorded during the recovery portion of tests was 5.3 feet measured one minute after pumping ceased. It is likely, based on pumping rates noted during the test, that actual drawdown was as high as 7 feet.

Recovery times required for 90 percent recovery during the five-gallon increment tests ranged from eight minutes to 30 minutes while 90 percent recovery for the 15 gallon test was 39 minutes. The 90 percent recovery level was used as a cutoff point.

During the pumping of Well No. B-4 drawdown was detected in only one other well, that being Well No. B-6. Well No. B-1B, which was the closest well to B-4 that was not dry, showed no response to the pumping program indicating that the sensing zone in this well is not directly connected hydraulically to Well No. B-4. The same can be said for Well B-1A. There was a rise in water level in Well B-1 suggesting some other

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phenomenon unrelated to pumping is affecting this well. Well B-1 was damaged prior to this study, which may have been responsible for the anomalous readings in this well. Other wells in the vicinity of the pump test were either dry (B-2B and B-8), too deep (B-5), or exhibited water levels so low (B-7) that they showed no effect of pumping.

As part of the pump test procedures, pH and specific conductance were taken during the first five pumping increments. The results are shown in Appendix C.

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#### 4.0 GEOCHEMICAL CONDITIONS

#### 4.1 RESULTS OF CHEMICAL ANALYSES OF SOIL SAMPLES

Soil samples were obtained at three-foot intervals in Borings B-6, B-7, and B-8 and representative samples were analyzed for the ions listed in Table 1. Test results are provided in Table 3 and shown schematically on Figure 2. Soil samples were not tested from Boring B-5. Samples would have been analyzed from this boring had any anomalous pH readings been obtained, but litmus paper testing indicated a pH of 7 for all samples.

Borings B-6 and B-7 both indicate elevated concentrations of cyanide. This cyanide in a basic solution appears to be confined strictly to the sand and gravel layer that may be fill. The silty clay/clayey silt fill above this horizon is somewhat acidic. The fine to medium sand, which is interpreted to be a natural soil, exhibits essentially a neutral pH, with only sulfate found in an anomalous concentration.

Boring B-8 exhibits soil which is moderately acidic with anomalous nitrate and sulfate concentrations in samples taken from depths between 5 and 20-25 feet. Below this depth, the soil is essentially neutral with no strongly anomalous ion concentrations. This is the only boring, including those previously drilled, that exhibits any anomalous ion concentrations in what is interpreted to be natural soil. Levels of contaminants in this boring decrease rapidly with depth.

#### 4.2 RESULTS OF CHEMICAL ANALYSES OF WATER SAMPLES

The results of the chemical analyses of all groundwater samples taken to date at the site are provided in Table 4. Several observations can be made from the data set as a whole:

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- Water from the deep well (B-5) is essentially clean. None of the chemical parameters tested exceed federal water quality criteria for drinking water.
- Groundwater from Borings B-6 and B-7 exhibit cyanide contamination. This is consistent with the soil analyses.
- The water quality in Wells B-1A and B-1B, which historically have had the highest cyanide concentrations, is improving. In B-1A, cyanide concentrations have been reduced from 1,050 ppm to 38 ppm since August 1984. In B-1B, cyanide has been reduced from 2,300 ppm to 1,400 ppm since May 1985.
- The water quality in B-4 has remained more or less stationary since May 1984. Sulfate concentrations have fallen, but nitrate and cyanide have increased with copper and silver remaining approximately the same.
- Boring B-1, previously considered "clean," exhibits groundwater contamination. At the present time, ion concentrations, with the exception of silver, are more concentrated in B-1 than in B-1A. Although most of the parameters were not tested for previously, the slight increase in pH since October 1983 indicates that water quality may have deteriorated since the boring was initially drilled and sampled. It is noted, however, that the concentrations reported are suspect, as the well was physically damaged prior to this investigation.

Water samples were not taken from Borings B-2 and B-3, which no longer exist, and from B-2B and B-8, which were dry.

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#### 5.0 ASSESSMENT OF IMPACTS

Several important observations can be made from the newly acquired data:

- Evidence from this investigation indicates that the high-pH groundwater containing cyanide is contained within the surficial fill and is residual.
- Low-pH groundwater in B-4 may be due to an active leak of unknown origin.
- No impact has been made to the deep aquifer directly beneath the site.

The following paragraphs discuss these observations in greater detail.

#### 5.1 HIGH-pH PLUME

High-pH groundwater containing cyanide is encountered within two different soil environments; fill comprised mainly of silt and clay and the sand and gravel that is now interpreted as fill. Prior to the drilling of Borings B-6 and B-7, the only soil known to contain anomalously high cyanide was the silty clay/clayey silt fill (Figure 2). Within this material, the extent of cyanide movement is well defined. Cyanide contamination in Borings B-6 and B-7 exists within a permeable sand and gravel unit that is now interpreted as fill. Concentrations were found to mitigate rapidly.

The silty clay/clayey silt fill appears to have received cyanide from the cyanide waste tank and the leak from the old cyanide sump. Cyanide values in this clayey soil are highest in B-1A and B-1B. Tests from this same fill material taken from underneath the former waste storage tank locations show a rapid attenuation to essentially a background level of cyanide within about 20 feet of the cyanide waste tank. The soil samples from the silty clay/clayey silt fill taken from B-4 exhibit similar evidence for this rapid attenuation, as cyanide concentrations

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measured from B-4 are nearly identical to those measured from beneath the acid waste tank. Borings B-6 and B-7 also indicate that cyanide values within 20 feet of the cyanide waste tank also attenuate rapidly in this clayey fill.

The influence of the leak from the cyanide sump and/or the old pipeline from the cyanide sump is not as clear as that from the cyanide waste tank. The cyanide concentrations in the silty clay/clayey silt fill penetrated by B-lA are the highest recorded, but this may be a highly localized phenomenon. In B-8, a distance of approximately 20 feet from B-lA, significant amounts of cyanide were not detected.

The cyanide encountered in Borings B-6 and B-7 is present within a sand and gravel unit that is interpreted as fill. This is a permeable unit and the cyanide has been transported a greater distance than in the silty clay/clayey silt fill. The groundwater samples indicate that there is substantial attentuation of cyanide concentration from the values encountered in B-1B.

The high-pH plume containing cyanide is not due to an active leak. As discussed in Section 4.2, the concentration of cyanide at all locations where several measurements have been made is decreasing. The available evidence indicates that conditions began to improve when the cyanide waste tank was decommissioned and the cyanide sump repaired. Considering that the level of decrease in cyanide concentrations in Well B-lA has been approximately two orders of magnitude, we conclude that no significant amounts of additional high-pH fluid containing cyanide are entering the ground in this area.

#### 5.2 LOW-pH PLUME

The acidic groundwater conditions encountered in Boring B-4 are unique to Area A-9. With the drilling of B-8, Boring B-4 is essentially surrounded by borings which exhibit very different hydrogeologic and

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geochemical characteristics. The intended purpose of Boring B-8 was to intersect the acid plume at a separate location so that B-4 could be monitored while a pumping test was conducted out of B-8. B-8 proved to be dry. Although B-8 was drilled to a depth of 40 feet, only mildly acidic soil was encountered to a depth of 20-25 feet with no groundwater. The pumping test conducted from B-4 indicated that the only boring with a slight hydraulic connection to B-4 is B-6, and this boring has a substantially different geochemistry. However, the pumping test indicated that the acidic water in B-4 will recharge.

The fact that the acidic water in B-4 will recharge after being pumped indicates that an active leak may be present. This observation is supported by the water quality test data, which show that the groundwater in B-4 has fairly constant values of ion concentrations.

The source of this leak is unknown. The extremely restricted area where acidic groundwater is found suggests that the source is a break in a pipe or some other conduit from the plant, such as an old trench. However, it is believed by plant personnel that all such sources have either been plugged, removed, or replaced. Given that the acid waste tank has been removed, and the tests of soil from underneath this tank did not indicate that it had leaked, the evidence points to the plant as the source, but the transport mechanism to B-4 is unknown.

#### 5.3 DEEP AQUIFER

The goal of drilling Boring B-5 was to investigate the main alluvial aquifer beneath the site and to determine if any of the chemical leaks had entered the groundwater. As it was anticipated that groundwater at the site would have a general flow toward the Ohio River, B-5 was located so as to be downgradient of surficial contamination.

Bedrock was expected to be over 90 feet deep based on information from nearby water wells and borings from the railroad bridge in Beaver. However, bedrock was shallower than anticipated, having been encountered

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at a depth of 63.5 feet. Water was located at a depth of about 61 feet and it appears reasonable to conclude that this aquifer represents the upgradient fringe of the main alluvial aquifer used as a water supply in Beaver.

No evidence of soil or water contamination was encountered in B-5. pH measurements were taken from composite soil samples at a five-foot spacing and neutral conditions were encountered. The groundwater from this well showed no indication of contamination and none of the parameters analyzed exceeded federal drinking water standards.

Very truly yours,

William J. Johnson Project Manager

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TABLES

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#### TABLE 1

## SAMPLE CONTAINERS AND PRESERVATION FOR WATER SAMPLES<sup>1</sup>

Parameter	Container <sup>2</sup>	Preservation
Cyanide	P, G	4°C, NaOH to pH12
Copper	P, G	HNO3 to pH<2
Silver	P, G	HNO3 to pH<2
Sulfate	P, G	4°C
Nitrate	P, G	4°C
Hq	Determined in Field and Laboratory	
Ammonia	P, G	$4^{\circ}$ C, $H_2$ SO $_4$ to pH<2
Chloride	P, G	None Required

1. Soil samples will be collected in glass containers with teflon-lined lids and stored at 4°C.

2. P = Plastic G = Glass

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#### TABLE 2

	SAMPLE T	YPE
PARAMETERS	SOIL	WATER <sup>(2)</sup>
Cyanide	9010(1)	335.5
Copper	$1310^{(1)}, 220.1^{(2)}$	220.1 <sup>(3)</sup>
Silver	1310 <sup>(1)</sup> , 7760 <sup>(1)</sup>	272.1, 272.2 <sup>(3)</sup>
Sulfate	(A), 375.3 <sup>(2)</sup>	375.3
Nitrate	(A), 352.1 <sup>(2)</sup>	352.1
рн	(B)	150.1
Ammonia	350.2 <sup>(2)</sup> , 350.3 <sup>(2)</sup>	350.2, 350.3
Chloride	(A), 325.3 <sup>(2)</sup>	325.3

#### ANALYTICAL PROCEDURES

- A. Methods of Soil Analysis, Part II, Chemical and Microbiological Properties, C. A. Black, Section 10-2, American Society of Agronomists, Madison, Wisconsin, 1982.
- B. Field and Laboratory Methods Applicable to Overburden and Mine Soil, Zoebek, Schuller, Freeman, Black, West Virginia University, March 1978.

1. Test Methods for Evaluating Solid Waste Physical/Chemical Methods, Second Edition, SW-846, USEPA, 1982.

- 2. Methods for Chemical Analysis of Water and Wastes, USEPA, 1979.
- 3. Includes Acid Digestion for Total Metals.

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TABLE 3

# ANALYTICAL RESULTS OF SOIL SAMPLES TAKEN FROM SITE BORINGS (OCTOBER, 1985)

нд (1:1)	6.80	4.60	4.40	8.70	8.70	00.9/00.6	6.50	6.40	4.95
SILVER (mg/1)	<0.01/<0.01	<0.01	<0.01	0.01	<0.01	0.03	<0.01	<0.01	<0.01
( <u>mg/1</u> )	<0.01	<0.01	<0.01	3.0/3.0	0.81	2.5	<0.01	<0.01	<0.01
SULFATE (mg/kg S04-2)	470	400/380	420	120	. 67	110	110	200	43
NITRATE (mg/kg- NO3-N)	37/37***	11	11	140	74	130	1.1	4.9	16
<u>CYANIDE</u> (mg/kg)	0.71	0.50	<0.50	16	27	06	0.89	<0.50	0.50/0.50
CHLORIDE (mg/kg)	41	36	. 62	120	96	450	18	53	48
AMMONIA (mg/kg NH <sub>3</sub> -N)	7.6	7.4	9.4	56	64	110	6.3	34	7.1/5.7
BOTTOM DEPTH OF SAMPLE BELOW SURFACE (ft)	2.0	5.0	8.0	11.0	14.0	17.0	20.0	3.5	6.5
BORING	و.	9	9	ę	9	و	9	7	٢

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mg/kg = milligrams per kilogram or parts per million. mg/l = milligrams per liter or parts per million. The indicated sample was analyzed in duplicate.

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BORING	٢	٢	٢	L	8	8	œ	∞	8	8	8	8	<sup>∞</sup>
OF SAMPLE BELOW SURFACE (ft)	12.5	15.5	18.5	20.0	3.0	6.0	0.6	12.0	15.0	18.0	21.0	27.0	36.0
AMMONIA (mg/kg nH <sub>3</sub> -n)	15	28	3.7	5.7	130	120	43	24/19	4.0	4.3/2.3	2.3	<0.5	1.6
CHLORIDE (mg/kg)	70	110	51	27	. 67	67	65/67	48	31	39	55	34	22
CYANIDE (mg/kg)	<0.50	84	<0.50	1.2	<0.50	<0.50	2.8	<0.50	<0.50	<0.50/0.67	0.87	<0.50	0.77
NITRATE (mg/kg- NO <sub>3</sub> -N)	44	92	3.7	2.2	2.7/4.0	8.2	14/13	310/310	310/310	190/190	660	15	19
SULFATE (mg/kg SO4-2)	63	140	270	120	880	540/530	400	86	250/220	160	140	52	54
( <u>T/gm</u> )	1.5	1.6	<0.01	<0.01	0.04	0.40/0.40	0.32	0.08	<0.01	<0.01	<0.01	<0.01	<0.01
<u>SILVER</u> (mg/l)	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	0.22/0.22	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01
PH (1:1)	9.35	8.80	6.55	7.10	7.70	4.45	4.10	5.10	5.60	5.50	5.15	7.00	7.50
	$\begin{array}{c cccc} \hline AMMONIA & \hline CHLORIDE & \hline CYANIDE & NITRATE & SULFATE & COPPER & SILVER \\ \hline (mg/kg & (mg/kg) & (mg/kg) & (mg/kg) & (mg/1 &) & (mg/1) \\ \hline NH_3-N) & NO_3^{-N} & SO_4^{-2} \end{pmatrix} & SO_4^{-2} \end{pmatrix}$	$\begin{array}{ccccccc} \text{UF SAMPLE} & \text{DMMONL} & \text{CHLORIDE} & \text{CYANIDE} & \text{NITRATE} & \text{SULFATE} & \text{COPPER} & \text{SILVER} \\ \hline \text{(FL)} & (\text{mg/kg}) & (\text{mg/kg}) & (\text{mg/kg}) & (\text{mg/kg}) & (\text{mg/l}^{-1}) & (\text{mg/l}) \\ & \text{NH}_3-\text{N} & \text{NH}_3-\text{N} & \text{NO}_3^{-1}\text{N} & \text{SO}_4^{-2}\text{N} & \text{SO}_4^{-2}\text{N} \\ \hline \end{array} \\ \hline 12.5 & 15 & 70 & <0.50 & 44 & 63 & 1.5 & <0.01 \\ \hline \end{array}$	$\begin{array}{c cccccc} \overrightarrow{Or} & \overrightarrow{Samtla} & \overrightarrow{OHONLA} & \overrightarrow{CHLORIDE} & \overrightarrow{CYANIDE} & \overrightarrow{NITRATE} & \overrightarrow{SULFATE} & \overrightarrow{COPPER} & \overrightarrow{SILVER} \\ \hline (ft) & (mg/kg) & (mg/kg) & (mg/kg) & (mg/kg) & (mg/l) & (mg/l) \\ \overrightarrow{NH_3-N} & \overrightarrow{NH_3-N} & \overrightarrow{NO_3-N} & \overrightarrow{SO_4^{-2}} & \overrightarrow{SO_4^{-2}} & \overrightarrow{NITRATE} & \overrightarrow{COPPER} & \overrightarrow{SILVER} & \overrightarrow{CMDP} \\ \hline 12.5 & 15 & 70 & <0.50 & 44 & 63 & 1.5 & <0.01 & 9 \\ \hline 15.5 & 28 & 110 & 84 & 92 & 140 & 1.6 & <0.01 & 9 \end{array}$	OF SAMPLE         AMMONIA         CHLORIDE         CYANIDE         NITRATE         SULFATE         COPPER         SILVER           (ft)         (mg/kg)         (mg/kg)         (mg/kg)         (mg/kg)         (mg/l)         (mg/l)           (ft)         (mg/kg)         (mg/kg)         (mg/kg)         (mg/kg)         (mg/l)         (mg/l)           NH3-N)         NH3-N)         N         NO         0.4         63         1.5         (0.01           12.5         28         110         84         92         140         1.6         (0.01           15.5         28         3.7         51         <0.50	DELOW SURFACE         AMMONIA         CHLORIDE         CYANIDE         NITRATE         SULFATE         COPER, Major         SILVER         COPER, Major         SILVER         SILVER         COPER, Major         SILVER         COPER, Major         SILVER         SILVER	DELIOW SURFACE (ft)         AMONIA (mg/kg) (mg/kg)         CHLORIDE (mg/kg) (mg/kg)         CTANTIE (mg/kg) (mg/kg)         NTRATE (mg/kg) (mg/kg)         SULFATE (mg/l)         SULFATE (mg/l)         SILFATE (mg/l)         SILFATE (mg/l) <th< td=""><td>DELION SURFACE (ft)         MNONLA (mg/kg) (mg/kg)         CHLORIDE (mg/kg) (mg/kg)         CYANIDE (mg/kg) (mg/kg)         NUTRATE (mg/kg) (mg/kg)         SULFATE (mg/kg) (mg/lg)         SULFATE (mg/lg) (mg/lg)         SULFATE (mg/lg) (mg/lg)         SULFATE (mg/lg)         SULFATE (mg/lg)&lt;</td><td>OF SAMPLE (f1)         AMONIA (mg/kg)         CHLORIDE (mg/kg)         NITRATE (mg/kg)         NITRATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/lg)         SULFATE (mg</td><td>DORUNG Index (F1)         OF A PARTIAL INDEX (F1)         MMONIA (F1)         CHLORIDE (mg/kg)         VITRATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/Lg)         SUL</td><td>Districts         Out-solution         MUNICATION         MUNICA</td><td>DURING INFORM         DURING (EL)         DURING (EL)</td><td>DUC NOTIFIE OF Solution Solution Solution Solution Solution Solution (ing/kg) (</td><td>DORMS         LULY NOTES         MMONTA         CHIORUS         CANNEL         MMONTA         CHIORUS         CANNEL         MMONTA         CHIORUS         NUTAGE         MMONTA         COPERIA         ELLON SUPPLICA         MMONTA         MMONTA         CHIORUS         MMONTA         MONTA         MMONTA         MMONTA</td></th<>	DELION SURFACE (ft)         MNONLA (mg/kg) (mg/kg)         CHLORIDE (mg/kg) (mg/kg)         CYANIDE (mg/kg) (mg/kg)         NUTRATE (mg/kg) (mg/kg)         SULFATE (mg/kg) (mg/lg)         SULFATE (mg/lg) (mg/lg)         SULFATE (mg/lg) (mg/lg)         SULFATE (mg/lg)         SULFATE (mg/lg)<	OF SAMPLE (f1)         AMONIA (mg/kg)         CHLORIDE (mg/kg)         NITRATE (mg/kg)         NITRATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/lg)         SULFATE (mg	DORUNG Index (F1)         OF A PARTIAL INDEX (F1)         MMONIA (F1)         CHLORIDE (mg/kg)         VITRATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/kg)         SULFATE (mg/Lg)         SUL	Districts         Out-solution         MUNICATION         MUNICA	DURING INFORM         DURING (EL)         DURING (EL)	DUC NOTIFIE OF Solution Solution Solution Solution Solution Solution (ing/kg) (	DORMS         LULY NOTES         MMONTA         CHIORUS         CANNEL         MMONTA         CHIORUS         CANNEL         MMONTA         CHIORUS         NUTAGE         MMONTA         COPERIA         ELLON SUPPLICA         MMONTA         MMONTA         CHIORUS         MMONTA         MONTA         MMONTA         MMONTA

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		(12-85)	Ŕ.		703	062	£03	280	769	0.02		-							7.4(5)						332	120	L 0	ES	• •	
	~	(11-85)	IS(4)		370	180	220	530	290	0.075									8.3		·							5495.		
	وو	(12-85)	41/38		180	16	120	170	120	0.04						-		-	1.6						,		ř.	WPL 005 54		
	9	(11-85)	26		270	120	180	150	150/160	0.027/0.021									1.6								1	EM		
• •	ß	(12-85)	4.5		17	0.03	2.6/2.8	16	0.17	0.02	\$								7.3(5)							·				
	S	(11-85)	0.83		15	<0.02	<b>1.9/1.9</b>	8	0.06/0.06	100°0>//100°0>									7.45											
	4	(12-85)	1.4		330/350	31/25	1800	710/780		0°00		•			· ···				3.7(5)		· · ·							• •		
	4	(11-85)	40/44		380/370	18	1960/1970	820	250	800-0					,	,			3.15											ł
	4	(5–85)	2.0	. <u> </u>	490	7	520	340	021	1.5									2.8											
	4	(5-84)		4		2.3	825	2600	061	0.05					:		0.59		2.7											
SAMPLES	4	(11-83)		650		520	6440	1160	198	0°08		0.38	0.55	m	11.0	3.0	1.42	<0.02	3.0											
	4(3)	(10-83)		160	•	49				2.2	22	0.64	2.6	Ħ	1.1				2.1											
<b>TABLE 4</b> L RESULTS OF GROUNDWATER TAKEN FROM SITE BORINGS	4(2)	(10-83)				1				3.3	64	0.73	61.0	EI	4.2		•		2.6/3.2	e, no tests		· .							•	
TABLE TS OF ( ROM SI'	53	(5-85)	3.5		390	1.5	160	260	3.4	0.39									7.5	n the table										
. RESUL	18	(12-85)	46		2400	1400	870	530		0 <b>.1</b>			<u> </u>						10.5								1			
ANALYTICAL	IB	(11-85)	<b>5</b> 8 .		2700	1100	570	640	0011	0.013/0.004									6.9	Where blanks are present in the table, no tests										1
ANA	E.	(5-85)	. 76		3500	2300	390	345	1600	2.3								-	1 10.3											Ŷ
	P1	(12-85)	18		76	8	52	86	01/11	17/16									11.3(5)	ided in $\mu_{\rm g}$	uupilcate.	¥T.	rv oli.	7				X		
	TA I	(11-85)	41		120	61/58	32	180	94 1	31									10.4	ich is prov	ut pazáreue	Valenc to b	re laborato							
	¥1	(5-85)	140		585	210	<b>Е</b>	650	011	88							· · · ·	_	10.7	ercury, wh	ampte was	to be equi	sociates a							
	(1) JA <sup>(1)</sup>	(8-84)				1050	156	1240	1000	0 8 8	 2		0.40						10.8	cept for m	dea, the s	onsidered	v Rizzo As						. •	
	<b>V</b> I	(6-84)		· · · · · · · · · · · · · · · · · · ·	- <u>.</u>	950	450	016	170	<b>9.</b>							1.0		10.6	n mg/l, ex	are provi	trench, o	es taken b							
	-	(11-85)	49		140	45	180	220	43	0.035									7.95	provided i	Sautev Own	e-A mori na	walysis. from samoly							• .
		(10-83)						r		0.72	0.19	0.06	<0.05		3.8	ę			7.4	ations are	ced. where	stually tak	mple for an • pH values							+ - -
	Well	Date	Amonia (NH <sub>3</sub> )	Residual Chlorine (Cl)	Chloride (Cl)	Cyanide (CN)	Nitrate (NO <sub>3</sub> )	Sulfate (SO <sub>4</sub> )	Copper (Cu)	Silver (Ag)	Zinc (Zn)	Cachnium . (Cd)	Chromiun (Cr)	Mercury (Hg)	Lead (Pb)	Barium (Ba)	Nickel (Ni)	Selenium (Se)	ħ	NOTE: All concentrations are provided in mg/1, except for mercury, which is provided in $\mu$ g/1.	:		<ol> <li>Atter purging.</li> <li>Insufficient sample for analysis.</li> <li>Field ny: other pH values from samples taken by Rizzo Associates are laboratory pH.</li> </ol>							

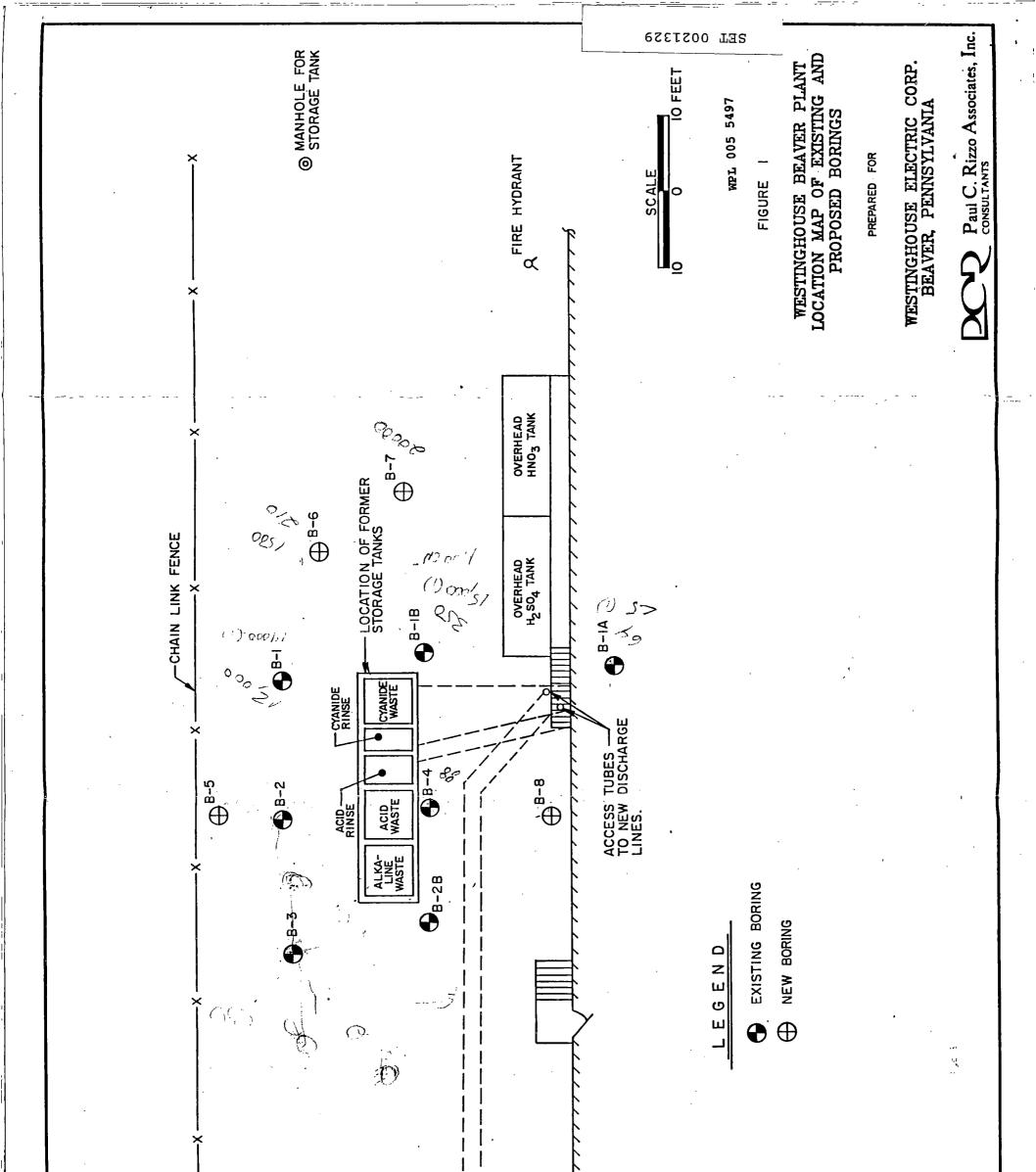
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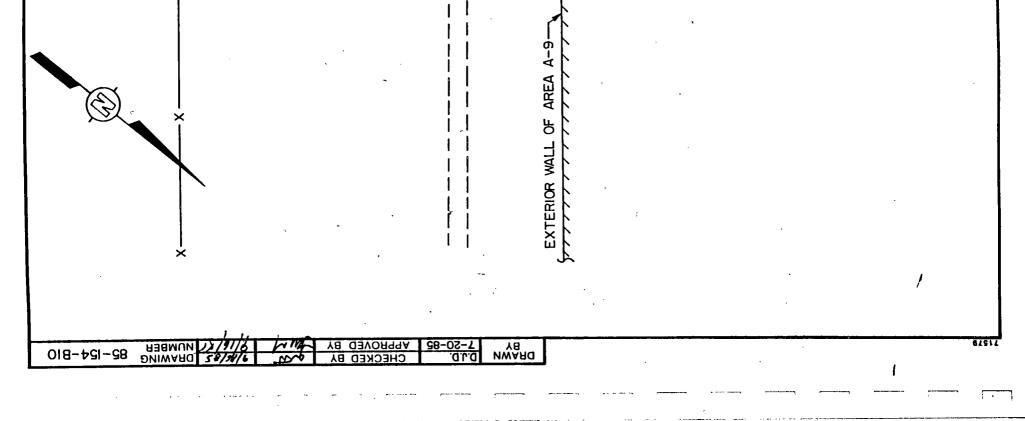


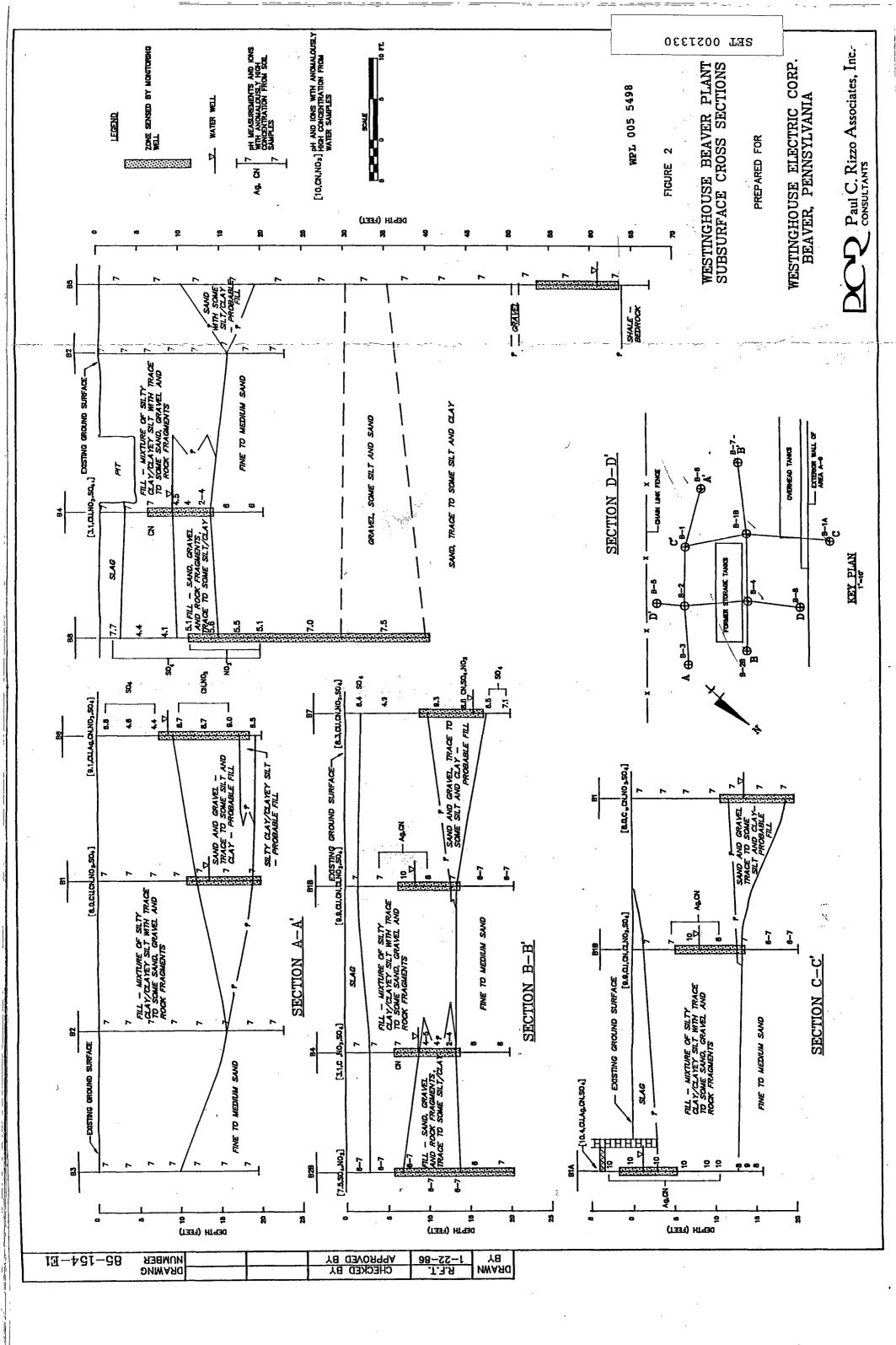
SET 0021328

WPL 005 5496

FIGURES







APPENDIX A BORING LOGS

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WPL 005 5499

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0     BROWN SILTY CLAY TO CLAYEY     STICKUP*       5     SILT     IDD       10     BROWN SILTY CLAY TO CLAYEY       11     IDD       12     BROWN MEDIUM SAND WITH SOME       13     BROWN MEDIUM SAND WITH SOME       15     VO NOW       15     VO NOW       16     BROWN MEDIUM SAND WITH SOME       17     BROWN FINE TO MEDIUM SAND       180     STERE       190     BROWN FINE TO MEDIUM SAND       100     STRACE TO SOME SILT AND CLAY       101     STRACE TO SOME SILT AND CLAY       101     STRACE TO SOME SILT AND CLAY       102     STRACE TO SOME SILT AND CLAY       103     STRACE TO SOME SILT AND CLAY       104     STRACE TO SOME SILT AND CLAY       105     STRACE TO SOME SILT AND CLAY       100     STRACE TO SOME SILT AND CLAY       100     STRACE TO SOME SILT AND CLAY	DATE FI	GAN I NISHED I SURFACE	10/30/8	5	BORING <u>B5</u>		FIELD ENGINE CHECKED BYI	
0     BROWN SILTY CLAY TO CLAYEY       5     SILT       10     Main Silty CLAY TO CLAYEY       11     Main Silty Clay TO CLAYEY       12     Main Silty Clay TO CLAYEY       13     Main Silty Clay TO CLAYEY       14     Main Silty Clay TO CLAYEY       15     Main Silty Clay To Clayer       15     Main Silty Clayer       16     Main Silty Clay	ELEV. (FEET)	DEPTH (FEET)	SAMPLE TYPE	PRDFILE	DESCRIPTION	U.S.C.S.	PIEZOMETER INSTALLATION	PENETRATION RESISTANCE (BLOVS PER FUOT) 10 30 50
101     BROWN MEDIUM SAND WITH SDME     SM-       15     ST     SM-       15     T     ST       15     ST     ST       15     T     ST       19.0'     ST     ST       25     ST     ST       1001     ST     ST       25     ST     ST       101     ST     ST       25     ST     ST       26     ST     ST       27     ST     ST       28     ST     ST       29.5'     ST     ST       30     ST     ST       30     ST     ST       30     ST     ST       30			BAILER	88 88 88	SILT		¢5.5	
BROWN FINE TO MEDIUM SAND 25 30 30 30 30 40 45 45 45 BROWN FINE TO MEDIUM SAND BROWN FINE TO MEDIUM SAND WITH 10 10 10 10 10 10 10 10 10 10			FROM CABLE		SILT AND CLAY		6'1.D, VELDED STEEL VITH PROTECTIVE	
BROWN FINE TO MEDIUM SAND WITH TRACE TO SOME SILT AND CLAY. TRACE TO SOME GRAVEL 30-35'			COMPOSITE		BROWN FINE TO MEDIUM SAND	sp	BOREHOLE	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			يا 20 ا	9 × 0 /	BROWN FINE TO MEDIUM SAND WITH TRACE TO SOME SILT AND CLAY,	gm- gc	5° DIA. BORING	
$-\frac{45}{-}$		 	uctive Dri	LAN.			ي م	
		45 	DESTR			SM		
50 - BURING NO. B5		 				sp~ sm		

e.

SET 0021332

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	DATE BEGAN ! DATE FINISHED !_ GROUND SURFACE	10/25/85	BORING_B5				VJJ
50     BROWN GRAVEL WITH SOME     DP       AND CLAY     SLOP       97     BROWN MEDIUM TO COARSE SAND       55     SOME SILT AND CLAY       55     BROWN WELL GRADED SAND WITH       56     SIME SILT AND CLAY       50     SIME SILT AND CLAY       51     SIME SILT AND CLAY       50     SIME SILT AND CLAY       51     SIME SILT AND CLAY       52     HARD DARK GRAY SHALE, SLIGHTLY       65     SIME       65     SIME       65     SIME       57     SIME       57     SIME       65     SIME       65     SIME       65     SIME       66     SIME       70     K       67.0'       80       90       91       92       93       94       94       95       95       96       96       97       98       98       99			DESCRIPTION	U.S.C.S.	PIEZOMET INSTALLAT	er Ton	PENETRATION RESISTANCE COLOVS PER FOOT) 10 30 50
BORING NO. 85 SHEET 2 OF 2	  	- 5 FODT COMPOSITE SAMPLES FROM CABLE TODL BAILER	SAND AND TRACE SILT AND CLAY 51.0' BROWN MEDIUM TO COARSE SAND SOME SILT AND CLAY 55.0' BROWN WELL GRADED SAND WITH SOME SILT AND CLAY TOP OF ROCK 63.5' HARD DARK GRAY SHALE, SLIGHTLY CLAYEY AT TOP 67.0'	gp- gn- gc sw sn	BUREHDLE (55.0') CULLAPSED MATERIAL (8.3') BENTONITE (4.0') BENTONITE		

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	GAN I			BORINGB6		FIELD ENGINEER CHECKED BY1	
	SURFACE						
ELEV. (FEET)	DEPTH (FEET)	SAMPLE TYPE	PROFILE	DESCRIPTION	U.S.C.S.	PIEZOMETER INSTALLATION	PENETRATION RESISTANCE (BLOWS PER FOOT) 10 30 50
				BROWN-BLACK TOPSOIL 0.3' FILL-BROWN SILTY CLAY TO CLAYEY SILT WITH SOME SAND AND FRAGMENTS OF BRICK, COAL AND WOOD - DRY 3.9' LOOSE TO MEDIUM DENSE LIGHT BROWN SILTY CLAY TO CLAYEY SILT AND TRACE SAND, SOME LAYERS OF SILTY SAND AND SAND AND GRAVEL.	S∩ ™	INSTALLATION STICKUP= 1.65' GROUT SLURRY (6.5') BENTONITE (1.0') BENTONITE (1.0') BENTONITE (1.0') BENTONITE (1.0') BENTONITE (1.0')	PENETRATIDN RESISTANCE (BLOW'S PER FOULT) 10 30 50
						BORING ND. B6	

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SHEET 1 DF 1

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SET 0021034

DATE FI	GAN I	10/22/8	5	BORING B7		FIELD ENGINEER CHECKED BYI	
ELEV.	SURFACE DEPTH (FEET)	SAMPLE TYPE	PRDFILE	DESCRIPTION	U.S.C.S.	PIEZUMETER INSTALLATION	PENETRATION RESISTANCE (BLOWS PER FOOT
	0		$\sim$	ASPHALT 0.3'	╞╴		
	E 1	5/	X	BLACK-GRAY SLAG 1.8'		STICKUP=	
	F -		$\bowtie$			HSTICKUP= STICKUP= J 1.75' J 1.75'	•
	<u> </u>	S	$\bigotimes$	FILL-MIXTURE OF CLAYEY SILT, SAND AND GRAVEL WITH FRAGMENTS			
	┝ -	2	$\bigotimes$	<b>UF BRICK AND WODD.</b> - GENERALLY DRY, MOIST AT BOTTOM	Į		
	}		$\bowtie$				
	- 10 -	2 R	Х	10.0'		BENTONITE	
		s	¥ ٥(	LODSE BROWN TO BROWN-GRAY	ł		
			: 3: a)	SAND AND GRAVEL TRACE TO	gm		
			ал • ¿ о ·	SOME SILT, TRACE TO SOME COBBLES, BLACK STAINS (POSSIBLY			
	- 15 -	~~~	?0	MANGANESE) AT BOTTOM - WET	sp-		
<u>_</u> 12-85	┣	5	ŝ	17.0'	9m	àu :	
12-00	┝ -	S	<u></u>	LOOSE BROWN FINE TO MEDIUM	sp	BENTONITE	
	- 20 -	$\langle $		SAND, TRACE SILT-DRY 19.2'	sp	(2.3')	
		~1~	0.0	MEDIUM DENSE BROWN SAND	<b> </b> <sup>3</sup> <sup>2</sup>		
				20.0'	Į		
	┝ ~			DESTRUCTIVE DRILLING TO 25.0'		(6.0')	
	- 25 -			25.0			
	┠			BOTTOM OF BORING AT 25.0'			
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	<b></b> -					BORING NO. B7	

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GRUIND SUFFACE EL ELEV. DEPTH SAMPLE I DESCRIPTION	DATE BEGAN 110 DATE FINISHED 1		BORING B8		FIELD ENGINEE CHECKED BYI_	
0     ASPHALT     0.27     D     D     D     D       10     SILL-LODSE BROWN TO BLACK FAGMENTS OF BRICK AND VOD AND FEBLES - DRY 2.57     Charles Status (9.07)     STICKUP- (1)     D     D       10     S     Charles Status (9.07)     Status (9.07)     Status (9.07)     Status (9.07)       10     S     Charles Status (9.07)     Status (9.07)     Status (9.07)       11     S     Charles Status (9.07)     Status (9.07)     Status (9.07)       12     S     Charles Status (9.07)     Status (9.07)     Status (9.07)       13     Charles Status (9.07)     Status (9.07)     Status (9.07)       20     Charles Status (9.07)     Status (9.07)     Status (9.07)       21     Charles Status (9.07)     Status (9.07)     Status (9.07)       25     Charles Status (9.07)     Status (9.07)     Status (9.07)       26     Charles Status (9.07)     Status (9.07)     Status (9.07)       26     Charles Status (9.07)<			-			
SLAG AND SILTY CLAY WITH     GROUT       SLAG AND SILTY CLAY WITH     GROUT       SLAG AND SILTY CLAY WITH     GROUT       VERY SILT BROWN TO BROWN     GRAVELLY       VERY SILT SILTY CLAY TO     GRAVELLY       CALL GRAY MOTHED SILTY CLAY TO     GRAVELLY       CALL STATE     S.S.       STATE     S.S.       CALL STATE     S.S.       CALL STATE     S.S.       CALL STATE     S.S.		SAMPLE	DESCRIPTION	U.S.C.S.		PENETRATION RESISTANCE (BLOVS PER FOOT 10 30 50
ANALYSIS SAMPLES BECAUSE OF NO RECOVERY IN S-11, 31-33'.	(FEET) (FEET)		ASPHALT 0.3' FILL-LODSE BROWN TO BLACK SLAG AND SILTY CLAY WITH FRAGMENTS OF BRICK AND WODD AND PEBBLES - DRY 2.5' VERY STIFF BROWN TO BROWN AND GRAY MOTTLED SILTY CLAY TO CLAYEY SILT BECOMES GRAVELLY WITH DEPTH, - DAMP P=3.75 TSF 6.5' MEDIUM STIFF SILTY CLAY TO CLAYEY SILT WITH SAND AND GRAVEL. WITH TRACE TO SOME SILT GENERALLY MOIST BECOMES WET AT 11.8'. 14.5' LODSE BROWN SAND AND GRAVEL WITH HORIZONTAL BLACK STREAKS. VERY SOFT BROWN SILT 20.1'-21.0' AND 22.6'-22.8' - WET GENERALLY DRY TO DAMP, BECOMES WET AT 28.6' 29.1' LODSE BROWN GRAVEL WITH COBBLES WET AT 28.6' 29.1' LODSE BROWN GRAVEL WITH COBBLES AND SAND, SOME SILT. 0.65' LAYER OF IRON STAINED GRAVEL AT TOP 39.5' LODSE BROWN GRAVEL WITH COBBLES AND SAND, SOME SILT. 0.65' LAYER OF IRON STAINED GRAVEL AT TOP 39.5' LODSE BROWN GRAVEL WITH COBBLES AND SAND, SOME GRAVEL. BLACK STAINS,-MOIST TO WET. 40.0' BUTTOM OF BORING AT 40.0' NUTE SAMPLE NOS, AND INTERVALS FROM		STICKUP= 1.6' GRUUT SLURRY (9.0') BENTONITE (1.8') BENTONITE (1.8') BENTONITE (1.8') BENTONITE (1.8') BENTONITE (1.8') BENTONITE (1.8') SAND (28.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT SAND (29.7') GRUUT (29.7') (29.7'	10 30 50

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#### WPL 005 5504

SET 0021336

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#### APPENDIX B

#### GEOPHYSICAL WELL LOGS



WPL 005 5505

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SET 0021337

NOTES

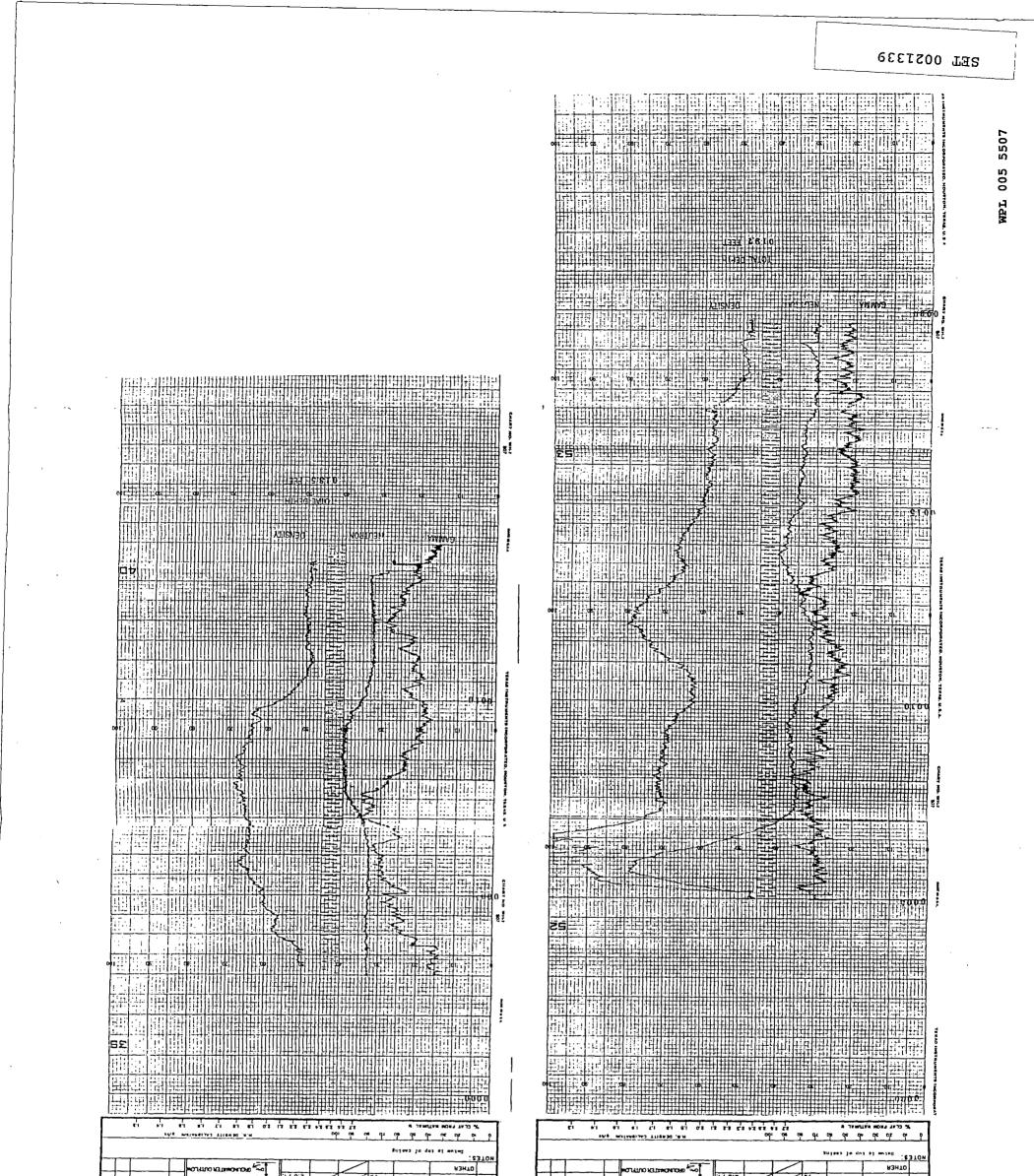
Well 4 was not logged because of an obstruction in the well. Interpretation is provided in terms of rock equivalents of the soils penetrated; i.e, the symbol for sandstone means sand; the symbol for shale/claystone means clay; etc.



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WPL 005 5506

SET 0021338



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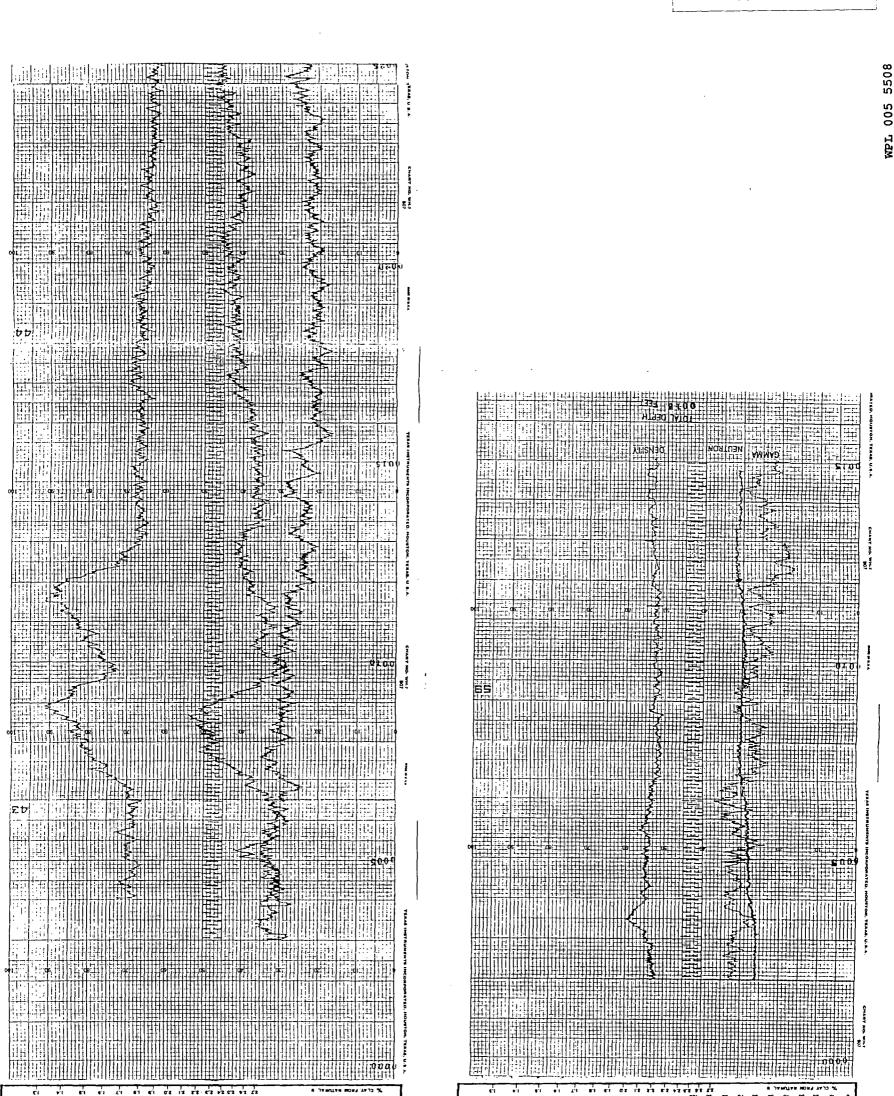
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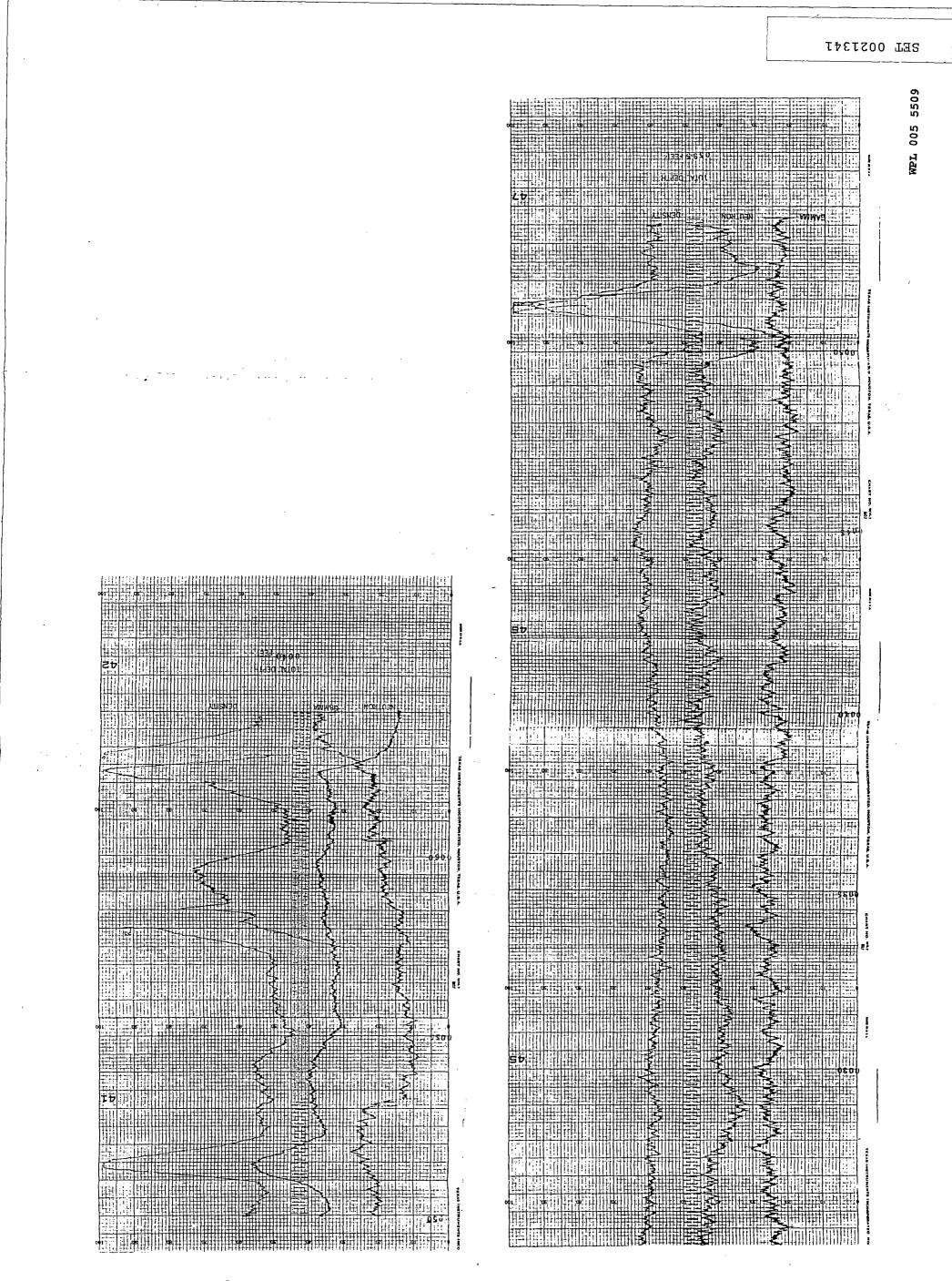
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Well Log for B-5, continued (Repeat of Lower Section of Log)

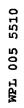
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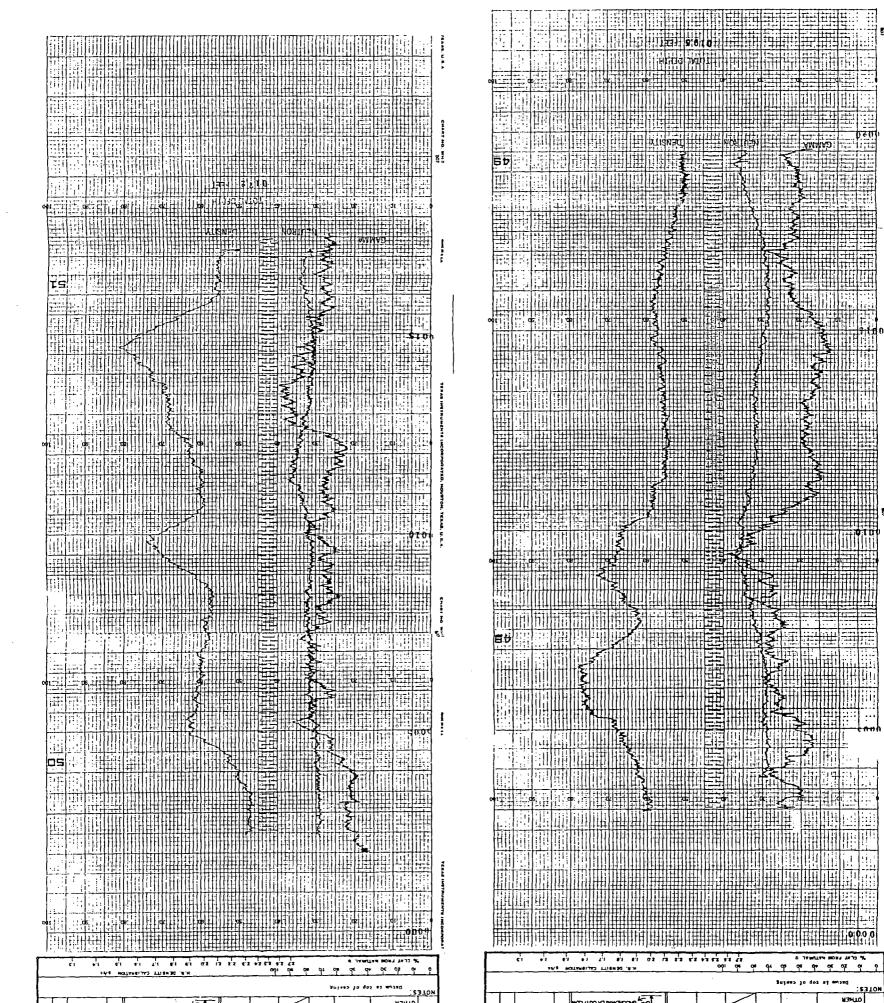
Well Log for B-5, continued

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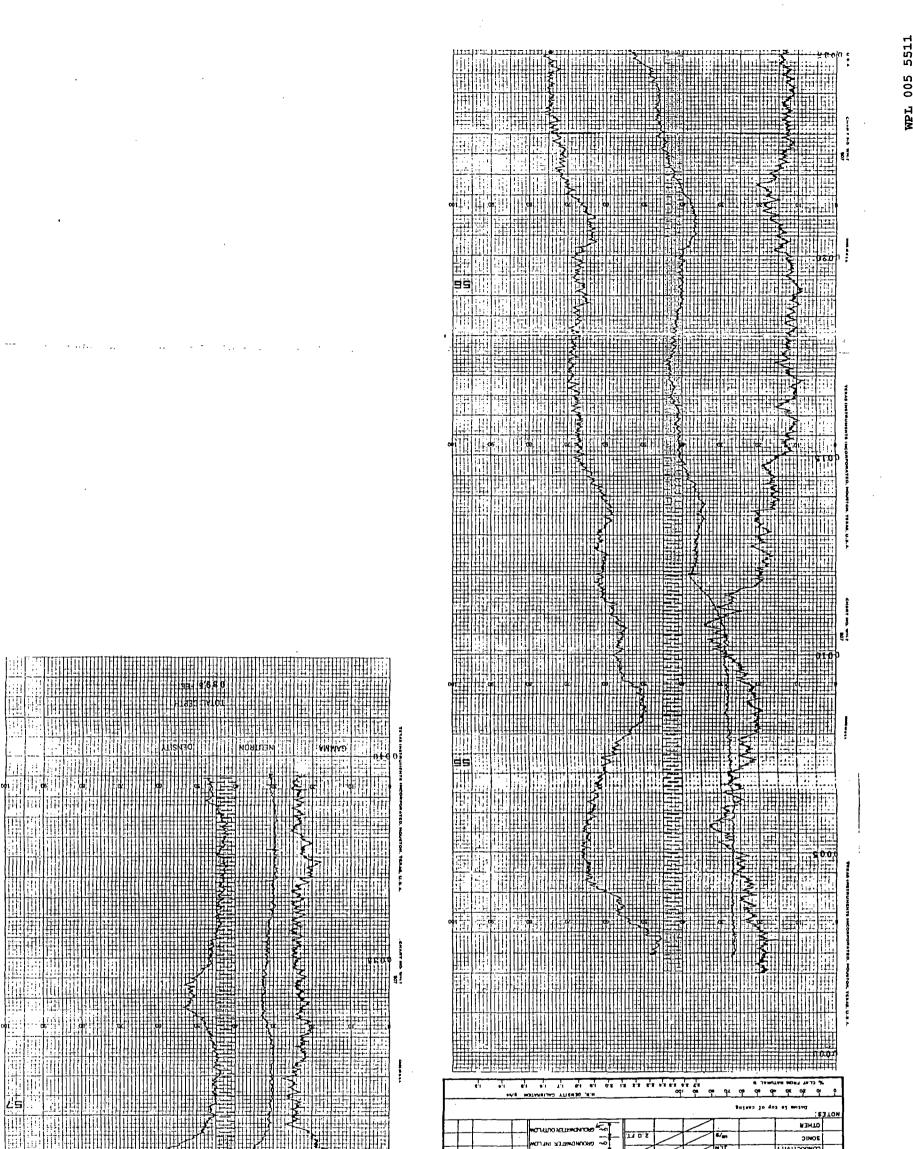


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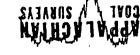




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**B · B** HOLE NO. DNKW3H 907

Well Log for 8-8, continued

APPENDIX C

PUMPING TEST DATA

WPL 005 5512



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SET 0021344

#### TABLE Cl

PUMPING INCREMENT	<u>pH</u> (pH units)	SPECIFIC CONDUCTANCE (umhos/cm)	TEMPERATURE °C
1	3.9	8,000	15°
2	3.1	9,500	12°
3	3.0	9,000	13°
4	2.85	9,200	12°
5	2.9	9,200	14°
6	2.85	8,000	_14°
7	2.95	8,000	13°
<sup>′</sup> 8	NA*	8,000	13° <sup>°</sup>

#### PUMPING TEST WATER QUALITY DATA

\* NA indicates information not available, due to malfunction of instrument.

WPL 005 5513



SET 0021345

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PROJECT NAME WESTINGHOUSE - BEAVER PLANT AREA A.9PROJECT NO. 85154
PIEZOMETER DESIGNATION B-1 FORMATION SENSED Sand + gravel
LOCATION DISTANCE FROM TEST WELL 25'
ELEVATION OF RISER PIPE SCREEN ELEVATION TO
DATE TEST BEGAN 11-25-85 TIME TEST BEGAN 11:43 AM
DEPTH TO WATER READ BY "M SCOPE" NO. IT-2

TIME	DEPTH TO WATER *	READ BY	ELAPSED TIME (MIN)	ELEVATION OF WATER	CORRE		CORRECTED ELEVATION OF WATER	REMARKS	
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PROJECT NAME WESTINGHOUSE-	BEAVER PLANT AREA A-9 PROJECT NO. 85154
PIEZOMETER DESIGNATION B.	IA FORMATION SENSED Fill
	DISTANCE FROM TEST WELL <u>31'</u>
ELEVATION OF RISER PIPE	SCREEN ELEVATION TO
DATE TEST BEGAN 11-25-85	TIME TEST BEGAN43 Am
DEPTH TO WATER READ BY "M	SCOPE" NO. IT-2

TIME	DEPTH TO WATER *	READ BY	ELAPSED TIME (MIN)	ELEVATION OF WATER	CORRE	CTIONS	CORRECTED ELEVATION OF WATER	REMARKS	
11:40	6.3	MPZ			<u> </u>	<u> </u>			
15:37	6.3'								
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 PROJECT NAME WESTINGHOUSE - BEAVER PLANT A.9 PROJECT NO.
 25154

 PIEZOMETER DESIGNATION B-1B
 FORMATION SENSED FILL - ALLUVIUM TILL

 LOCATION\_\_\_\_\_\_
 DISTANCE FROM TEST WELL 22'

 ELEVATION OF RISER PIPE\_\_\_\_\_\_
 SCREEN ELEVATION \_\_\_\_\_\_

 DATE TEST BEGAN\_11-25-85
 TIME TEST BEGAN\_11-25-85

 DEPTH TO WATER READ BY "M SCOPE" NO.
 TV-2

TIME	DEPTH TO WATER *	READ BY	ELAPSED TIME (MIN.)	ELEVATION OF WATER	CORRE	CTIONS	CORRECTED ELEVATION OF WATER	REMARKS
9:45	7.8	MPZ		<u></u>		<u> </u>		BEGAN PUMPING B-4@11:43
11:44 30	9.9		1.5					
11.46	9.9	i	3					
1208	9.7		25					
12 30	9,7		9			K		BEGAN PUMPING B-40 12:21
13 04	97		S			$\square$		BEGAN PUMPING B-4@ 12:56
13:39	9,7		6					BEGAN PUMPING 8-4@ 13:33
14:19	9.7		22					
15:10	9.7	Y	)	/				BEGAN PUMPING B-4@ 15:09
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PROJECT NAME WESTINGHOUSE - BEAVER !!	PLANT AREA A-9PROJECT NO. 85154
PIEZOMETER DESIGNATION B-2-8	FORMATION SENSED Sand + gravel
	DISTANCE FROM TEST WELL _/5
ELEVATION OF RISER PIPE	SCREEN ELEVATION TO
DATE TEST BEGAN 11-25-85 TIMI	E TEST BEGAN 11:43 Am
DEPTH TO WATER READ BY "M SCOPE" N	10. <u>IT-Z</u>

TO WATER *         BY (MIN)         TIME (MIN)         OF WATER         ATM **         TIDAL TIDAL OF WATER         ELEVATION OF WATER         REMARKS           9:50         DRY         MP2	
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PROJECT NAME WESTINGHOUSE -	BEAVER PLANT. AREA A-9 PROJECT NO. 85154
PIEZOMETER DESIGNATION	B-4 FORMATION SENSED
LOCATION	DISTANCE FROM TEST WELL
ELEVATION OF RISER PIPE	SCREEN ELEVATION TO
DATE TEST BEGAN 11-25-85	TIME TEST BEGAN 11:43 Am
DEPTH TO WATER READ BY "M	SCOPE" NO. IT-2

TIME	DEPTH TO WATER *	READ By	ELAPSED TIME (MIN.)	ELEVATION OF WATER	CORRE	TIDAL	CORRECTED ELEVATION OF WATER	REMARKS
11:43	8.5	MPZ	0				/	START PUMPING B-4
11:47	(		4	$\mathbf{N}$				2 GAL.
11:51	-		ß					4 GAL.
12:00	+		17					5 GAL (SHUT OFF FUMP)
2.01	10.0		18					
12:02	9.0'		19					
2:03	9.0		20					
12:04	87		21					
2 05 30	8.7		22.5	-				
12:07	8.7		24				7	
12:16	8.7		33		X		/	
12:21	-		3		Ν		1	START FUMPING B-4
12:25			4		$\square$			3 GAL.
12:27	-		6					4 GAL.
12:31			10				· · ·	5 GAL, (SHUT OFF FUMF)
12:33	11.1		12					
12:36	10.5		15					
12:38	9.5		17			$\mathbf{V}$		
12:39	9.3		18		/	Λ		
12:40	9.2		19 .			$\square$	<u> </u>	
12:43	9.0		22	L			<u> </u>	
12:56	-		<u></u>					START PUMPING B-4
12:58			2		/		l	2.5 GAL.
12.59			3		1/		ļ	3 GAL.
13:01	ļ	<u>                                      </u>	5	ļ	V	<b></b>	<u>}</u>	4 GAL
13.05	-		9.8		4	<u> </u>	l\	5 GAL (SHUT OFF PUMP)
13:07			11	/	1		<u> </u>	
13:08			/2	L/			<u> </u>	
13:08 "	9.6		12.5			<u> </u>	$\downarrow$	
13.11.30	10.5		15.5		<u> </u>	<u> </u>	<u>↓</u>	
13112	10.0		16		1	<u> </u>	<u>↓</u>	
13: 250	7.71		16.5		<u> </u>	1	$\downarrow $	L
13:13 30			17.5			<u> </u>		
13:15 50			19.5			ļ		
13:33			0	1/			<u> </u>	START PUINPING B-4
13:35		₩ V	2	V				2.5 GAL.

PROJECT NAME WESTINGHOUSE - BEAVER PLAN	IT. AREA A-9 PROJECT NO. 85154
PIEZOMETER DESIGNATION	FORMATION SENSED Sund & Gravel
LOCATIOND	ISTANCE FROM TEST WELL
ELEVATION OF RISER PIPE SCREI	
DATE TEST BEGAN 11-25-85 TIME TE	ST BEGAN 11:43 Am
DEPTH TO WATER READ BY "M SCOPE" NO.	IT-2

TIME	DEPTH TO WATER *	READ BY	ELAPSED TIME (MIN)	ELEVATION OF WATER	CORRE	TIDAL	CORRECTED ELEVATION OF WATER	REMARKS
13:37	_	MPZ	4	N				4 GAL
13:39	-		6	<u>\</u>		<u> </u>		45GAL
12:43	-		10		L	l		5 GAL. (SHUT OFF PUMP)
13:45	10.6	· ·	12		· .		//	
3:45 40	11.0'		12.7				/	
13:46'	10.1		13		ļ		/	
3:47	10.5'		14					·
13:49	9.5	<u> </u>	16		<u> </u>			
13:50	9. <b>4</b> ′		17		<u> </u>			···
13:53	9.2		20				/	
13:55	-		22					
13:57	-		0		$\Lambda$		1	START PUMPING B-4
13:59			2		$\downarrow \underline{\Lambda}$			2 GAL
14:0230			5.5					3 GAL.
14:08	-		11					4 GAL
14:10	12.3		13		$ \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{j=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{i=1}^{n} \sum_{$			5 GAL (SHUT OFF FUMP)
14:11	12.0		14		I = I	Ζ		
14:11 30	11.9		14.5		· .	$\checkmark$		
14:12	11.7		15		·	Λ		
14:13	11.2		16			$\square$		
14.1330	10.7		16.5	<u>i</u>			<u>.</u>	
14:14	10.4		17				· · · · ·	
14:14 30	10.3		17.5		/			
14:16	7.5		19		[/			
14:18	9.3		21		7 .		\	
14:21	-		0-		Χ		Ν	START PUMPING B-4
14:22	. –		<u> </u>		1			2 GAL
14:24"	-		3,5					3 GAL
14:27			6					3.5 GAL
14:33	-		12					5 GAL (SHUT OFF PUMP)
14: 34 30			13.5					· · · · · · · · · · · · · · · · · · ·
14.35	12.5'		14					
14:36	Ť		15				=	
14:36 30			15,5					
14:38			17	17				•
14:40	1	<u> </u> ↓	19	V				V.

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TIME	DEPTH	READ	ELAPSED	ELEVATION	(	CTIONS		
	TO WATER *	BY	TIME (MIN.)	OF WATER	ATM.**	TIDAL	ELEVATION OF WATER	REMARKS
14:42	9.5'	MPZ	21	Λ				
14:43	9.4		22					
14:46 30	-		0					START PUMPING B-4
14:48	-	· ·	1.5				/	2 GAL
14:50 30	-		4					3 GAL.
14:54	-		7.5					4 GAL.
14:59			12.5					5 GAL. (SHUT DEF PUMP)
15:00	13.8'		13.5					
15:00 <sup>20</sup>	13.3'		14			L		· · · · · · · · · · · · · · · · · · ·
15:01	12.8		14.5		<u> </u>		/	L
1 <u>5</u> :01 <sup>20</sup>	12.6'		15		¥		V	
15:0200	12.3		16		<u> </u>	ļ/	1	
15:02-0	12.0		17		$\square$	L_/		
15:06:0	10.2'		_ 20			/		
15:07 20	9.8		21				·	
15:09	9.5'		22.5		1			
15:12	-		0			Ζ		START PUMPING B-4
15:15			3		<u>`</u>	¥		2 GAL.
15:1630	_		4.5			<u>^</u>		3 GAL.
15:25	-		13		<u> </u>	<u> </u>	<u> </u>	5 GAL.
15:2930	-		17.5	1		$\square$		GGAL
15:0	-		26	<u></u>			·	9.5 GAL
16:11	-		59		<u> </u>	<u> </u>		15 GAL. (SHUT OFF PUMP
16:1230	13.2'		60.5		$\downarrow$			
14.13	129'		61		/		<u>\</u>	
16 1330	12.7'	<u> </u>	61.5		A		<u> </u>	
16:14	12.5'		62	//	<u> </u>			· · · · · · · · · · · · · · · · · · ·
16:14 30			62.5	<u> /</u>	<u> </u>		↓_\	
16:15	12.0'		63	<u> </u>		ļ	<u> </u>	· · · · · · · · · · · · · · · · · · ·
16.16	11.8'		64	<u>     /   </u>			<u> </u>	
16.17	11.4'		65	<u> </u>	ļ	<u> </u>		· · · · · · · · · · · · · · · · · · ·
16:18	10.9		64	<u> _/</u>			$ \rightarrow $	
16:19	10.5		67	1/			$\downarrow$	·
16:20	9.8'		68	ļ/	4		+	······
16:21	9.7'		69	<u> /</u>			·	
16:22	9.5'	I ↓	70	1/	1	1 '	·	

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\* FROM THE TOP OF THE RISER PIPE

PROJECT NAME <u>WESTINGHOUSE BEAVER PLANT, AREA A-9</u> PROJECT NO. <u>85154</u> PIEZOMETER DESIGNATION <u>8-4</u> FORMATION SENSED <u>Sand & gravel</u> LOCATION <u>DISTANCE FROM TEST WELL</u> ELEVATION OF RISER PIPE <u>SCREEN ELEVATION</u> <u>TO</u> DATE TEST BEGAN <u>11-25-85</u> TIME TEST BEGAN <u>11:43 AM</u> DEPTH TO WATER READ BY "M SCOPE" NO. <u>TT-2</u>

TIME	IME DEPTH TO WATER *		AD Y	ELAPSED TIME (MIN)	ELEVATION OF WATER	CORRE	TIDAL	CORRECTED ELEVATION OF WATER		REMARKS	
16:23	9.4'	M	۶	71							
6:25				73							
6:30	9.3'			78							
6:35	9.2'		·	83		(	-				
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PROJECT NAME WESTINGHOUSE - BEAVER PLAN	IT. AREA A-9 PROJECT NO. 85154
PIEZOMETER DESIGNATION 8-5	FORMATION SENSED Sand and gravel
	ISTANCE FROM TEST WELL
ELEVATION OF RISER PIPE SCRE	EN ELEVATION TO TO
DATE TEST BEGAN 11-25-85 TIME TE	ST BEGAN 11:43 Am
DEPTH TO WATER READ BY "M SCOPE" NO.	LT-2

TIME	DEPTH TO WATER *	READ BY	ELAPSED TIME (MIN.)	ELEVATION OF WATER	CORRE	TIDAL	CORRECTED ELEVATION OF WATER	REMARKS	
9:05	63.0	MPZ				-			
15:30	63.3'	MPZ		-					
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PROJECT NAME WESTINGHOUSE - BEAVER PLANT, AREA A-9 PROJECT NO. 35154 PIEZOMETER DESIGNATION <u>B-6</u> FORMATION SENSED Tetrace sands and gove LOCATION <u>DISTANCE FROM TEST WELL 37'</u> ELEVATION OF RISER PIPE SCREEN ELEVATION <u>TO</u> DATE TEST BEGAN <u>11-25-85</u> TIME TEST BEGAN <u>11-43 Am</u> DEPTH TO WATER READ BY "M SCOPE" NO. <u>II-2</u>

TIME	DEPTH TO WATER *	READ BY	ELAPSED TIME (MIN)	ELEVATION OF WATER	CORRE		CORRECTED ELEVATION OF WATER	REMARKS
9:42	10.5'	MPZ	0		Ì			BEGAN PUMPING B-4@11:43
11:54	11.0'		11	$\mathbf{N}$				
12:00	11.0		17					
12:09	ıı.5'		26					
12:09 30	<u>11.5</u>		245					
12-24	11.0		3					BEGAN PUMPING B-4@ 12:21
12:20	11.0		5					
12:28	11.4'		7				/	
12:20	11.7		9	· \				
12 41	1.4		20					
12:57	11.3				Y		V	BEGAN PUMPING By @ 12:56
12:59	11.5		3		$\Lambda$	/	1	
13'01	11.6		5		$\square$			
13:03	11.6		7					
13:20	11.7		24					
13:34	11.7							BEBAN PUMPING B-4@ 13:33
13:35	11.7		2			/		
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13:38	11.7		5			Ά		
13:55	11.5		22			<u> \</u>		
13:57	11.7		0					BEGAN PUMPING 8-4 @ 13:57
13:59	11.7		2					
14 01 30	11.7		4.5	ļ				
14:06	11.7		9		1/			
14:20	11.7		23		/	<u> </u>	¥	
14:21	11.7		0	ļ,	4		₽,	BEGAN PUMPING B-4@14:21
14:24 30	11.7	<u> </u>	3.5	ļ/	<u> </u>		↓	·····
14:27	11.7	┠- │	6	<u> /</u>			<u> </u>	
14:44	11.6		23	ļ	<u> </u>	<u> </u>	↓	
14:48	11.6	<b>↓</b>	1.5	<u> </u>	<u> </u>	ļ	<b>↓</b> \	BEGAN PUMPING B-4@ 446
14:53	11.5	<b> </b>	6.5	<u>     /                                </u>	<u> </u>		<u>↓                                    </u>	
<u>15 //</u>	11.7	<u> </u>	24.5	<u>     /                                </u>	<u> </u>		$\downarrow \longrightarrow$	
15:14	11.6		2		<u> </u>	<u> </u>	$\downarrow$ $\downarrow$	BEGAN PUMPING B-4@ 15:12
15:18	11.5		6		·	ļ	$\downarrow $	
15:26	11.6		14	/		<u> </u>	\	
15:33	11.6	Υ Υ	21	V			l^	SET 0021355

WPL 005 5523

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PROJECT NAME WESTINGHOUSE - BEAVE	<u>R PLANT, AREA A-9</u> PROJECT NO. <u>85154</u>
PIEZOMETER DESIGNATION B-6	FORMATION SENSED Sand & Gravel
	DISTANCE FROM TEST WELL
ELEVATION OF RISER PIPE	SCREEN ELEVATION TO
	TIME TEST BEGAN Am
DEPTH TO WATER READ BY "M SCOPI	E" NO. <u>IT-2</u>

TIME	DEPTH TO WATER *	READ BY	ELAPSED TIME (MIN.)	ELEVATION OF WATER	CORRE	TIDAL	CORRECTED ELEVATION OF WATER	REMARKS	
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# FROM	4 THE TO	P OF TH	IF RISER P	PIPE	## AT	MOSPHER	RIC		]

PROJECT NAME WESTINGHOUSE - BEAVEN	2. PLANT. AREA A-9 PROJECT NO. 85-154
PIEZOMETER DESIGNATION	FORMATION SENSED Sand + gravel
	DISTANCE FROM TEST WELL42'
ELEVATION OF RISER PIPE	SCREEN ELEVATION TO
DATE TEST BEGAN 11-25-85 TIL	ME TEST BEGAN 11:43 Am
DEPTH TO WATER READ BY "M SCOPE"	NO. <u>TT-2</u>

TIME	DEPTH	TH READ		ELAPSED	ELEVATION	CORRE	CTIONS	CORRECTED		
	TO WATER *	BY	TIME (MIN.)	OF WATER	ATN.**	TIDAL	ELEVATION OF WATER	REMARKS		
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5:34	17.4'	+					·			
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PROJECT NAME WESTINGHOUSE - BEA	IER PLANT AREA A-9 PROJECT NO. 85154
PIEZOMETER DESIGNATION_ B-8	FORMATION SENSED Sand + gravel
	DISTANCE FROM TEST WELL 16'
ELEVATION OF RISER PIPE	SCREEN ELEVATION TO
DATE TEST BEGAN 11-25-85	TIME TEST BEGAN 11:43 Am
DEPTH TO WATER READ BY "M SCO	DPE" NO. IT-2

TIME	DEPTH TO WATER *	READ BY	ELAPSED TIME (MIN.)	ELEVATION OF WATER		CTIONS	CORRECTED ELEVATION OF WATER	REMARKS	
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