

TETRA TECH, INC.

By: RH Date: 11/11/2016 Subject: Morgantown Road
Checked By: JB Date: 11/14/2016 PCSM Design and Evaluation

PURPOSE:

The purpose of these calculations is to design a Post-Construction Stormwater Management (PCSM) Plan for the Morgantown Road block valve site as part of the Sunoco Pipeline L.P. Pennsylvania Pipeline Project. The site is located within New Morgan Township, Berks County, Pennsylvania. Permanent stormwater controls will be developed to satisfy PADEP and New Morgan Township's approved Act 167 Plan.

PCSM DESIGN REQUIREMENTS:

The PCSM design for this project follows the PA Department of Environmental Protection's (PADEP) Pennsylvania Stormwater Best Management Practices Manual (BMP Manual), December 2006; and the standard design criteria from PA Title 25, Chapter 102.8.(g)(2) and (3). The design criteria evaluated for the site are summarized below.

Act 167 Consistency

The Morgantown Road block valve site is located in Berks County, which does not have a county-wide Act 167 Plan adopted. However, New Morgan Township has enacted the Schuylkill River Act 167 Plan. According to the management plan and associated map, the Morgantown block valve site is located in Management District A. The rate requirements of that district are such that the proposed 2-year storm should be reduced to the 1-year storm. All other proposed storm events should be reduced to their original existing condition. By designing in accordance with PADEP's Stormwater BMP Manual, the requirements outlined in Spring Township's Act 167 Plan will be fulfilled.

Recommended Volume Control Guideline

Use of Control Guideline 1 is recommended where site conditions offer the opportunity to reduce the increase in runoff volume as follows:

- Do not increase the post-development total runoff volume for all storms equal to or less than the two-year/24-hour event;
- Existing (pre-development) non-forested pervious areas must be considered meadow (good condition) or its equivalent; and
- 20 percent of existing impervious area, when present, shall be considered meadow (good condition) or its equivalent.

This site will utilize two slow-release BMPs to manage the two-year/24-hour volume increase.

Recommended Peak Rate Control Guideline

The recommended control guideline for peak rate control is:

• Do not increase the peak rate of discharge for the 2-year through 100-year events (at minimum); as necessary, provide additional peak rate control as required by applicable and approved Act 167 plan.

 New Morgan Township's approved Act 167 Plan establishes release rate requirements in postconstruction conditions to be at or better than the 1-year storm event. All other storm events should be at or better than their pre-construction release rate.

This site will utilize two slow-release BMPs to manage the two-year through 100-year peak rate increases. These BMPs, in conjunction with diversion channels and collection channels, will also help to increase the time of concentration within the drainage area.

Recommended Water Quality Control Guideline

Control Guideline 1 will provide water quality control and stream channel protection as well as flood control protection. The use of a slow-release BMP has been approved by PADEP as an appropriate way to meet the requirements of Control Guideline 1 when onsite infiltration is not feasible.

Infiltration

Infiltration rates for the PCSM BMPs have been determined from site infiltration testing conducted in accordance of the PA BMP Manual. Documentation for infiltration testing and design infiltration rates can be found in Attachment 5 of the Site Restoration/Post Construction Stormwater Management Plan. Infiltration test locations and recommended design rates are also labeled on the PCSM Plan Drawings in Attachment 6.

During the onsite infiltration tests, the depth to seasonal high groundwater and shallow bedrock or another confining layer were evaluated. The post-construction stormwater management facility for the site has been designed to maintain 2 feet of separation between the ponding elevation of the facility and the seasonal high water table and bedrock.

The post-construction stormwater management design utilizes two slow-release BMPs to manage runoff volume due to rapid infiltration onsite.

Loading Ratio

The loading ratio guidelines do not apply because the design does not propose an infiltration BMP.

Disturbed Area

To meet Standard Worksheet 10 guidelines, 90% of the disturbed area is contained by the proposed PCSM BMPs.

Karst Topography

Morgantown Road block valve site is not located within an area of karst terrain.

Special Protection Watershed

Morgantown Road block valve is located within a special protection watershed. The project site was designed to minimize the total amount of impervious area. The impervious area for the Morgantown Road block valve was limited to the amount that is required to safely construct and operate the block valve. In addition, the previously proposed gravel turn-around was eliminated, and replaced with a grass area.

Non-discharge alternatives were analyzed for this block valve site. The location of the Morgantown Road block valve site was evaluated by ASME B31.4 Valve Spacing 434.15.2(e) which states that mainline valves should not be more than 7.5 miles apart. The valve sites were located in such a way that they avoided environmentally sensitive areas (such as wetlands and floodplains), were close to an existing road, and close to power. Land owner preference was also accounted for while locating the block valve sites. Once all of these factors were taken into account, several block valve sites, including Morgantown Road, were located in special protection watersheds.

Non-discharge alternatives were also considered when determining the type of BMP proposed. Morgantown Road block valve site utilizes slow-release BMPs to manage stormwater. Stormwater runoff is infiltrated to the maximum extent possible. Stormwater runoff is spread out to flow through areas that have been restored to meadow conditions, to slow-release BMPs, or to undisturbed area. There will not be an increase in stormwater runoff rate or volume to prevent the physical degradation of the receiving water, such as scour, and stream bank destabilization. Stormwater runoff volume is not increasing throughout post-construction, and any post-construction stormwater discharge is managed so that it will not degrade the physical, chemical or biological characteristics of the receiving stream.

Runoff from the site will be managed by two slow-release BMPs. Ponded runoff will be temporarily stored upslope of the trench until it infiltrates and filters through the soil media. Due to the design of the slow-release BMPs the stormwater runoff will be released in sheet flow down a stabilized slope, without causing erosion, rather than concentrating the flow. Filtration through the existing vegetation and soil is an efficient way to remove suspended stormwater pollutants such as sediment, as the suspended particles are physically filtered from the stormwater as it flows through the vegetation and percolates into the soil.

The extent of the disturbed area will be minimized, and the duration of disturbance will be minimized by stabilizing disturbed areas as soon as practicable. Cut and fill for the project site has been minimized. Where possible based on the criteria listed above, sites were located in areas with shallow slopes to minimize the amount of cut and fill required. At Morgantown Road block valve site, the grading was done to tie into existing contours, which did lead to some cut and fill requirements. This was done so that the block valve site was graded towards the natural slope. No direct discharge to surface water occurs at the site. The site will be restored promptly with proper vegetative cover techniques.

Antidegradation requirements for the special protection watershed are met because the post-construction stormwater infiltration volume equals or exceeds the pre-construction stormwater infiltration volume, and post-construction stormwater discharge is pretreated via infiltration berms. The runoff is managed so that it will not degrade the physical, chemical, or biological characteristics of the receiving stream.

BERKS COUNTY, PENNSYLVANIA

FIGURE NUMBER

2

0



NOAA Atlas 14, Volume 2, Version 3 Location name: Robeson Twp, Pennsylvania, USA*

Latitude: 40.1887°, Longitude: -75.8815° Elevation: 675.45 ft**



source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

D	Average recurrence interval (years)									
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.339 (0.308-0.373)	0.403 (0.366-0.444)	0.471 (0.427-0.519)	0.519 (0.471-0.571)	0.574 (0.518-0.631)	0.611 (0.548-0.671)	0.648 (0.579-0.711)	0.679 (0.604-0.746)	0.714 (0.631-0.785)	0.740 (0.650-0.815
10-min	0.540 (0.491-0.595)	0.643 (0.585-0.710)	0.753 (0.683-0.830)	0.829 (0.751-0.912)	0.913 (0.824-1.00)	0.972 (0.872-1.07)	1.03 (0.919-1.13)	1.07 (0.957-1.18)	1.13 (0.997-1.24)	1.16 (1.02-1.28)
15-min	0.675 (0.613-0.744)	0.808 (0.735-0.892)	0.952 (0.864-1.05)	1.05 (0.950-1.15)	1.16 (1.04-1.27)	1.23 (1.10-1.35)	1.30 (1.16-1.43)	1.35 (1.21-1.49)	1.42 (1.25-1.56)	1.46 (1.28-1.61)
30-min	0.924 (0.839-1.02)	1.11 (1.01-1.23)	1.35 (1.23-1.49)	1.52 (1.37-1.67)	1.71 (1.54-1.88)	1.85 (1.66-2.03)	1.98 (1.77-2.18)	2.10 (1.87-2.31)	2.25 (1.99-2.48)	2.36 (2.07-2.60)
60-min	1.15 (1.05-1.27)	1.40 (1.27-1.54)	1.73 (1.57-1.91)	1.97 (1.79-2.17)	2.28 (2.05-2.50)	2.50 (2.25-2.75)	2.73 (2.44-3.00)	2.95 (2.63-3.25)	3.23 (2.86-3.55)	3.44 (3.02-3.79)
2-hr	1.38 (1.24-1.54)	1.67 (1.50-1.88)	2.08 (1.86-2.33)	2.40 (2.15-2.69)	2.83 (2.51-3.16)	3.17 (2.80-3.54)	3.52 (3.09-3.93)	3.88 (3.38-4.33)	4.36 (3.77-4.87)	4.74 (4.06-5.30)
3-hr	1.51 (1.35-1.70)	1.83 (1.64-2.06)	2.28 (2.04-2.57)	2.63 (2.35-2.96)	3.10 (2.75-3.48)	3.48 (3.06-3.89)	3.86 (3.38-4.32)	4.25 (3.69-4.75)	4.79 (4.12-5.35)	5.20 (4.44-5.83)
6-hr	1.88 (1.68-2.13)	2.27 (2.04-2.57)	2.83 (2.53-3.20)	3.28 (2.92-3.70)	3.92 (3.46-4.40)	4.43 (3.89-4.97)	4.98 (4.34-5.57)	5.57 (4.80-6.22)	6.39 (5.44-7.14)	7.05 (5.93-7.89)
12-hr	2.30 (2.05-2.61)	2.77 (2.47-3.15)	3.47 (3.09-3.93)	4.05 (3.60-4.58)	4.89 (4.30-5.50)	5.61 (4.88-6.27)	6.38 (5.50-7.13)	7.22 (6.15-8.05)	8.44 (7.07-9.42)	9.47 (7.81-10.6)
24-hr	2.68 (2.43-2.98)	3.22 (2.93-3.59)	4.04 (3.66-4.49)	4.72 (4.26-5.24)	5.70 (5.12-6.32)	6.53 (5.84-7.22)	7.41 (6.59-8.18)	8.37 (7.39-9.24)	9.76 (8.53-10.8)	10.9 (9.46-12.1)
2-day	3.11 (2.80-3.49)	3.76 (3.38-4.21)	4.72 (4.24-5.28)	5.50 (4.92-6.15)	6.60 (5.88-7.37)	7.51 (6.67-8.37)	8.48 (7.49-9.45)	9.51 (8.34-10.6)	11.0 (9.55-12.2)	12.2 (10.5-13.6)
3-day	3.28 (2.96-3.68)	3.96 (3.56-4.44)	4.96 (4.46-5.55)	5.76 (5.17-6.45)	6.91 (6.17-7.72)	7.85 (6.98-8.76)	8.85 (7.82-9.87)	9.91 (8.71-11.1)	11.4 (9.94-12.8)	12.7 (10.9-14.1)
4-day	3.45 (3.11-3.87)	4.16 (3.75-4.67)	5.19 (4.68-5.83)	6.03 (5.41-6.75)	7.22 (6.45-8.06)	8.19 (7.29-9.15)	9.22 (8.15-10.3)	10.3 (9.07-11.5)	11.9 (10.3-13.3)	13.1 (11.4-14.7)
7-day	4.05 (3.67-4.49)	4.86 (4.40-5.38)	6.00 (5.43-6.64)	6.94 (6.26-7.67)	8.28 (7.44-9.14)	9.38 (8.40-10.4)	10.5 (9.40-11.6)	11.8 (10.4-13.0)	13.6 (11.9-14.9)	15.0 (13.1-16.6)
10-day	4.63 (4.21-5.11)	5.53 (5.03-6.11)	6.73 (6.12-7.44)	7.70 (6.98-8.50)	9.05 (8.18-9.98)	10.1 (9.13-11.2)	11.3 (10.1-12.4)	12.4 (11.1-13.7)	14.0 (12.4-15.5)	15.3 (13.5-17.0)
20-day	6.26 (5.79-6.81)	7.42 (6.86-8.08)	8.85 (8.18-9.62)	9.97 (9.21-10.8)	11.5 (10.6-12.5)	12.7 (11.6-13.7)	13.9 (12.7-15.0)	15.0 (13.7-16.3)	16.7 (15.1-18.1)	17.9 (16.2-19.5)
30-day	7.78 (7.25-8.38)	9.17 (8.55-9.88)	10.7 (9.96-11.5)	11.9 (11.0-12.8)	13.4 (12.4-14.4)	14.6 (13.5-15.7)	15.7 (14.5-16.9)	16.8 (15.5-18.2)	18.3 (16.8-19.7)	19.4 (17.7-20.9)
45-day	9.85 (9.26-10.5)	11.6 (10.9-12.3)	13.3 (12.5-14.2)	14.6 (13.7-15.6)	16.2 (15.2-17.3)	17.4 (16.3-18.6)	18.6 (17.4-19.8)	19.7 (18.4-21.0)	21.0 (19.5-22.4)	22.0 (20.4-23.5)
60-day	11.8 (11.1-12.6)	13.8 (13.1-14.7)	15.8 (14.9-16.8)	17.2 (16.3-18.3)	19.1 (18.0-20.3)	20.4 (19.2-21.7)	21.6 (20.3-23.0)	22.7 (21.3-24.2)	24.1 (22.6-25.7)	25.1 (23.5-26.7)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

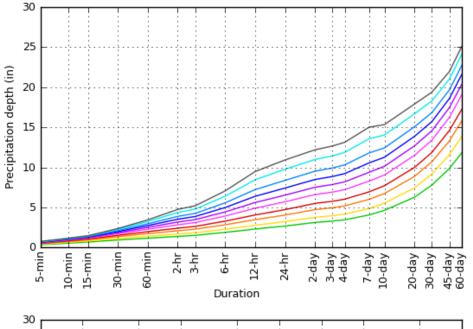
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

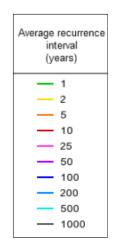
Please refer to NOAA Atlas 14 document for more information.

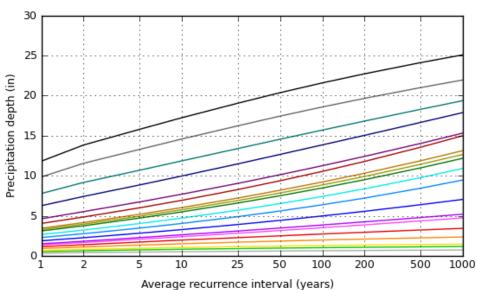
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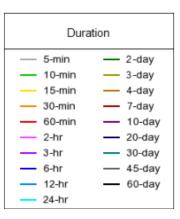
PF graphical

PDS-based depth-duration-frequency (DDF) curves Latitude: 40.1887°, Longitude: -75.8815°







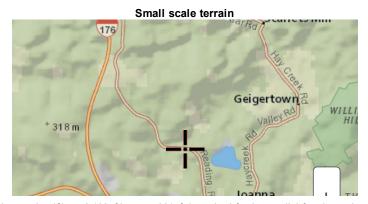


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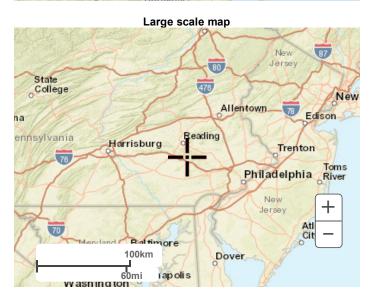
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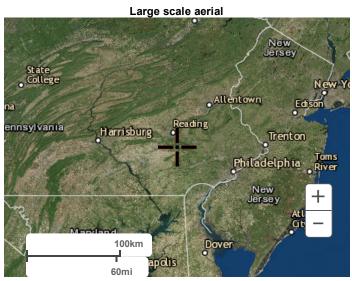
Maps & aerials





Large scale terrain Brig . Ne NNSYLVANIA Allentown Edison Harrisburg Trenton Phil delphia Toms Riv + MARYLAND NEW JERSEY Atlantic City 100km Dover 60mi Delaware Bay





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US Department of Commerce

National Oceanic and Atmospheric Administration

National Weather Service

National Water Center

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Disclaimer

WORKSHEET 1. GENERAL SITE INFORMATION

Date:	November 11, 2016	
Project Name:	Morgantown Road	
Municipality:	New Morgan	
County:	Berks	
Total Area (acres):	8.23	
Major River Basin:	Delaware	
Watershed:	Lower Delaware	
Sub Basin:	Schuylkill	
Nearest Surface Wa	ter to Receive Runoff: Hay Creek	
Chapter 93 - Design	ated Water Use: Exceptional Value (EV)	
Impaired according List Causes	YES X NO	
Is Project Subject to	o, or Part of:	
Municipal Se	eparate Storm Sewer System (MS4) Requirements	YES
Existing or F	NO X YES	
If yes, distan	NO X	
Approved A	ct 167 Plan?	YES X
Existing Rive	er Conservation Plan?	NO YES NO X

Worksheet 2. Sensitive Natural Resources

INSTRUCTIONS

1. Provide Sensitive Resources Map according to non-structural BMP 5.4.1 in Chapter 5. This map should identify wetlands, woodlands, natural drainage ways, steep slopes, and other sensitive natural areas.

See pre-development drainage area map

2. Summarize the existing extent of each sensitive resource in the Existing Sensitive Resources Table (below, using Acres). If none present, insert 0.

Woodlands - 1.43 acres

3. Summarize Total Protected Area as defined under BMPs in Chapter 5.

0.00 acres

4. Do not count any area twice. For example, an area that is both a floodplain and a wetland may only be considered once.

EXISTING NATURAL SENSITIVE RESOURCE	MAPPED? Yes/no/n/a	TOTAL AREA (Ac.)	PROTECTED AREA (Ac.)
Waterbodies	N/A		
Floodplains	N/A		
Riparian Areas	N/A		
Wetlands	N/A		
Woodlands	Yes	1.43	0.00
Natural Drainage Ways	N/A		
Steep Slopes, 15% - 25%	N/A		
Steep Slopes, over 25%	N/A		
Other:			
Other:			
TOTAL EXISTING:		1.43	0.00

Worksheet 3. Nonstructural BMP Credits	
PROTECTED AREA	
1.1 Area of Protected Sensitive/Special Value Features (see WS 2)	0.00 Ac.
	0.00 AC.
1.2 Area of Riparian Forest Buffer Protection	<u>0.00</u> Ac.
3.1 Area of Minimum Disturbance/Reduced Grading	<u>0.00</u> Ac
TOTAL	<u>0.00</u> Ac
Site Area Minus Area - O This is the area that requires stormwater management Minus Area - Stormwater Management	Area
VOLUME CREDITS	
3.1 Minimum Soil Compaction (See Chapter 8, page 22 – SW BMP Manual)	
Lawn ft ² x 1/4" x 1/12 =	ft³
Meadow ft^2 x 1/3" x 1/12 =	ft ³
3.3 Protect Existing Trees (See Chapter 8, page 23 – SW BMP Manual)	
For Trees within 100 feet of impervious area:	
Tree Canopy ft ² x 1/2" x 1/12 =	ft³
5.1 Disconnect Roof Leaders to Vegetated Areas (See Chapter 8 page 25 – SW BMP Manual))
For runoff directed to areas protected under 5.8.1 and 5.8.2	
Roof Area ft ² x 1/3" x 1/12 =	ft ³
For all other disconnected roof areas	
Roof Area ft ² x 1/4" x 1/12 =	ft ³
5.2 Disconnect Non-Roof impervious to Vegetated Areas (See Chapter 8, page 26 – SW BN	/IP Manual)
For Runoff directed to areas protected under 5.8.1 and 5.8.2	
Impervious Area ft ² x 1/3" x 1/12 =	ft ³
For all other disconnected roof areas	
Impervious Area ft ² x 1/4" x 1/12 =	ft ³
*For use on Worksheet 5	ft³

WORKSHEET 4. CHANGE IN RUNOFF VOLUME FOR 2-YR STORM EVENT

PROJECT: Morgantown Road 8.23 acres Drainage Area:

3.22 in 2-Year Rainfall:

Total Site Area: 1.43 acres Protected Site Area: N/A acres Managed Site Area: 1.43 acres

Existing Conditions

Cover Type/Condition	Soil Type	Area (sf)	Area (ac)	CN	S	la (0.2*S)	Q Runoff ¹ (in)	Runoff Volume ³ (ft ³)
Woods	В	62,291	1.43	55	8.18	1.64	0.26	1,333
TOTAL:		62,291	1.43					1,333

Developed Conditions

Cover Type/Condition	Soil Type	Area (sf)	Area (ac)	CN	S	la (0.2*S)	Q Runoff ¹ (in)	Runoff Volume ³ (ft ³)
Impervious-Gravel	В	8,712	0.20	85	1.76	0.35	1.77	1,288
Meadow	В	53,579	1.23	58	7.24	1.45	0.35	1,555
Woods	В	0	0.00	55	8.18	1.64	0.26	0
TOTAL:		62,291	1.43					2,843

2-Year Volume Increase (ft ³):	1,510
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2-Year Volume Increase = Developed Conditions Runoff Volume - Existing Conditions Runoff Volume

1. Runoff (in) = Q = (P - 0.2S)2 / (P + 0.8S) where

P = 2-Year Rainfall (in) S = (1000/CN)-10

2. Runoff Volume (CF) = $Q \times Area \times 1/12$

Q = Runoff (in)

Area = Land use area (sq. ft.)

Note: Runoff Volume must be calculated for EACH land use type/condition and HSGI. The use of a weighted CN value for volume calculations is not acceptable.

PROJECT SUB-BAS		Morgantown Road				
	Required Co	ontrol Volume (ft³) - from Workshe	et 4:			1,510
	Non-structu	ral Volume Credit (ft³) - <i>from Worl</i>	ksheet 3:			N/A
		(maximum is 25% of re		ne)		,
		Ctrustural Valuma Da	(£4 ³).			4.540
	(Required C	Structural Volume Recontrol Volume minus Non-structu	. , ,			1,510
Propos		n PA Stormwater Best Managemer ces Manual Chapter 6	nt A	rea (ft²)		ne Reduction ently Removed (ft ³)
6.4.1	Porous Pave					
6.4.2	Infiltration Ba					
6.4.3	Infiltration Be					
6.4.4	Infiltration Tr					
6.4.5		/Bioretention				
6.4.6 6.4.7	Dry Well/See					
6.4.8	Vegetated S					
6.4.9	Vegetated 5					
6.4.10	Berm	iller ourp				
6.5.1	Vegetated R	oof				
6.5.2	Capture and					
6.6.1	Constructed					
6.6.2	Wet Pond/Re	etention Basin				
6.7.1	Riparian Buf	fer/Riparian Forest Buffer Restoratio	n			
6.7.2		Restoration/Reforestation				
6.7.3	Soil Amendn					
6.8.1	Level Spread					
6.8.2	Special Stora					
Other:	Slow Releas	e Concept		504		2,421
		Tota	I Structural	l Volume (ft ³):		2,421
		Structural V	olume Requ	uirement (ft ³):		1,510
				DIFFERENCE:		-911
VOLUME	CREDIT DET	ERMINATION - BERM A				
1	Detained are	ea runoff volume from Hydraflow		= 1,855	cf	
2	Storage volu	me of the BMP		= 1,485	cf	
3		lume within 72 hours after the 2-yr/24 w rate of slow release dewatering	1-hr event	= 2,333	cf	
VOLUME	CREDIT DET	ERMINATION - BERM B				
1	Detained are	ea runoff volume from Hydraflow		= 2,323	cf	
2	Storage volu	me of the BMP		= 936	cf	
3		lume within 72 hours after the 2-yr/24 w rate of slow release dewatering	1-hr event	= 2,074	cf	

WORKSHEET 10. WATER QUALITY COMPLIANCE FOR	RNITRATE
Does the site design incorporate the following BMPs to address nitrate pollution? A rating is achieved if at least 2 Primary BMPs for nitrate are provided across the site o BMPs for nitrate are provided across the site (or the	
PRIMARY BMPs FOR NITRATE:	YES NO
NS BMP 5.4.2 - Protect / Conserve / Enhance Riparian Buffers	
NS BMP 5.5.4 - Cluster Uses at Each Site	
NS BMP 5.6.1 - Minimize Total Disturbed Area	\boxtimes
NS BMP 5.6.3 - Re-Vegetate / Re-Forest Disturbed Areas (Native Species)	\triangleright
NS BMP 5.9.1 - Street Sweeping / Vacuuming	
Structural BMP 6.7.1 - Riparian Buffer Restoration	
Structural BMP 6.7.2 - Landscape Restoration	
SECONDARY BMPs FOR NITRATE:	
NS BMP 5.4.1 - Protect Sensitive / Special Value Features	
NS BMP 5.4.3 - Protect / Utilize Natural Drainage Features	
NS BMP 5.6.2 - Minimize Soil Compaction	\boxtimes
Structural BMP 6.4.5 - Rain Garden / Bioretention	
Structural BMP 6.4.8 - Vegetated Swale	
Structural BMP 6.4.9 - Vegetated Filter Strip	
Structural BMP 6.6.1 - Constructed Wetland	
Structural BMP 6.7.1 - Riparian Buffer Restoration	
Structural BMP 6.7.2 - Landscape Restoration	
Structural BMP 6.7.3 - Soils Amendment/Restoration	\square

TIME OF CONCENTRATION ADJUSTMENT - BERM A

POST CONSTRUCTION TC TO BMP (DETAINED TC) BEFORE ADJUSTMENT

9.7 MIN

STRUCTURAL VOLUME PROVIDED BY BMP

1,485 CF

RATES OF RUNOFF TO THE BMP (FROM HYDRAFLOW REPORT)

Storm Event	Q (CFS)
2 YR/24 HR	0.633
10 YR/24 HR	2.744
50 YR/24 HR	6.042
100 YR/24 HR	7.828

ADDITIONAL RESIDENCE TIME (MIN) = (STRUCTURAL VOLUME PROVIDED BY BMP / RATE OF RUNOFF TO BMP) / 60

Storm Event	Q (CFS)	Additional Residence Time (min.)
2 YR/24 HR	0.633	39.100
10 YR/24 HR	2.744	9.020
50 YR/24 HR	6.042	4.096
100 YR/24 HR	7.828	3.162

ADJUSTED TC = POST CONSTRUCTION TC TO BMP BEFORE ADJUSTMENT + ADDITIONAL RESIDENCE TIME

		Additional	Adjusted Time of
Storm Event	Q (CFS)	Residence Time	Concentration
		(min.)	(min.)
2 YR/24 HR	0.633	39.100	48.800
10 YR/24 HR	2.744	9.020	18.720
50 YR/24 HR	6.042	4.096	13.796
100 YR/24 HR	7.828	3.162	12.862

TIME OF CONCENTRATION ADJUSTMENT - BERM B

POST CONSTRUCTION TC TO BMP (DETAINED TC) BEFORE ADJUSTMENT

10 MIN

STRUCTURAL VOLUME PROVIDED BY BMP

936 CF

RATES OF RUNOFF TO THE BMP (FROM HYDRAFLOW REPORT)

Storm Event	Q (CFS)
2 YR/24 HR	0.881
10 YR/24 HR	5.064
50 YR/24 HR	12.18
100 YR/24 HR	16.1

ADDITIONAL RESIDENCE TIME (MIN) = (STRUCTURAL VOLUME PROVIDED BY BMP / RATE OF RUNOFF TO BMP) / 60

Storm Event	Q (CFS)	Additional Residence Time (min.)
		(1111111)
2 YR/24 HR	0.881	17.707
10 YR/24 HR	5.064	3.081
50 YR/24 HR	12.180	1.281
100 YR/24 HR	16.100	0.969

ADJUSTED TC = POST CONSTRUCTION TC TO BMP BEFORE ADJUSTMENT + ADDITIONAL RESIDENCE TIME

		Additional	Adjusted Time of
Storm Event	Q (CFS)	Residence Time	Concentration
		(min.)	(min.)
2 YR/24 HR	0.881	17.707	27.707
10 YR/24 HR	5.064	3.081	13.081
50 YR/24 HR	12.180	1.281	11.281
100 YR/24 HR	16.100	0.969	10.969

Underdrain Dewatering Rate Calculation

Project: Morgantown BMP: A

		Filter Media		
			Min. Infiltration Rate	
Layer	Media	Thickness - T (ft)	- K (ft/min) ¹	Flow Rate (cfs) ²
1	Clean Gravel	N/A	2	N/A
2	Coarse Sand	N/A	0.02	N/A
3	Fine Sand	2	0.002	0.00900
4	Other ³	N/A	N/A	N/A
=		·	linimum Flow Rate (cfs)	0.009

1. From Principles of Geotechnical Engineering Third Edition, Braja Das, 1994

2. Q=KA(Hm+T/T)

A = Area (square feet) = 270 Hm = Head above media (feet) = 2

3. Infiltration rate measured in field or laboratory

	Perforated Pipe			
	Perforation Area (square	# Perforations per Foot -		
Pipe	inch) ⁴	N	Pipe Length - L (ft)	Flow Rate (cfs) ⁵
1	1.00	1	90	4.42
2	N/A	N/A	N/A	N/A
			Total Flow Rate (cfs)	4.42

4. Reference: PVC: certainteed.com HDPE: ads-pipe.com

5. Q= N*L*cAov(2GH)

 $c = \text{Orifice Coefficient} = 0.6 \\ \text{Ao= Perforation Area (sq. ft.)} \\ \text{G= Grav. Accel. (ft/sec^2)} \\ \text{H= Average Head (ft)} = 3.5 \\$

Pipe Discharge				
Pipe	Pipe Diameter - D (in)	Pipe Roughness Coefficient -n	Pipe Slope - S ⁶	Flow Rate (cfs) ⁷
1	4	0.012	0.003703704	0.09
2	N/A	N/A	N/A	N/A
			Total Flow Rate (cfs)	0.09

6. For flat pipe, use hydraulic grade (pipe diameter/pipe length) for the pipe slope

7. From Manning's equation (attach separate calculation worksheet)

Limiting flow rate from combined underdrain system - QI (cfs) =	0.009
Detained volume based on 2-year/24-hour storm (cu-ft) =	1,485
Total Dewatering Volume including volume in voids(cu-ft) =	1,701
Dewatering Time (sec) = 2HA/QI =	189,000
Dewatering Time (hrs) =	52.50

Underdrain Dewatering Rate Calculation

Project: Morgantown BMP: B

		Filter Media		
Min. Infiltration Rate				
Layer	Media	Thickness - T (ft)	- K (ft/min) ¹	Flow Rate (cfs) ²
1	Clean Gravel	N/A	2	N/A
2	Coarse Sand	N/A	0.02	N/A
3	Fine Sand	2	0.002	0.00780
4	Other ³	N/A	N/A	N/A
_		M	linimum Flow Rate (cfs)	0.008

1. From Principles of Geotechnical Engineering Third Edition, Braja Das, 1994

2. Q=KA(Hm+T/T)

A = Area (square feet) = 234 Hm = Head above media (feet) = 2

3. Infiltration rate measured in field or laboratory

	Perforated Pipe			
Pipe	Perforation Area (square inch) ⁴	Pipe Length - L (ft)	Flow Rate (cfs) ⁵	
1	1.00	1	78	3.83
2	N/A	N/A	N/A	N/A
_	_	-	Total Flow Rate (cfs)	3.83

4. Reference: PVC: certainteed.com HDPE: ads-pipe.com

5. Q= N*L*cAov(2GH)

 $c = \text{Orifice Coefficient} = 0.6 \\ \text{Ao= Perforation Area (sq. ft.)} \\ \text{G= Grav. Accel. (ft/sec^2)} \\ \text{H= Average Head (ft)} = 3.5 \\$

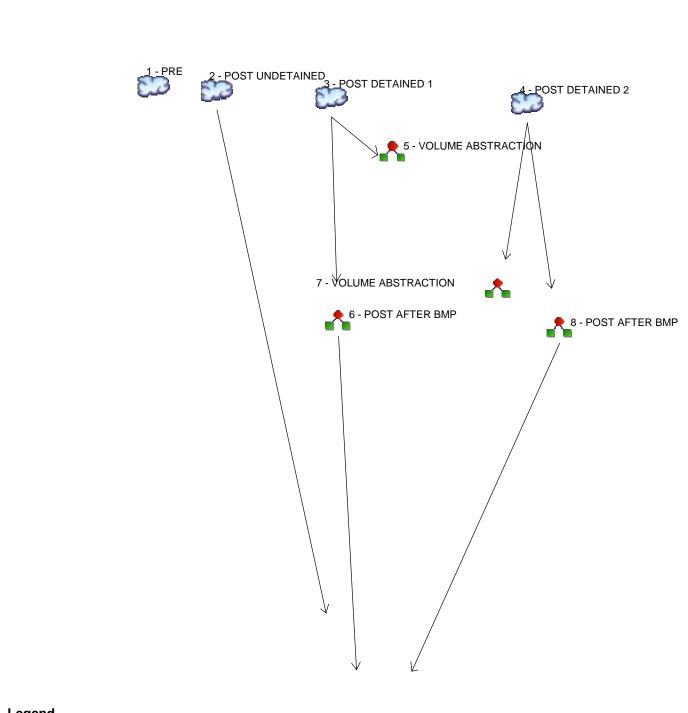
Pipe Discharge				
Pipe	Pipe Diameter - D (in)	Pipe Roughness Coefficient -n	Pipe Slope - S ⁶	Flow Rate (cfs) ⁷
1	4	0.012	0.004273504	0.09
2	N/A	N/A	N/A	N/A
			Total Flow Rate (cfs)	0.09

6. For flat pipe, use hydraulic grade (pipe diameter/pipe length) for the pipe slope

7. From Manning's equation (attach separate calculation worksheet)

Limiting flow rate from combined underdrain system - QI (cfs) =	0.008
Detained volume based on 2-year/24-hour storm (cu-ft) =	936
Total Dewatering Volume including volume in voids(cu-ft) =	1,123
Dewatering Time (sec) = 2HA/QI =	144,000
Dewatering Time (hrs) =	40.00

Watershed Model Schematic



Legend

Hyd.	<u>Origin</u>	<u>Description</u>	
1	SCS Runoff	f PRE	
2	SCS Runoff	f POST UNDETAINED 9 - POST AT POI	
3	SCS Runoff	f POST DETAINED 1	
4	SCS Runoff	f POST DETAINED 2	
5	Diversion1	VOLUME ABSTRACTION	
6	Diversion2	POST AFTER BMP	
7	Diversion1	VOLUME ABSTRACTION	
8	Diversion2	POST AFTER BMP	
9	Combine	POST AT POI	

Project: Morgantown.gpw

Tuesday, 11 / 1 / 2016

Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff			1.490			9.557		23.26	30.82	PRE
2	SCS Runoff			0.413			2.274		5.332	7.008	POST UNDETAINED
3	SCS Runoff			0.633			2.744		6.042	7.828	POST DETAINED 1
4	SCS Runoff			0.881			5.064		12.18	16.10	POST DETAINED 2
5	Diversion1	3		0.633			2.744		3.874	2.915	VOLUME ABSTRACTION
6	Diversion2	3		0.040			2.028		6.042	7.828	POST AFTER BMP
7	Diversion1	4		0.881			3.912		2.351	1.491	VOLUME ABSTRACTION
8	Diversion2	4		0.346			5.064		12.18	16.10	POST AFTER BMP
9	Combine	2, 6, 8		0.471			8.625		23.27	30.70	POST AT POI

Proj. file: Morgantown.gpw

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Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

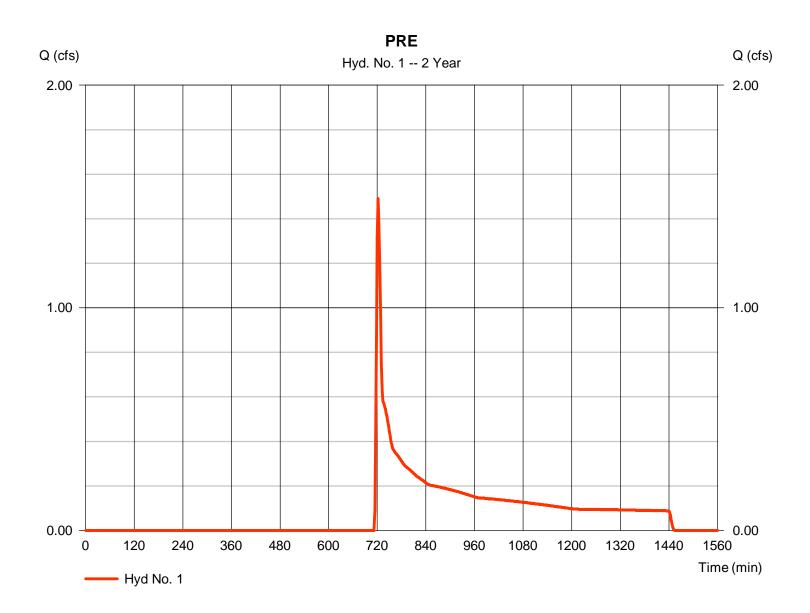
Tuesday, 11 / 1 / 2016

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 1.490 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 2 min Hyd. volume = 7,671 cuftDrainage area Curve number = 8.230 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 3.22 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(7.780 \times 55) + (0.450 \times 58)] / 8.230$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 3.22 = 4.00	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpave =5.18	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})71.0		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							8.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

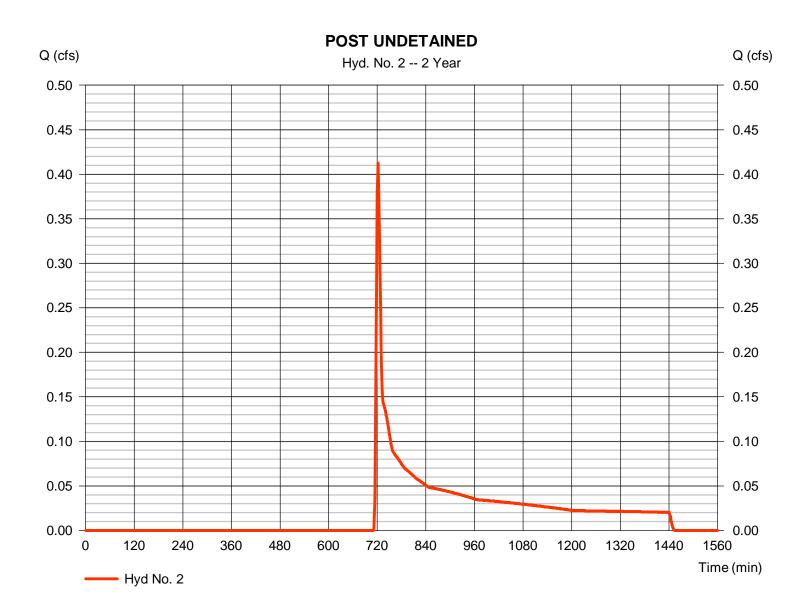
Tuesday, 11 / 1 / 2016

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 0.413 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 2 min Hyd. volume = 1,855 cuftDrainage area Curve number = 1.790 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 3.22 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.030 \times 85) + (0.430 \times 58) + (1.330 \times 55)] / 1.790$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4 $\,$

Hyd. No. 2POST UNDETAINED

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 3.22 = 4.00	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpaved =5.18	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})71.0		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							8.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

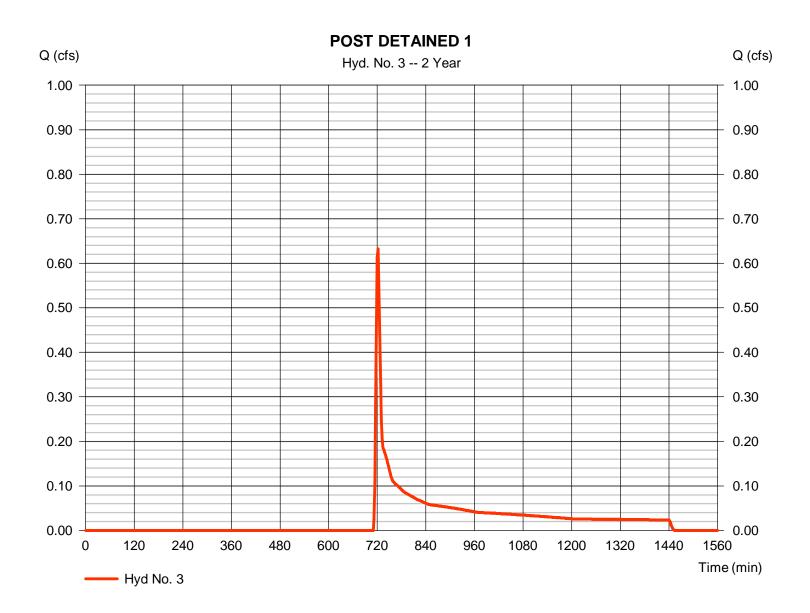
Tuesday, 11 / 1 / 2016

Hyd. No. 3

POST DETAINED 1

Hydrograph type = SCS Runoff Peak discharge = 0.633 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 2 min Hyd. volume = 2.323 cuftDrainage area Curve number = 1.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.70 \, \text{min}$ Total precip. = 3.22 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.140 \times 85) + (0.460 \times 58) + (1.240 \times 55)] / 1.840$



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Hyd. No. 3POST DETAINED 1

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 3.22 = 4.00	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	6.19	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 621.00 = 6.60 = Unpaved =4.15	d	94.00 5.30 Paved 4.68		80.00 11.30 Unpave 5.42	d		
Travel Time (min)	= 2.50	+	0.33	+	0.25	=	3.08	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})209.0		0.0		0.0			
Travel Time (min)	= 0.46	+	0.00	+	0.00	=	0.46	
Total Travel Time, Tc							9.70 min	

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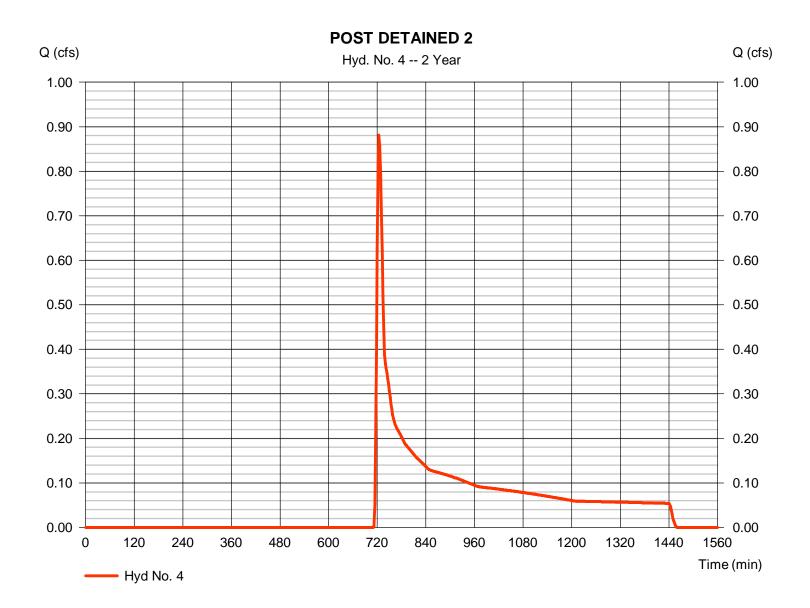
Tuesday, 11 / 1 / 2016

Hyd. No. 4

POST DETAINED 2

Hydrograph type = SCS Runoff Peak discharge = 0.881 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 2 min Hyd. volume = 4,915 cuftCurve number Drainage area = 4.600 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.00 \, \text{min}$ Total precip. = 3.22 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.040 \times 85) + (0.750 \times 58) + (3.810 \times 55)] / 4.600$



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4 $\,$

Hyd. No. 4POST DETAINED 2

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 3.22 = 4.00 = 6.19	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 784.00 = 6.00 = Unpaved =3.95	d	110.00 13.60 Paved 7.50		72.00 12.50 Unpave 5.70	ed	
Travel Time (min)	= 3.31	+	0.24	+	0.21	=	3.76
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.030 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							10.00 min

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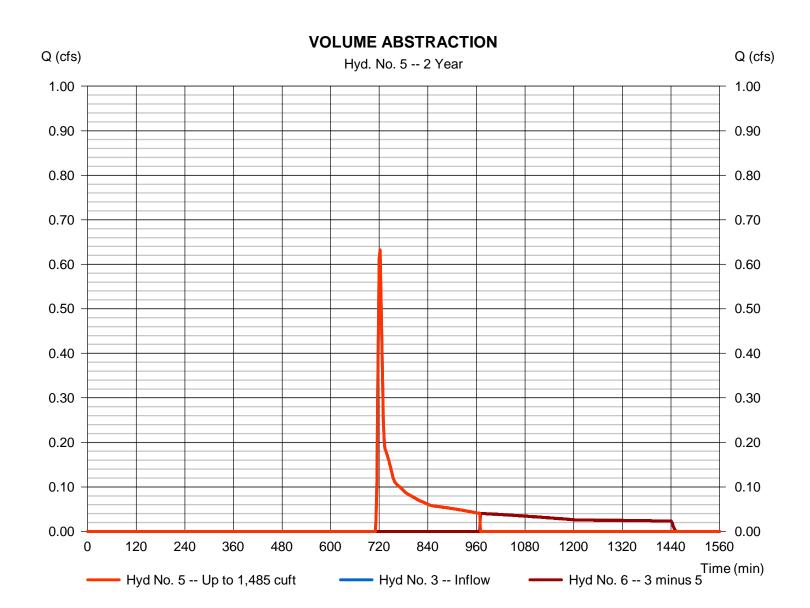
Hyd. No. 5

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 0.633 cfsStorm frequency= 2 yrsTime to peak= 722 minTime interval= 2 minHyd. volume= 1,489 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 6

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



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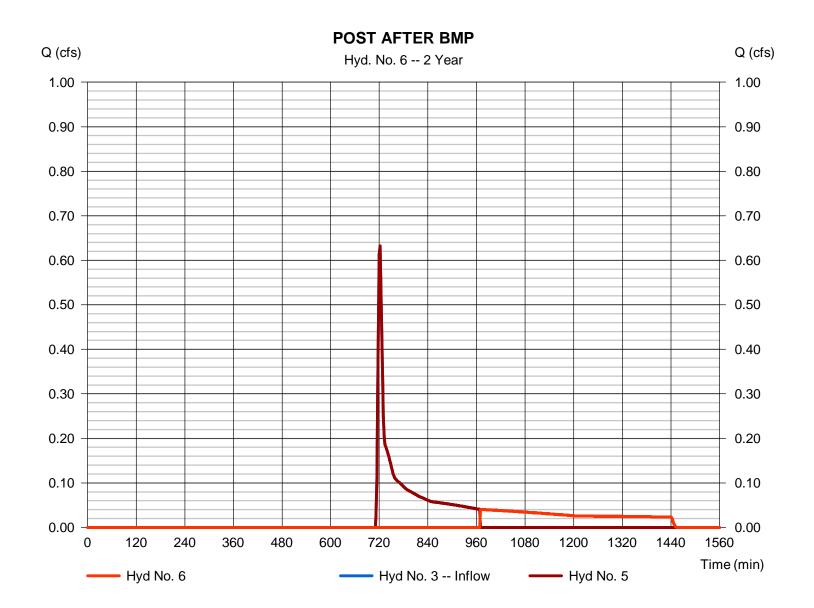
Hyd. No. 6

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 0.040 cfsStorm frequency= 2 yrsTime to peak= 970 minTime interval= 2 minHyd. volume= 834 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 5

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

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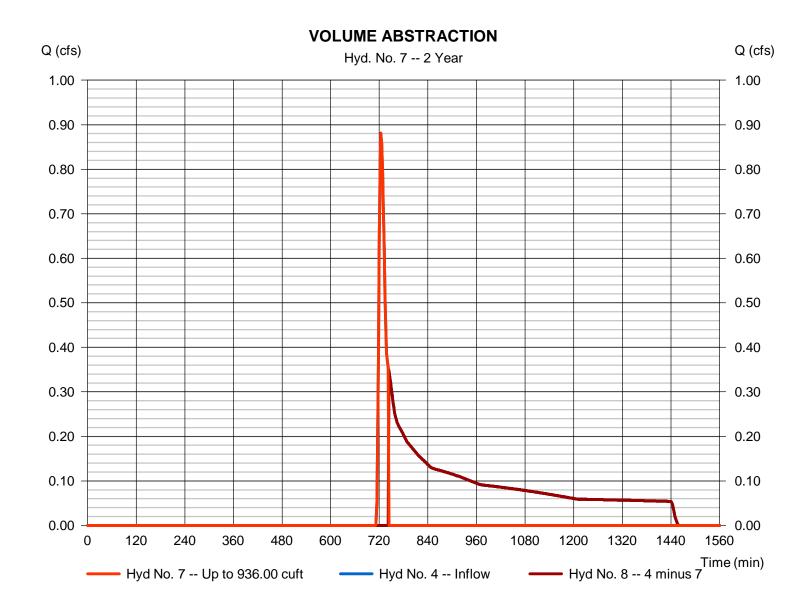
Hyd. No. 7

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 0.881 cfsStorm frequency= 2 yrsTime to peak= 724 minTime interval= 2 minHyd. volume= 960 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 8

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



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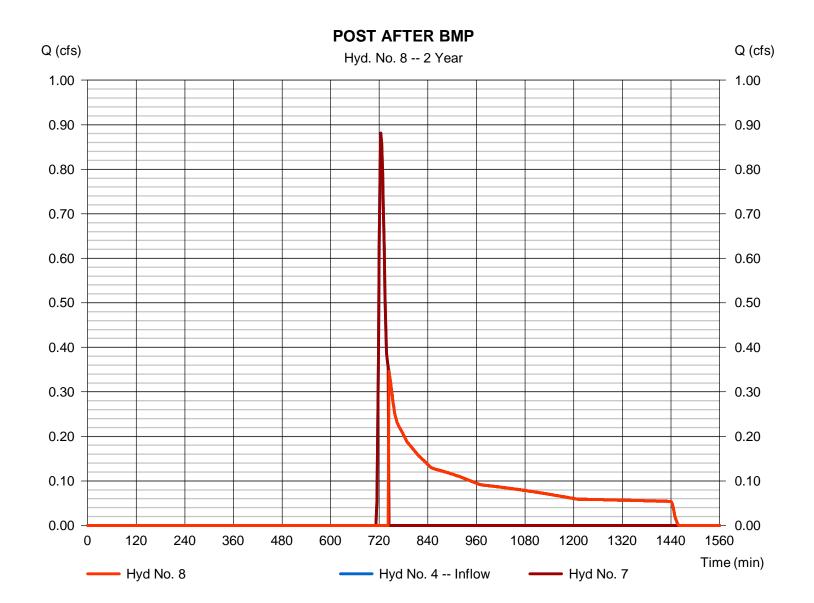
Hyd. No. 8

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 0.346 cfsStorm frequency= 2 yrsTime to peak= 744 minTime interval= 2 minHyd. volume= 3,955 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 7

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



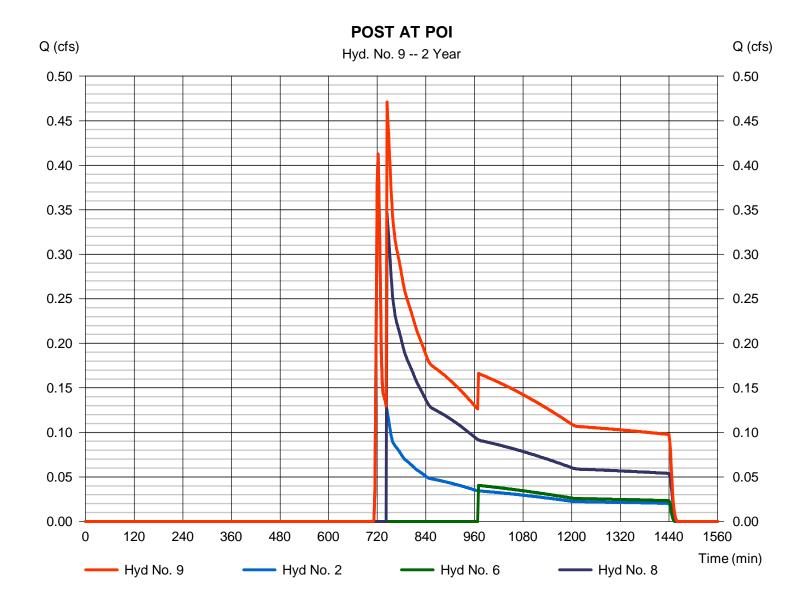
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Tuesday, 11 / 1 / 2016

Hyd. No. 9

POST AT POI

Hydrograph type = Combine Peak discharge = 0.471 cfsStorm frequency = 2 yrsTime to peak = 744 min Time interval = 2 min Hyd. volume = 6,644 cuftInflow hyds. Contrib. drain. area = 2, 6, 8= 1.790 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	9.557	2	720	25,212				PRE
2	SCS Runoff	2.274	2	720	5,853				POST UNDETAINED
3	SCS Runoff	2.744	2	720	6,801				POST DETAINED 1
4	SCS Runoff	5.064	2	722	15,510				POST DETAINED 2
5	Diversion1	2.744	2	720	1,633	3			VOLUME ABSTRACTION
6	Diversion2	2.028	2	724	5,167	3			POST AFTER BMP
7	Diversion1	3.912	2	718	1,293	4			VOLUME ABSTRACTION
8	Diversion2	5.064	2	722	14,217	4			POST AFTER BMP
Mo	rgantown.gpv	v			Return I	Period: 10 \	⁄ear	Tuesday, 1	1 / 1 / 2016

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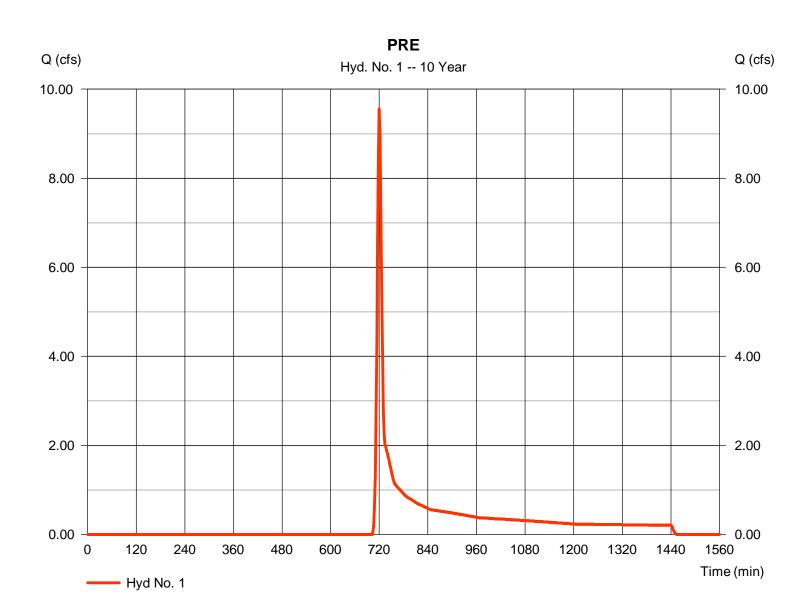
Tuesday, 11 / 1 / 2016

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 9.557 cfsStorm frequency Time to peak = 10 yrs= 720 min Time interval = 2 min Hyd. volume = 25.212 cuftDrainage area Curve number = 8.230 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 4.72 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(7.780 \times 55) + (0.450 \times 58)] / 8.230$



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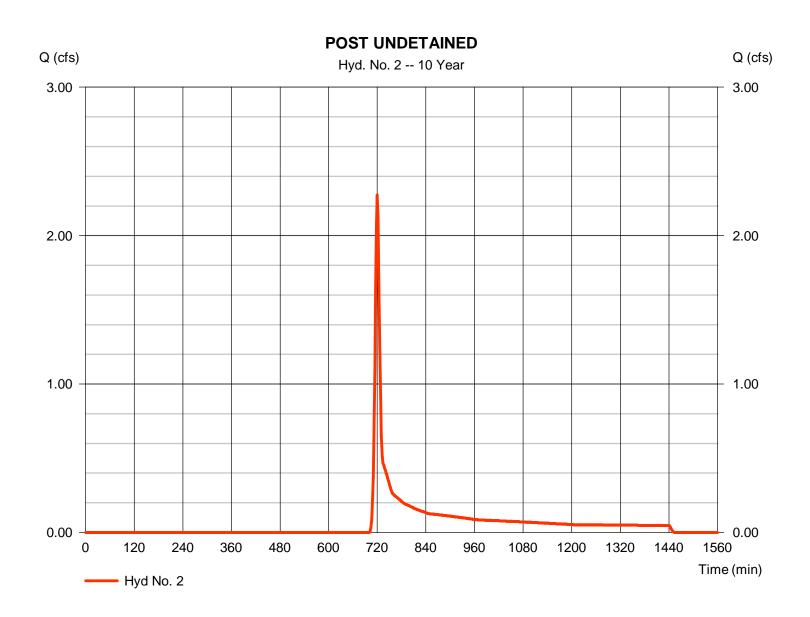
Tuesday, 11 / 1 / 2016

Hyd. No. 2

POST UNDETAINED

= SCS Runoff Hydrograph type Peak discharge = 2.274 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 2 min Hyd. volume = 5.853 cuftCurve number Drainage area = 1.790 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 4.72 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.030 \times 85) + (0.430 \times 58) + (1.330 \times 55)] / 1.790$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

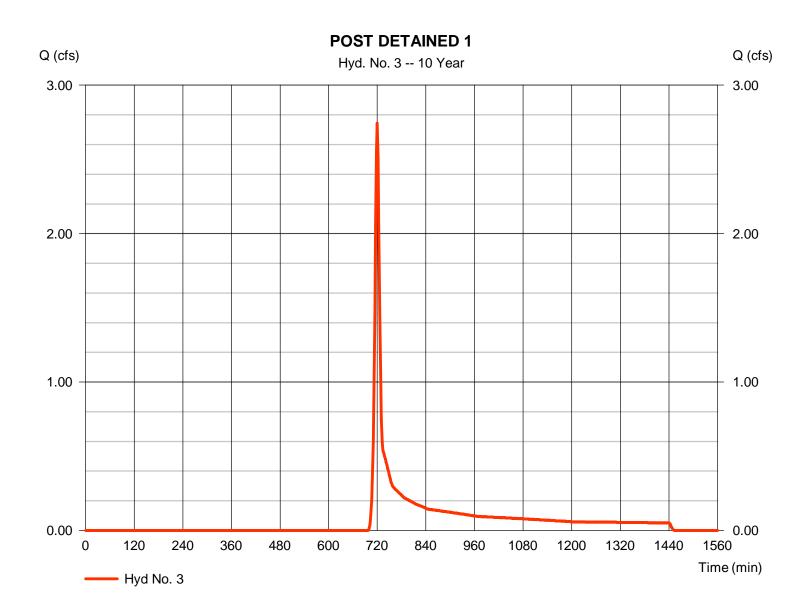
Tuesday, 11 / 1 / 2016

Hyd. No. 3

POST DETAINED 1

= SCS Runoff Hydrograph type Peak discharge = 2.744 cfs= 720 min Storm frequency Time to peak = 10 yrsTime interval = 2 min Hyd. volume = 6.801 cuftCurve number Drainage area = 1.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.70 \, \text{min}$ Total precip. = 4.72 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 85) + (0.460 \times 58) + (1.240 \times 55)] / 1.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

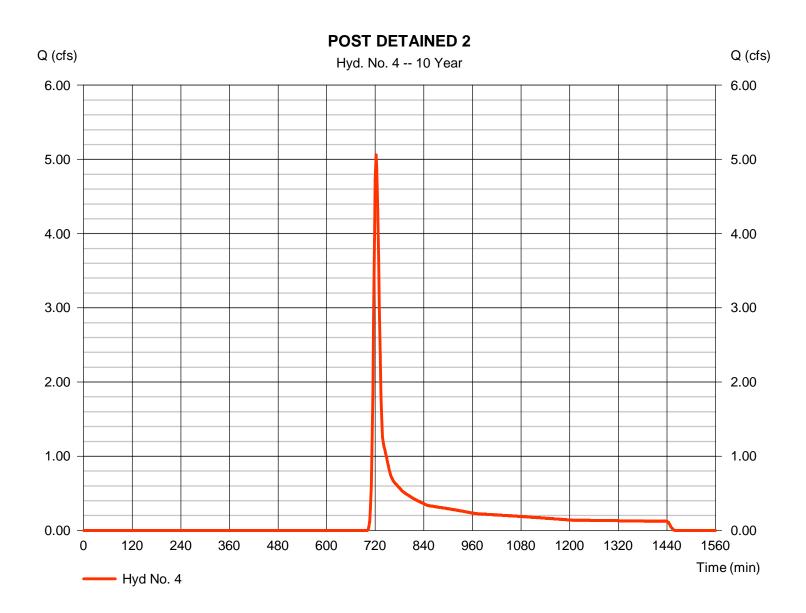
Tuesday, 11 / 1 / 2016

Hyd. No. 4

POST DETAINED 2

Hydrograph type = SCS Runoff Peak discharge = 5.064 cfsStorm frequency Time to peak = 722 min = 10 yrsTime interval = 2 min Hyd. volume = 15.510 cuft Curve number Drainage area = 4.600 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.00 \, \text{min}$ Total precip. = 4.72 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.040 \times 85) + (0.750 \times 58) + (3.810 \times 55)] / 4.600$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

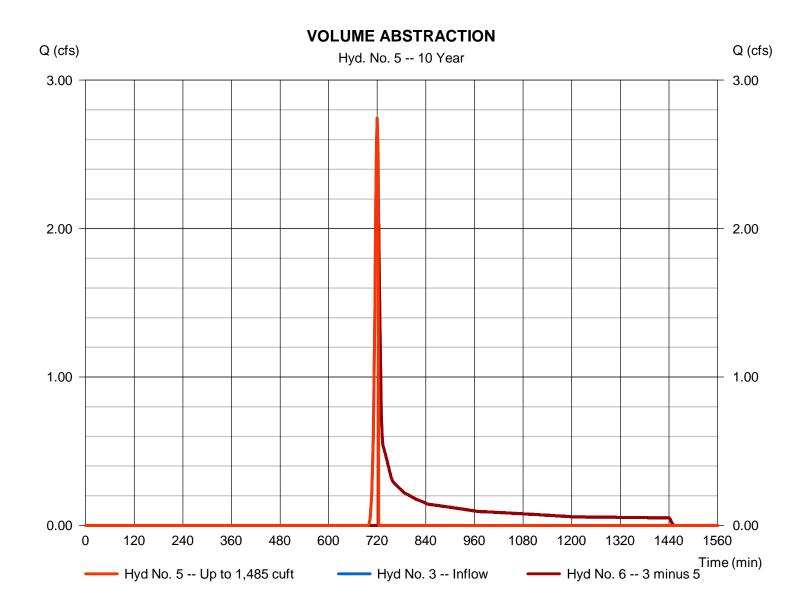
Hyd. No. 5

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 2.744 cfsStorm frequency= 10 yrsTime to peak= 720 minTime interval= 2 minHyd. volume= 1,633 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 6

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



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Tuesday, 11 / 1 / 2016

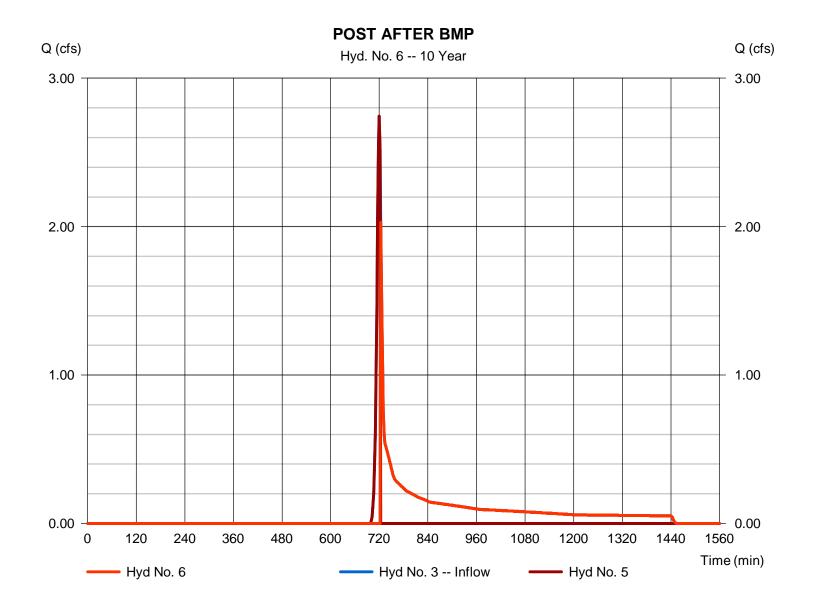
Hyd. No. 6

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 2.028 cfsStorm frequency= 10 yrsTime to peak= 724 minTime interval= 2 minHyd. volume= 5,167 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 5

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

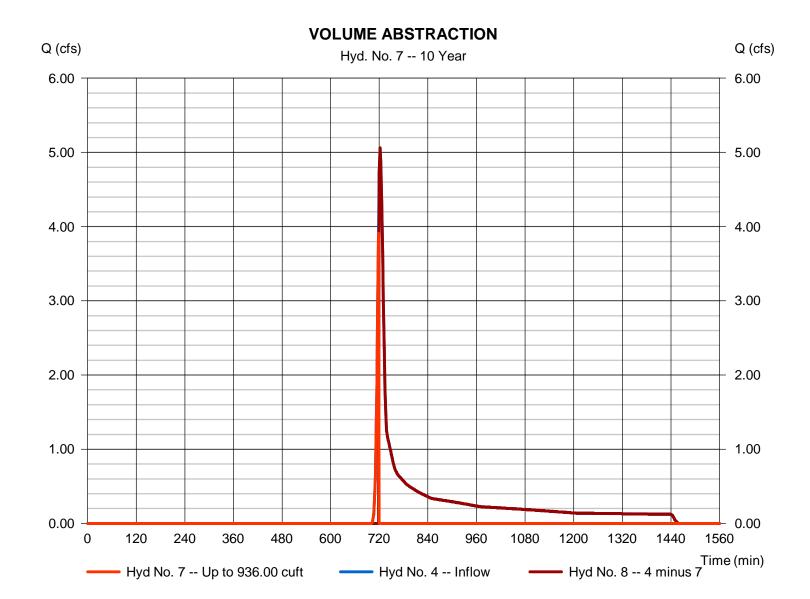
Hyd. No. 7

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 3.912 cfsStorm frequency= 10 yrsTime to peak= 718 minTime interval= 2 minHyd. volume= 1,293 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 8

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

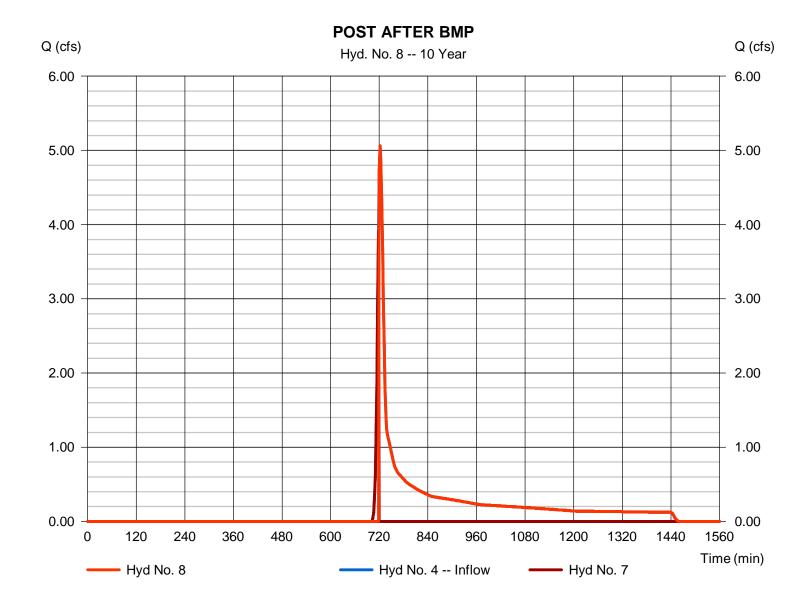
Hyd. No. 8

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 5.064 cfsStorm frequency= 10 yrsTime to peak= 722 minTime interval= 2 minHyd. volume= 14,217 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 7

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



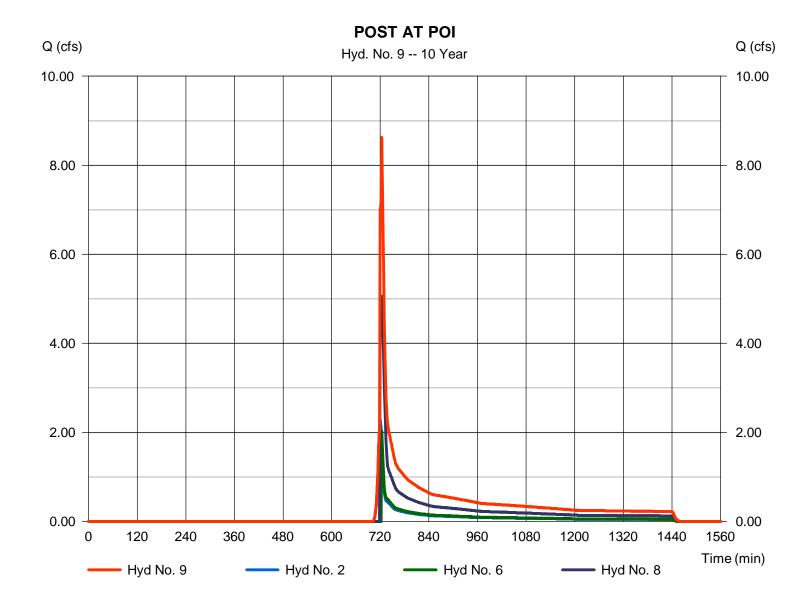
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Hyd. No. 9

POST AT POI

Hydrograph type = Combine Peak discharge = 8.625 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 2 min Hyd. volume = 25,238 cuftInflow hyds. Contrib. drain. area = 2, 6, 8= 1.790 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	23.26	2	720	54,716				PRE
2	SCS Runoff	5.332	2	720	12,466				POST UNDETAINED
3	SCS Runoff	6.042	2	720	13,997				POST DETAINED 1
4	SCS Runoff	12.18	2	722	33,037				POST DETAINED 2
5	Diversion1	3.874	2	714	1,798	3			VOLUME ABSTRACTION
6	Diversion2	6.042	2	720	12,198	3			POST AFTER BMP
7	Diversion1	2.351	2	708	1,002	4			VOLUME ABSTRACTION
8	Diversion2	12.18	2	722	32,035	4			POST AFTER BMP
Mo	rgantown.gpv	V			Return	Period: 50 \	Year	Tuesday. 1	11 / 1 / 2016

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

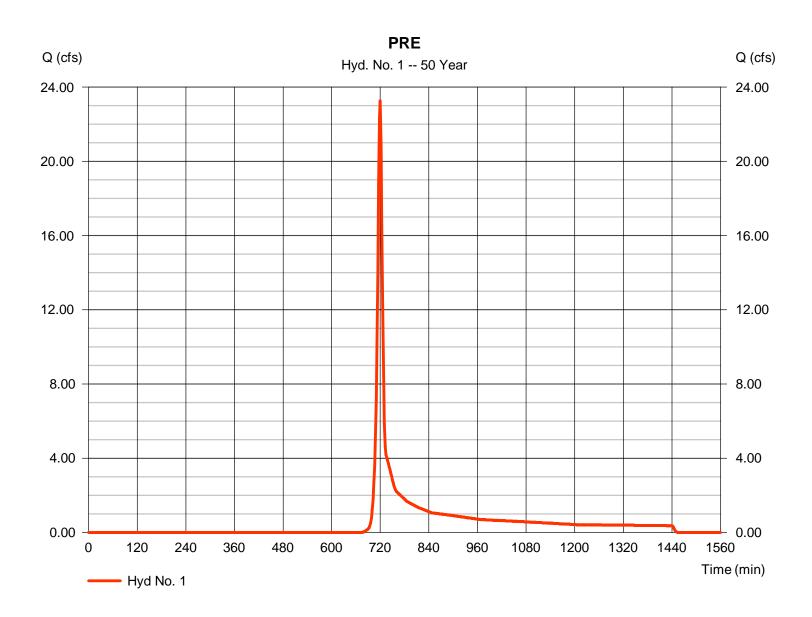
Tuesday, 11 / 1 / 2016

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 23.26 cfs= 720 min Storm frequency Time to peak = 50 yrsTime interval = 2 min Hyd. volume = 54.716 cuft Curve number Drainage area = 8.230 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 6.53 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(7.780 \times 55) + (0.450 \times 58)] / 8.230$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

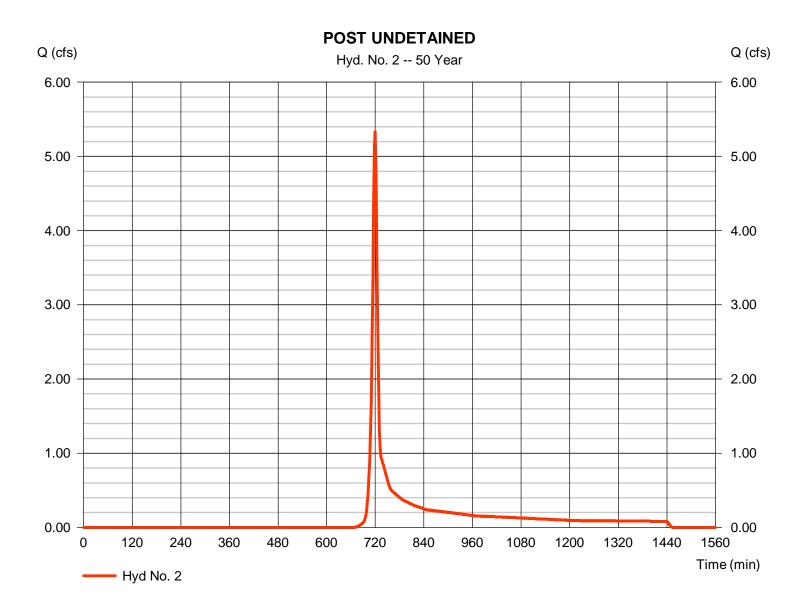
Tuesday, 11 / 1 / 2016

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 5.332 cfsStorm frequency Time to peak = 50 yrs= 720 min Time interval = 2 min Hyd. volume = 12.466 cuft Curve number Drainage area = 1.790 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 6.53 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.030 \times 85) + (0.430 \times 58) + (1.330 \times 55)] / 1.790$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

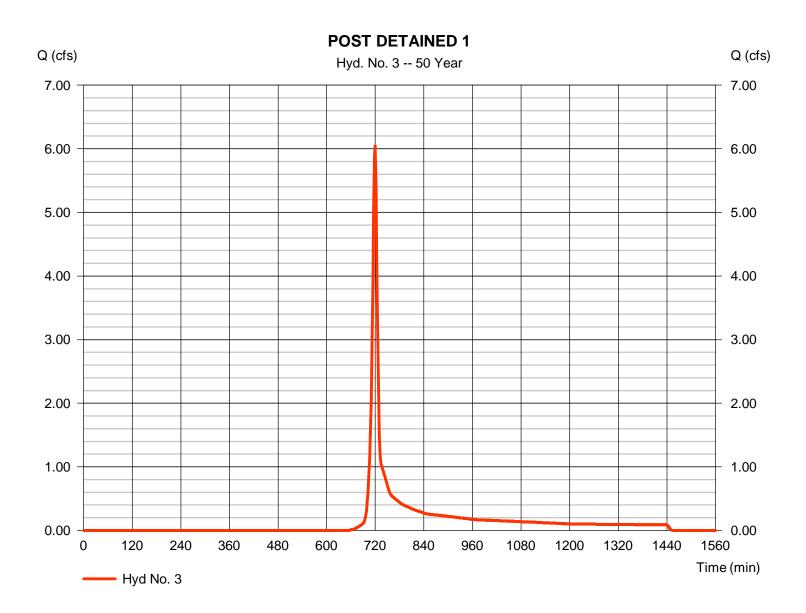
Tuesday, 11 / 1 / 2016

Hyd. No. 3

POST DETAINED 1

Hydrograph type = SCS Runoff Peak discharge = 6.042 cfsStorm frequency Time to peak = 720 min = 50 yrsTime interval = 2 min Hyd. volume = 13.997 cuft Curve number Drainage area = 1.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.70 \, \text{min}$ Total precip. = 6.53 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 85) + (0.460 \times 58) + (1.240 \times 55)] / 1.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

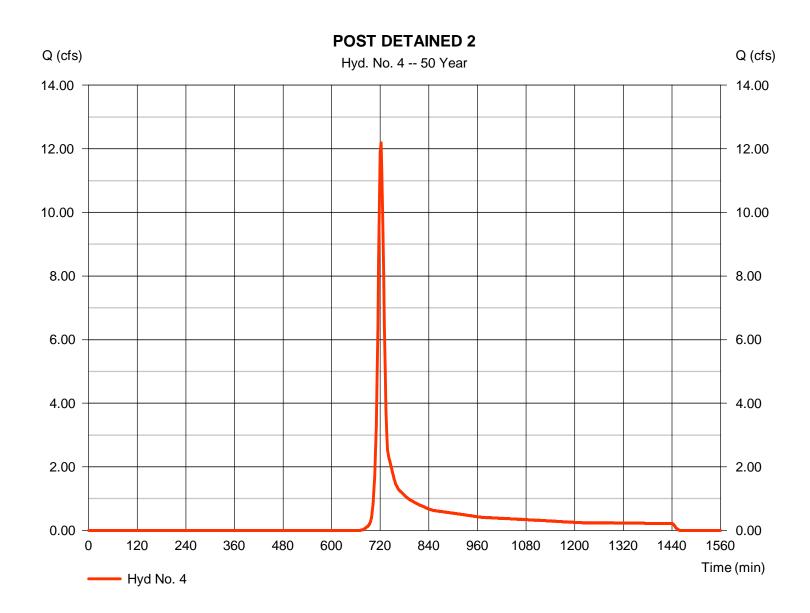
Tuesday, 11 / 1 / 2016

Hyd. No. 4

POST DETAINED 2

Hydrograph type = SCS Runoff Peak discharge = 12.18 cfsStorm frequency Time to peak = 722 min = 50 yrsTime interval = 2 min Hyd. volume = 33.037 cuft Drainage area Curve number = 4.600 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.00 \, \text{min}$ Total precip. = 6.53 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.040 \times 85) + (0.750 \times 58) + (3.810 \times 55)] / 4.600$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

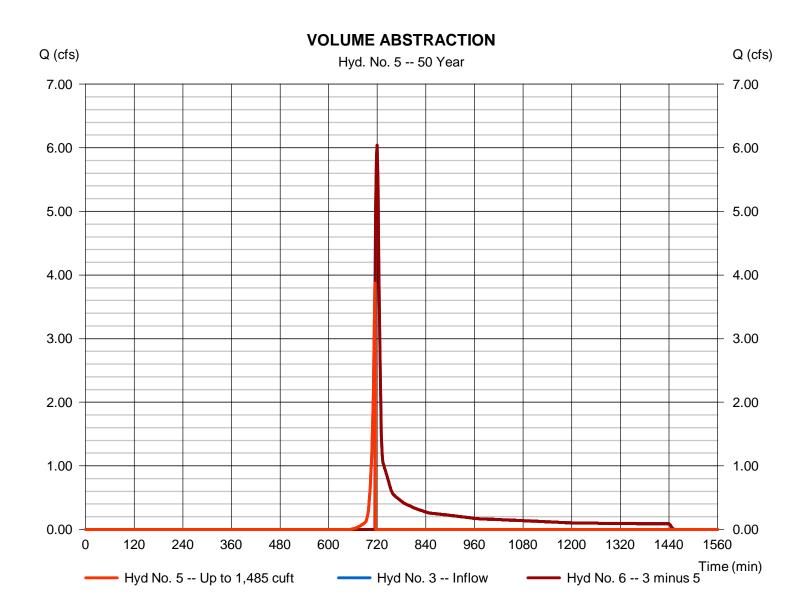
Hyd. No. 5

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 3.874 cfsStorm frequency= 50 yrsTime to peak= 714 minTime interval= 2 minHyd. volume= 1,798 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 6

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

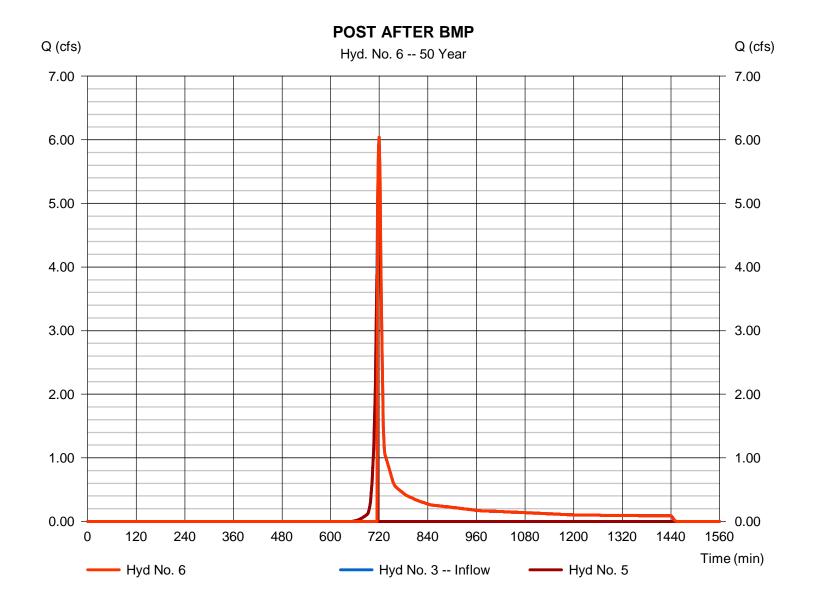
Hyd. No. 6

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 6.042 cfsStorm frequency= 50 yrsTime to peak= 720 minTime interval= 2 minHyd. volume= 12,198 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 5

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

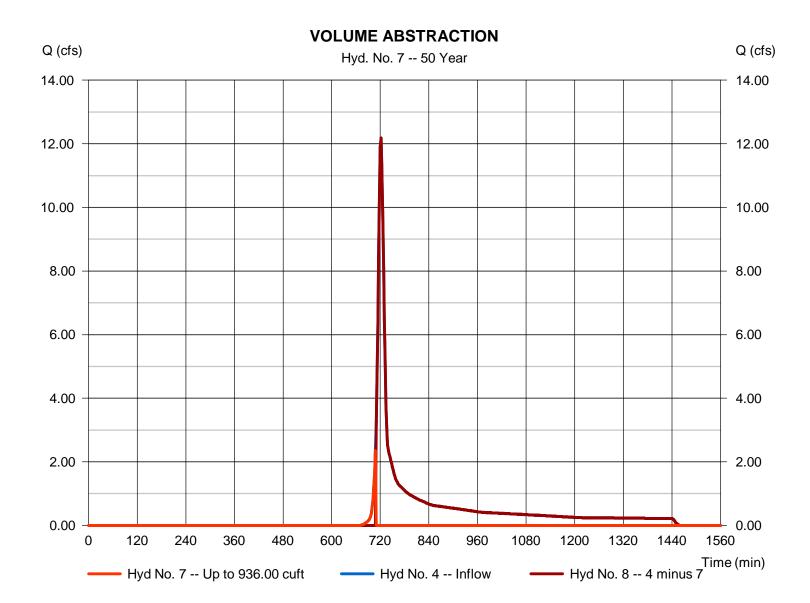
Hyd. No. 7

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 2.351 cfsStorm frequency= 50 yrsTime to peak= 708 minTime interval= 2 minHyd. volume= 1,002 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 8

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

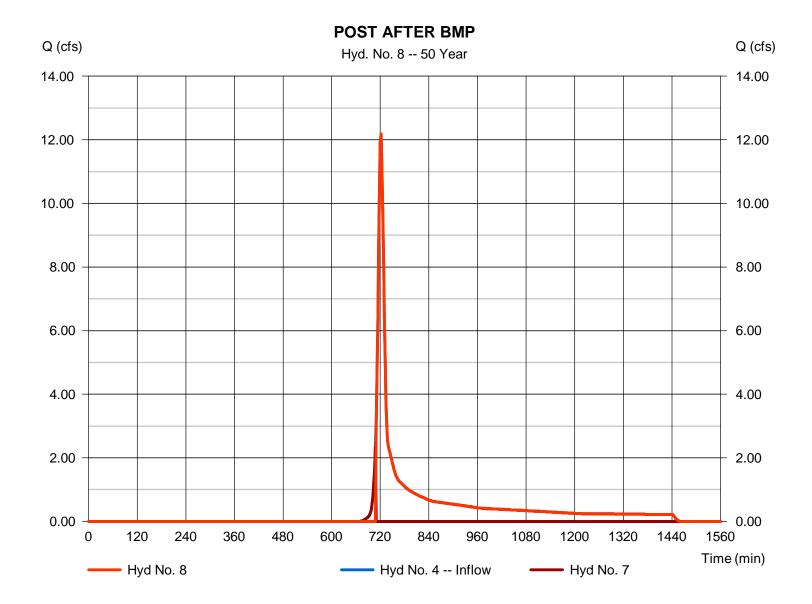
Hyd. No. 8

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 12.18 cfsStorm frequency= 50 yrsTime to peak= 722 minTime interval= 2 minHyd. volume= 32,035 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 7

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



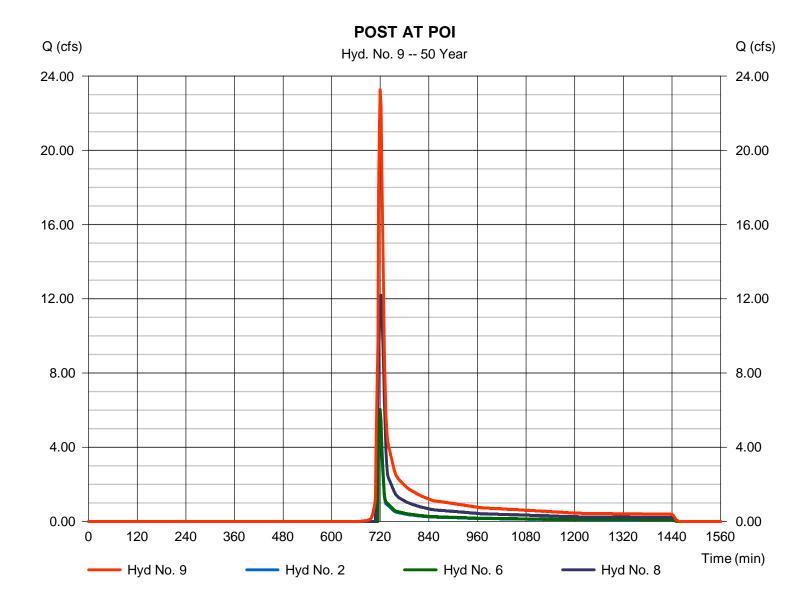
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Hyd. No. 9

POST AT POI

Hydrograph type = Combine Peak discharge = 23.27 cfsStorm frequency = 50 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 56,699 cuftInflow hyds. Contrib. drain. area = 1.790 ac= 2, 6, 8



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	30.82	2	720	71,361				PRE
2	SCS Runoff	7.008	2	720	16,173				POST UNDETAINED
3	SCS Runoff	7.828	2	720	17,980				POST DETAINED 1
4	SCS Runoff	16.10	2	722	42,860				POST DETAINED 2
5	Diversion1	2.915	2	710	1,625	3			VOLUME ABSTRACTION
6	Diversion2	7.828	2	720	16,355	3			POST AFTER BMP
7	Diversion1	1.491	2	702	942	4			VOLUME ABSTRACTION
8	Diversion2	16.10	2	722	41,918	4			POST AFTER BMP
Mo	rgantown.gpv	V			Return I	Period: 100	Year	Tuesday, 1	1 / 1 / 2016

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

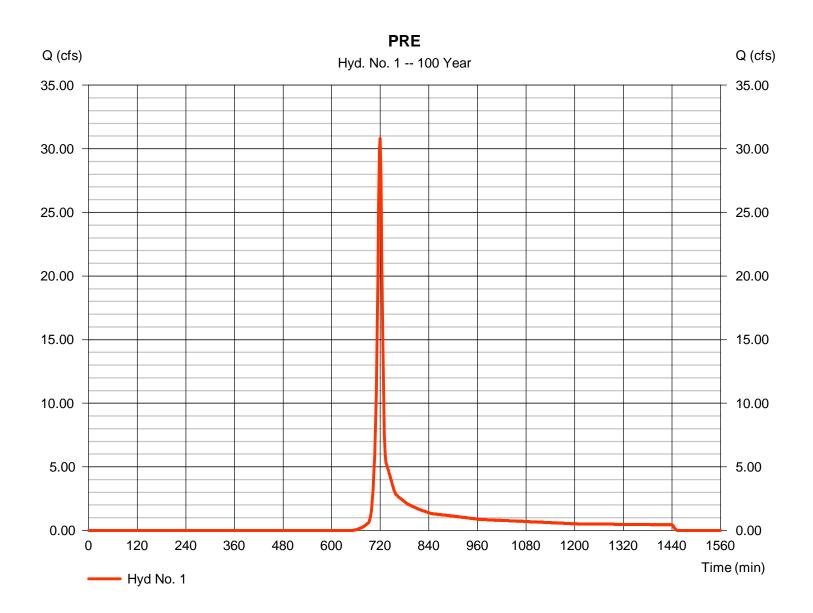
Tuesday, 11 / 1 / 2016

Hyd. No. 1

PRE

Hydrograph type = SCS Runoff Peak discharge = 30.82 cfsStorm frequency Time to peak = 720 min = 100 yrsTime interval = 2 min Hyd. volume = 71,361 cuftCurve number Drainage area = 8.230 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 7.41 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(7.780 x 55) + (0.450 x 58)] / 8.230



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

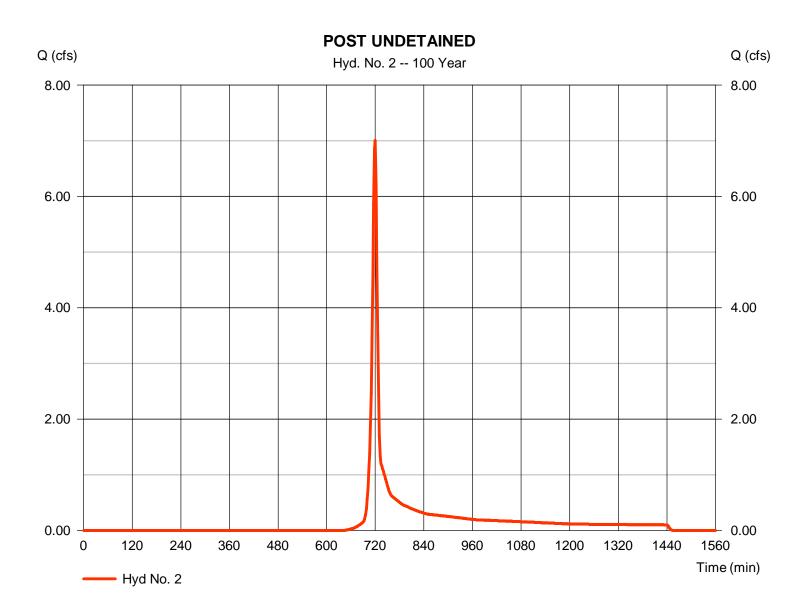
Tuesday, 11 / 1 / 2016

Hyd. No. 2

POST UNDETAINED

= 7.008 cfsHydrograph type = SCS Runoff Peak discharge = 720 min Storm frequency Time to peak = 100 yrsTime interval = 2 min Hyd. volume = 16.173 cuft Curve number Drainage area = 1.790 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 7.41 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.030 \times 85) + (0.430 \times 58) + (1.330 \times 55)] / 1.790$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

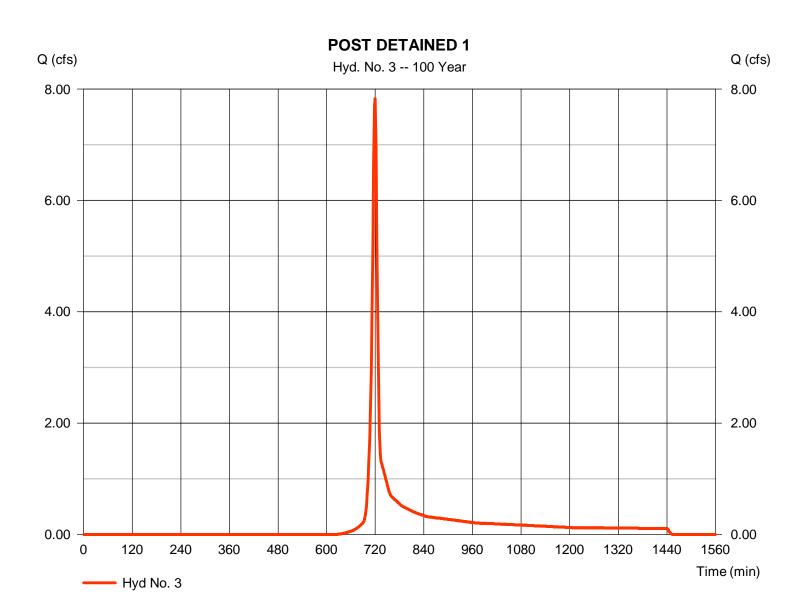
Tuesday, 11 / 1 / 2016

Hyd. No. 3

POST DETAINED 1

= SCS Runoff Hydrograph type Peak discharge = 7.828 cfsStorm frequency Time to peak = 720 min = 100 yrsTime interval = 2 min Hyd. volume = 17.980 cuftCurve number Drainage area = 1.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.70 \, \text{min}$ Total precip. = 7.41 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 85) + (0.460 \times 58) + (1.240 \times 55)] / 1.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

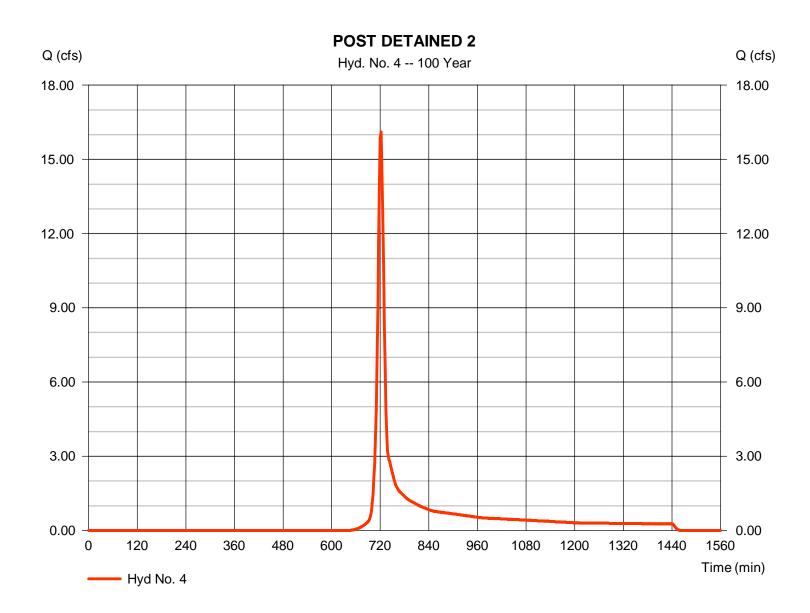
Tuesday, 11 / 1 / 2016

Hyd. No. 4

POST DETAINED 2

Hydrograph type = SCS Runoff Peak discharge = 16.10 cfsStorm frequency Time to peak = 722 min = 100 yrsTime interval = 2 min Hyd. volume = 42.860 cuftDrainage area Curve number = 4.600 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.00 \, \text{min}$ Total precip. = 7.41 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.040 \times 85) + (0.750 \times 58) + (3.810 \times 55)] / 4.600$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

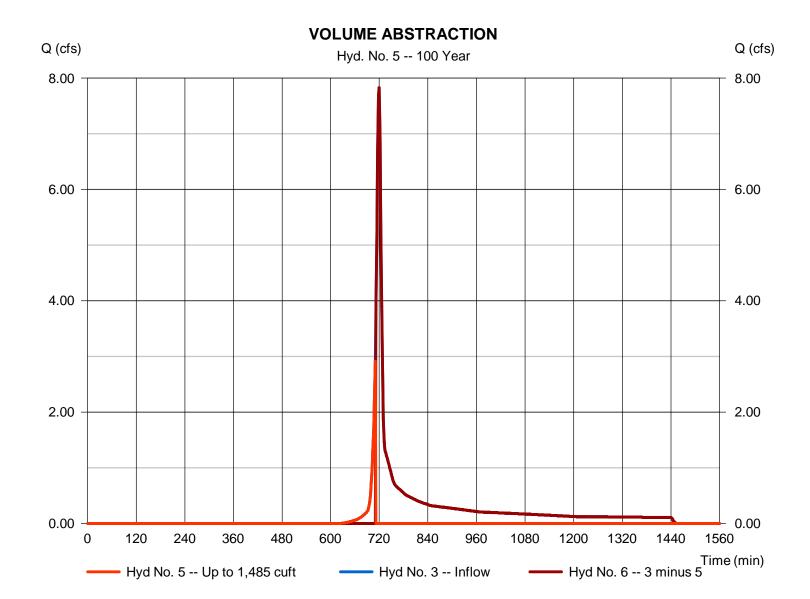
Hyd. No. 5

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 2.915 cfsStorm frequency= 100 yrsTime to peak= 710 minTime interval= 2 minHyd. volume= 1,625 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 6

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Hyd. No. 6

POST AFTER BMP

Hydrograph type = Diversion2 Storm frequency = 100 yrs Time interval = 2 min

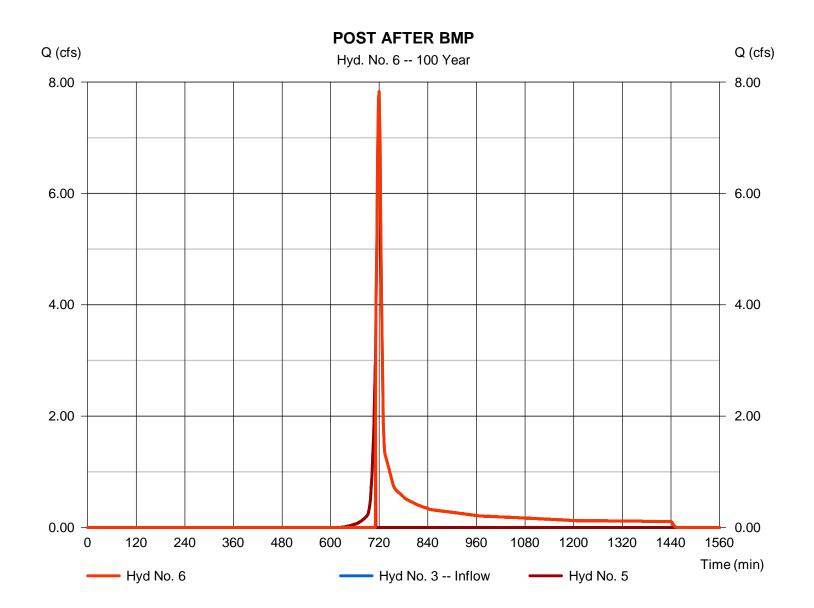
Inflow hydrograph = 3 - POST DETAINED 1

Diversion method = First Flush Volume

Peak discharge = 7.828 cfs Time to peak = 720 min Hyd. volume = 16,355 cuft

2nd diverted hyd. = 5

Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

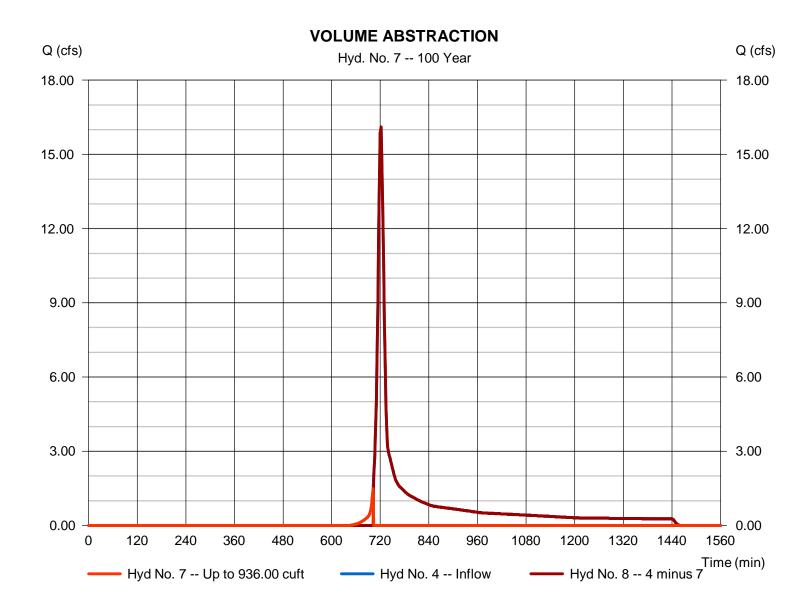
Hyd. No. 7

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 1.491 cfsStorm frequency= 100 yrsTime to peak= 702 minTime interval= 2 minHyd. volume= 942 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 8

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

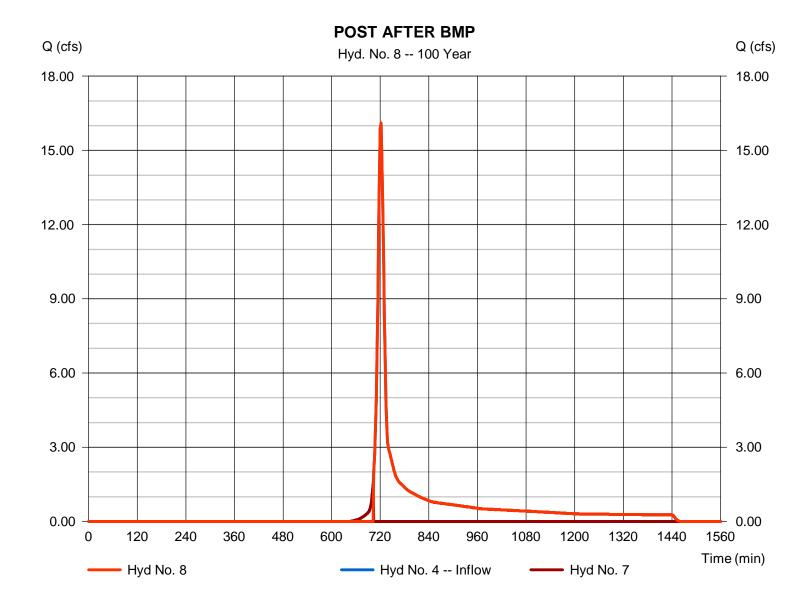
Hyd. No. 8

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 16.10 cfsStorm frequency= 100 yrsTime to peak= 722 minTime interval= 2 minHyd. volume= 41,918 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 7

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



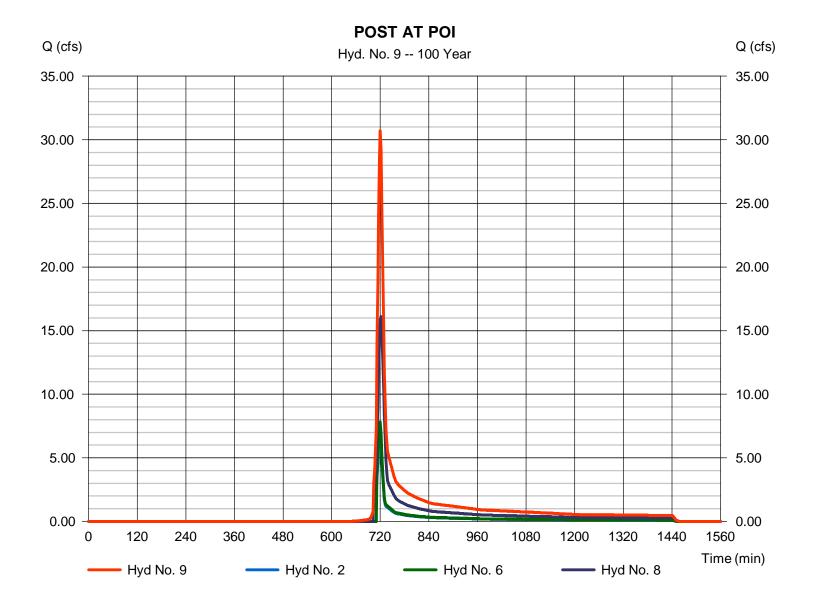
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Hyd. No. 9

POST AT POI

Hydrograph type = Combine Peak discharge = 30.70 cfsStorm frequency = 100 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 74,446 cuft Inflow hyds. Contrib. drain. area = 1.790 ac= 2, 6, 8



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)											
(Yrs)	В	D	E	(N/A)								
1	51.6351	12.5000	0.8881									
2	53.0270	11.7000	0.8506									
3	0.0000	0.0000	0.0000									
5	61.9255	12.6000	0.8350									
10	65.3595	12.8000	0.8167									
25	51.9840	10.7000	0.7342									
50	50.0341	10.3000	0.7046									
100	42.0647	8.6000	0.6468									

File name: Morgantown IDF.IDF

Intensity = $B/(Tc + D)^E$

Return Period (Yrs)		Intensity Values (in/hr)														
	5 min	10	15	20	25	30	35	40	45	50	55	60				
1	4.06	3.25	2.72	2.35	2.07	1.85	1.67	1.53	1.41	1.31	1.23	1.15				
2	4.84	3.87	3.24	2.80	2.47	2.22	2.02	1.85	1.71	1.59	1.49	1.40				
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
5	5.65	4.58	3.88	3.38	3.00	2.70	2.46	2.26	2.10	1.96	1.84	1.73				
10	6.22	5.08	4.32	3.78	3.36	3.04	2.78	2.56	2.38	2.22	2.09	1.97				
25	6.88	5.62	4.79	4.21	3.77	3.42	3.14	2.91	2.72	2.55	2.41	2.28				
50	7.32	6.00	5.14	4.52	4.06	3.70	3.41	3.17	2.96	2.79	2.63	2.50				
100	7.78	6.35	5.44	4.81	4.33	3.96	3.66	3.41	3.20	3.02	2.87	2.73				

Tc = time in minutes. Values may exceed 60.

P-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Morgantown Road\Hydraflow Rev 1\Morgantown Precip.pc

		F	tainfall F	Precipitat	tion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	2.68	3.22	0.00	4.04	4.72	5.70	6.53	7.41
SCS 6-Hr	1.88	2.27	0.00	2.83	3.28	3.92	4.43	4.98
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

7

8

9

Diversion1

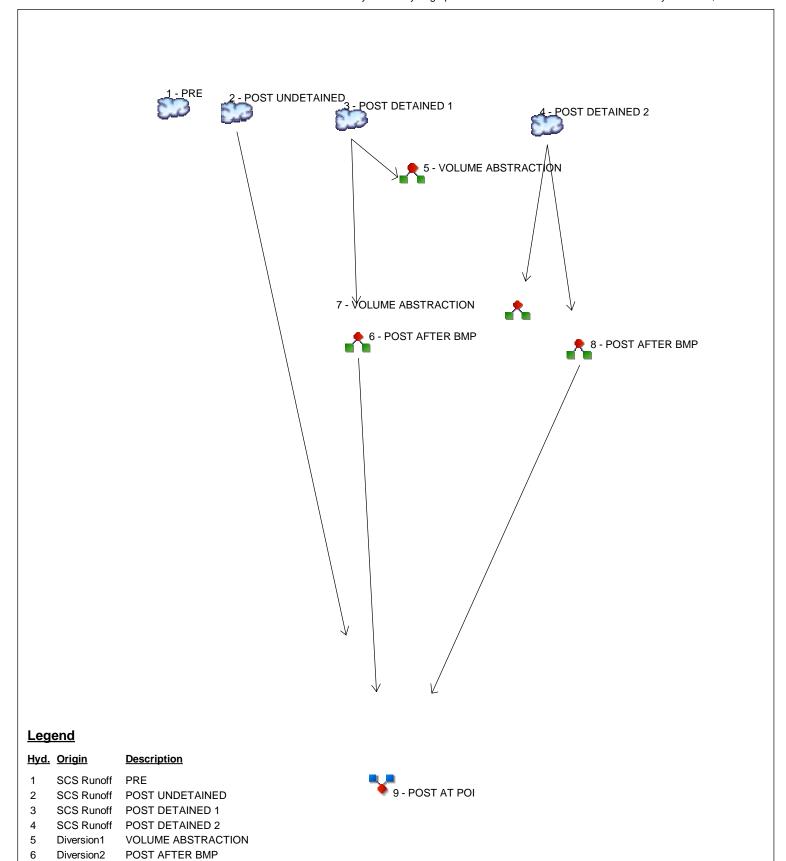
Diversion2

Combine

VOLUME ABSTRACTION

POST AFTER BMP

POST AT POI



Project: \\nuss010fp2\shared\Marcellus_M-Z - Mark Sladic\Sunoco Logistics (SXL)\\112I\cup05952sd\\\mathbf{M}\)ar\\mathbf{n}\)er\\mathbf{E}\text{a}\text{200P}\text{6}\text{ase 2}\\\mathbf{E}\text{SCG}\mathbf{P}-2\\\mathbf{F}\)

Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

yd. o.	Hydrograph	Inflow				Peak Out	tflow (cfs)	Peak Outflow (cfs)									
О.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description						
1	SCS Runoff			1.490							PRE						
2	SCS Runoff			0.413							POST UNDETAINED						
3	SCS Runoff			0.217							POST DETAINED 1						
4	SCS Runoff			0.512							POST DETAINED 2						
5	Diversion1	3		0.217							VOLUME ABSTRACTION						
6	Diversion2	3		0.041							POST AFTER BMP						
7	Diversion1	4		0.512							VOLUME ABSTRACTION						
3	Diversion2	4		0.364							POST AFTER BMP						
)	Combine	2, 6, 8		0.454							POST AT POI						

Proj. file: \\nuss010fp2\shared\Marcellus_M-Z - Mark Sladic\Sunoco Logistics (\$XIL)\est2\\C05958 /(201958 / (201958 - East Phase 2)\ESC\GP-2\

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.490	2	722	7,671				PRE
2	SCS Runoff	0.413	2	722	1,855				POST UNDETAINED
3	SCS Runoff	0.217	2	754	2,323				POST DETAINED 1
4	SCS Runoff	0.512	2	738	4,766				POST DETAINED 2
5	Diversion1	0.217	2	754	1,490	3			VOLUME ABSTRACTION
6	Diversion2	0.041	2	1000	833	3			POST AFTER BMP
7	Diversion1	0.512	2	738	948	4			VOLUME ABSTRACTION
8	Diversion2	0.364	2	758	3,819	4			POST AFTER BMP
9	Combine	0.454	2	758	6,506	2, 6, 8			POST AT POI

\\nuss010fp2\shared\Marcellus_M-Z - Mark \$laket\@unkeriooblo@istear(SXL)\112ICU5@sea(Matinder/E20st@hase 2)\ESCGP-2\PPP\02.5

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

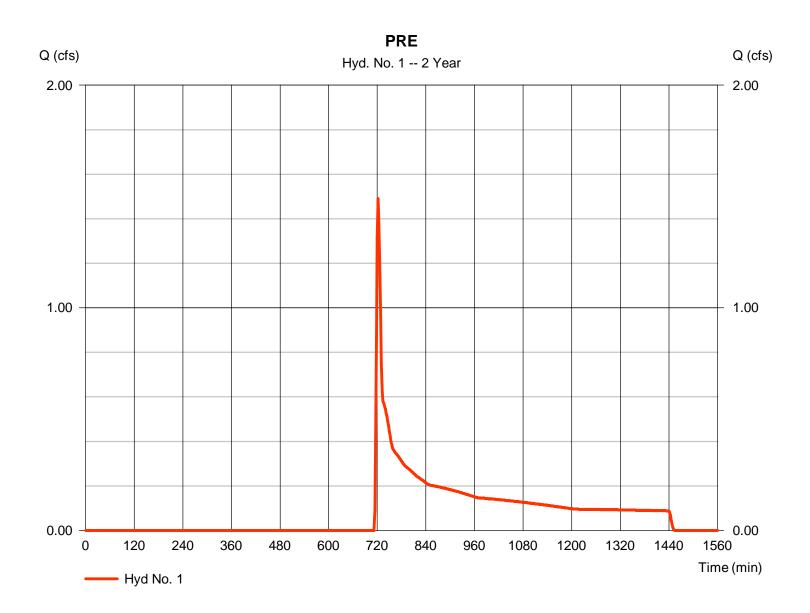
Tuesday, 11 / 1 / 2016

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 1.490 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 2 min Hyd. volume = 7,671 cuftDrainage area Curve number = 8.230 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 3.22 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(7.780 x 55) + (0.450 x 58)] / 8.230



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>	<u>B</u>		<u>C</u>		<u>Totals</u>						
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 3.22 = 4.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00							
Travel Time (min)	= 6.19	+	0.00	+	0.00	=	6.19					
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpave =5.18	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00							
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42					
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015							
Flow length (ft)	({0})71.0		0.0		0.0							
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16					
Total Travel Time, Tc	Total Travel Time, Tc											

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

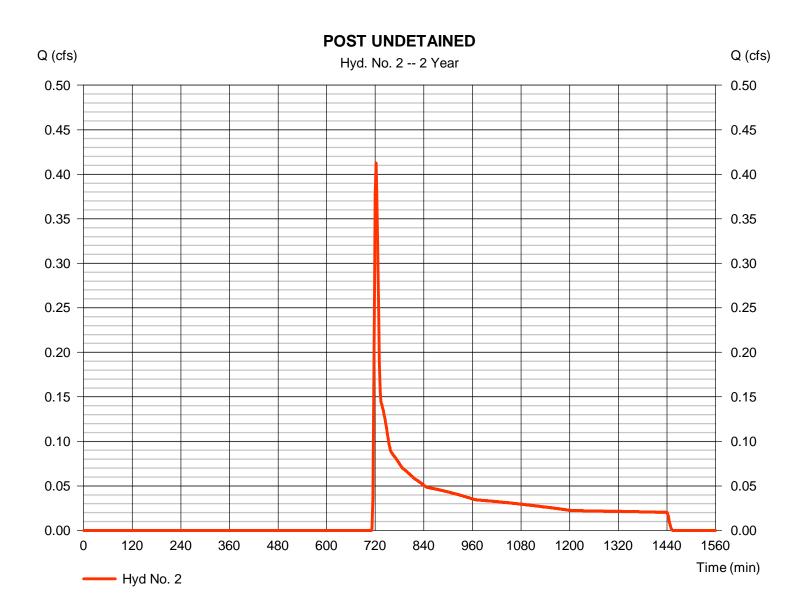
Tuesday, 11 / 1 / 2016

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 0.413 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 2 min Hyd. volume = 1,855 cuftDrainage area Curve number = 1.790 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 3.22 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.030 \times 85) + (0.430 \times 58) + (1.330 \times 55)] / 1.790$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 2POST UNDETAINED

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 3.22 = 4.00	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpaved =5.18	d	0.00 0.00 Unpave 0.00	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})71.0		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							8.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

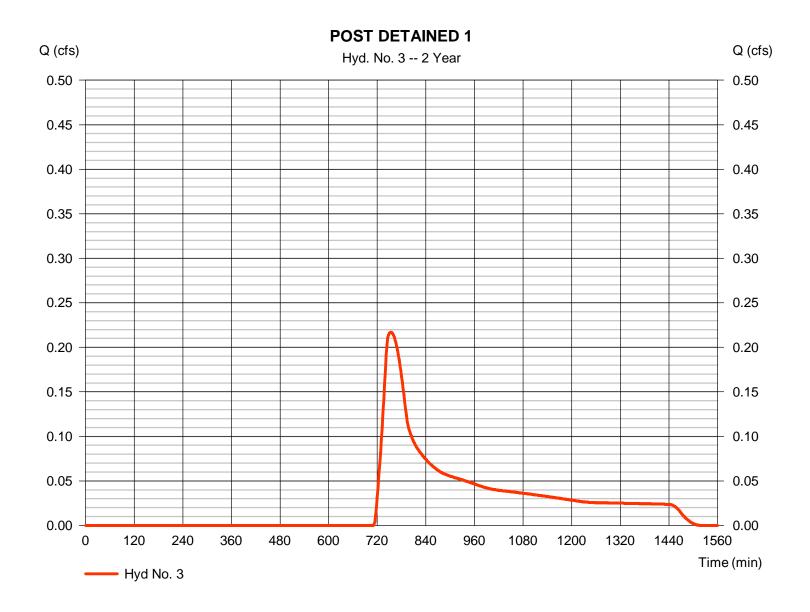
Tuesday, 11 / 1 / 2016

Hyd. No. 3

POST DETAINED 1

Hydrograph type = SCS Runoff Peak discharge = 0.217 cfsStorm frequency Time to peak = 2 yrs= 754 min Time interval = 2 min Hyd. volume = 2,323 cuftDrainage area Curve number = 1.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) = 48.80 min Total precip. = 3.22 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 85) + (0.460 \times 58) + (1.240 \times 55)] / 1.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

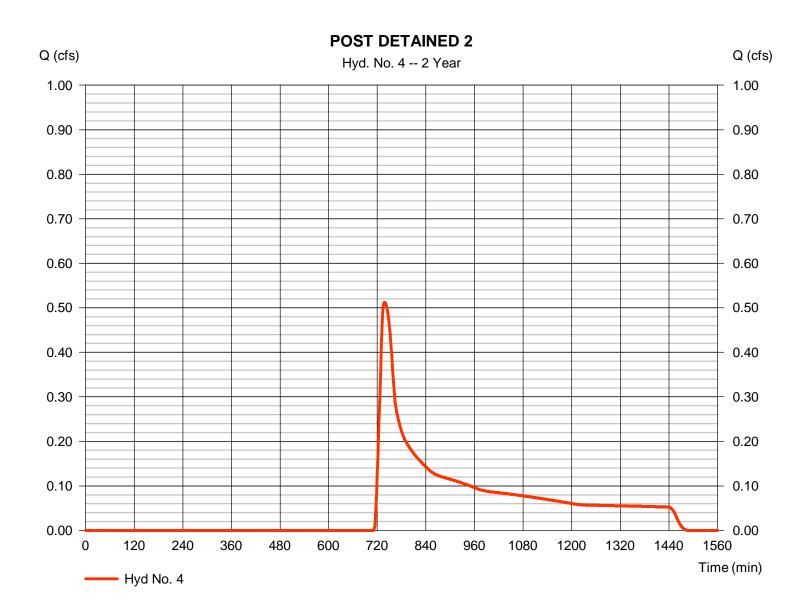
Tuesday, 11 / 1 / 2016

Hyd. No. 4

POST DETAINED 2

Hydrograph type = SCS Runoff Peak discharge = 0.512 cfsStorm frequency Time to peak = 738 min = 2 yrsTime interval = 2 min Hyd. volume = 4,766 cuftDrainage area Curve number = 4.600 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) = 27.70 min Total precip. = 3.22 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.040 \times 85) + (0.750 \times 58) + (3.810 \times 55)] / 4.600$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

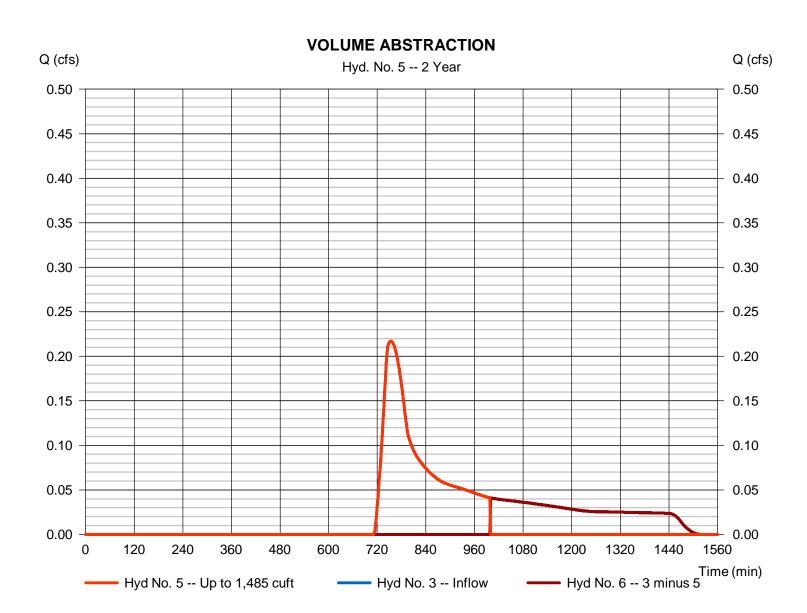
Hyd. No. 5

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 0.217 cfsStorm frequency= 2 yrsTime to peak= 754 minTime interval= 2 minHyd. volume= 1,490 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 6

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

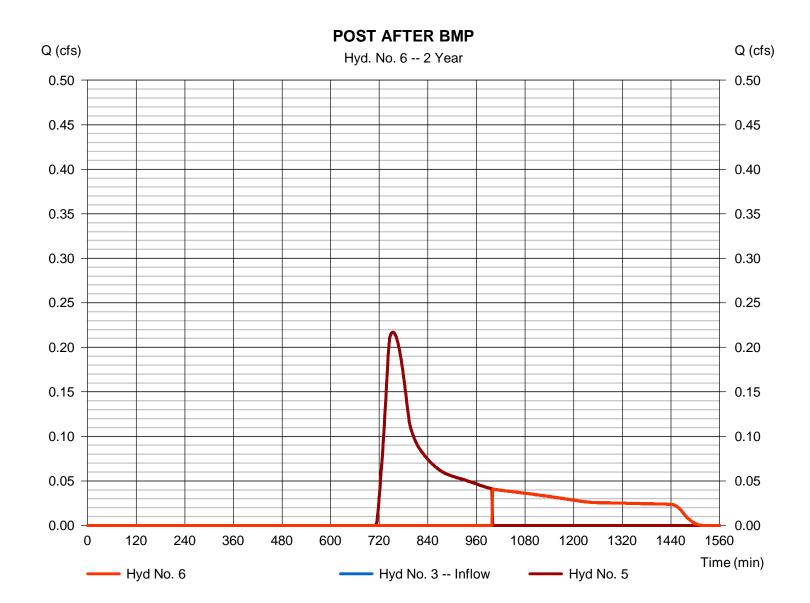
Hyd. No. 6

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 0.041 cfsStorm frequency= 2 yrsTime to peak= 1000 minTime interval= 2 minHyd. volume= 833 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 5

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

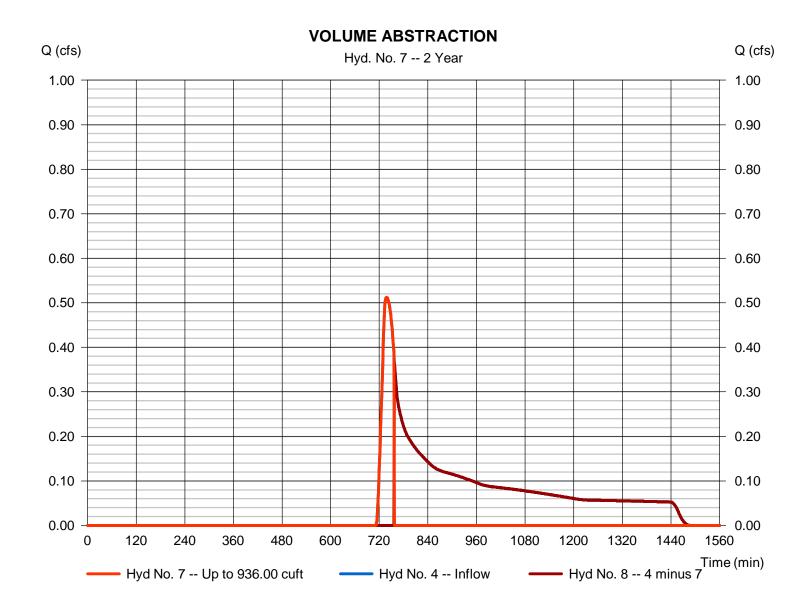
Hyd. No. 7

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 0.512 cfsStorm frequency= 2 yrsTime to peak= 738 minTime interval= 2 minHyd. volume= 948 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 8

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

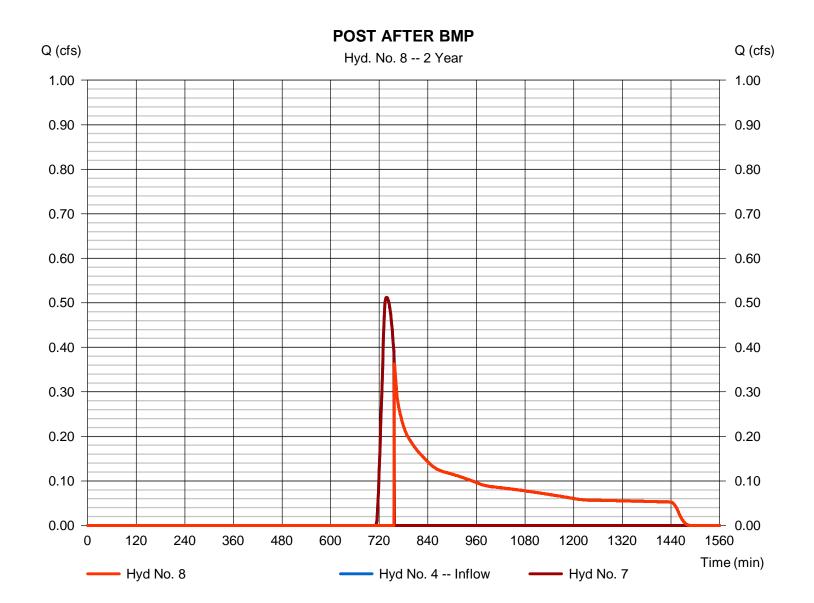
Hyd. No. 8

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 0.364 cfsStorm frequency= 2 yrsTime to peak= 758 minTime interval= 2 minHyd. volume= 3,819 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 7

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



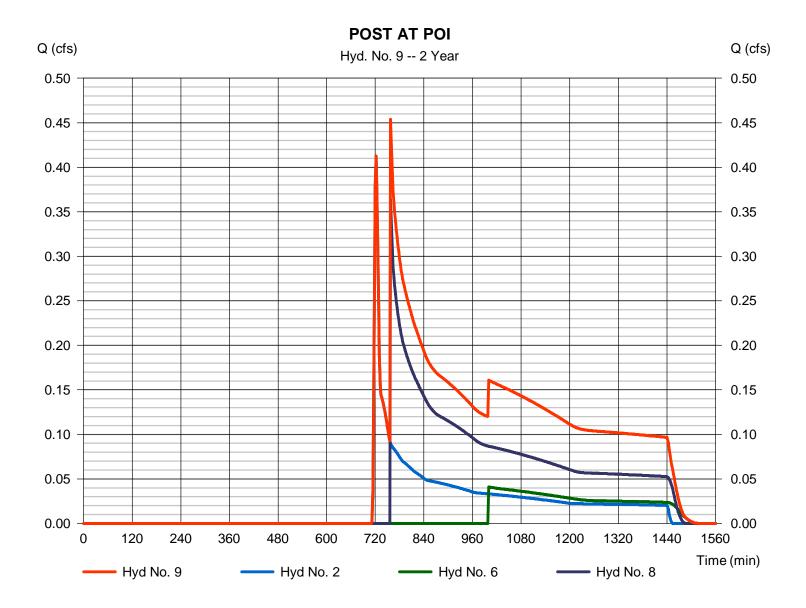
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Hyd. No. 9

POST AT POI

Hydrograph type = Combine Peak discharge = 0.454 cfsStorm frequency = 2 yrsTime to peak = 758 min Time interval = 2 min Hyd. volume = 6,506 cuftInflow hyds. Contrib. drain. area = 2, 6, 8= 1.790 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)							
1	51.6351	12.5000	0.8881								
2	53.0270	11.7000	0.8506								
3	0.0000	0.0000	0.0000								
5	61.9255	12.6000	0.8350								
10	65.3595	12.8000	0.8167								
25	51.9840	10.7000	0.7342								
50	50.0341	10.3000	0.7046								
100	42.0647	8.6000	0.6468								

File name: Morgantown IDF.IDF

Intensity = $B/(Tc + D)^E$

Return					Intens	ity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	4.06	3.25	2.72	2.35	2.07	1.85	1.67	1.53	1.41	1.31	1.23	1.15
2	4.84	3.87	3.24	2.80	2.47	2.22	2.02	1.85	1.71	1.59	1.49	1.40
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5.65	4.58	3.88	3.38	3.00	2.70	2.46	2.26	2.10	1.96	1.84	1.73
10	6.22	5.08	4.32	3.78	3.36	3.04	2.78	2.56	2.38	2.22	2.09	1.97
25	6.88	5.62	4.79	4.21	3.77	3.42	3.14	2.91	2.72	2.55	2.41	2.28
50	7.32	6.00	5.14	4.52	4.06	3.70	3.41	3.17	2.96	2.79	2.63	2.50
100	7.78	6.35	5.44	4.81	4.33	3.96	3.66	3.41	3.20	3.02	2.87	2.73

Tc = time in minutes. Values may exceed 60.

P-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Morgantown Road\Hydraflow Rev 1\Morgantown Precip.pc

		Rainfall Precipitation Table (in)											
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr					
SCS 24-hour	2.68	3.22	0.00	4.04	4.72	5.70	6.53	7.41					
SCS 6-Hr	1.88	2.27	0.00	2.83	3.28	3.92	4.43	4.98					
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					

7

8

9

Diversion1

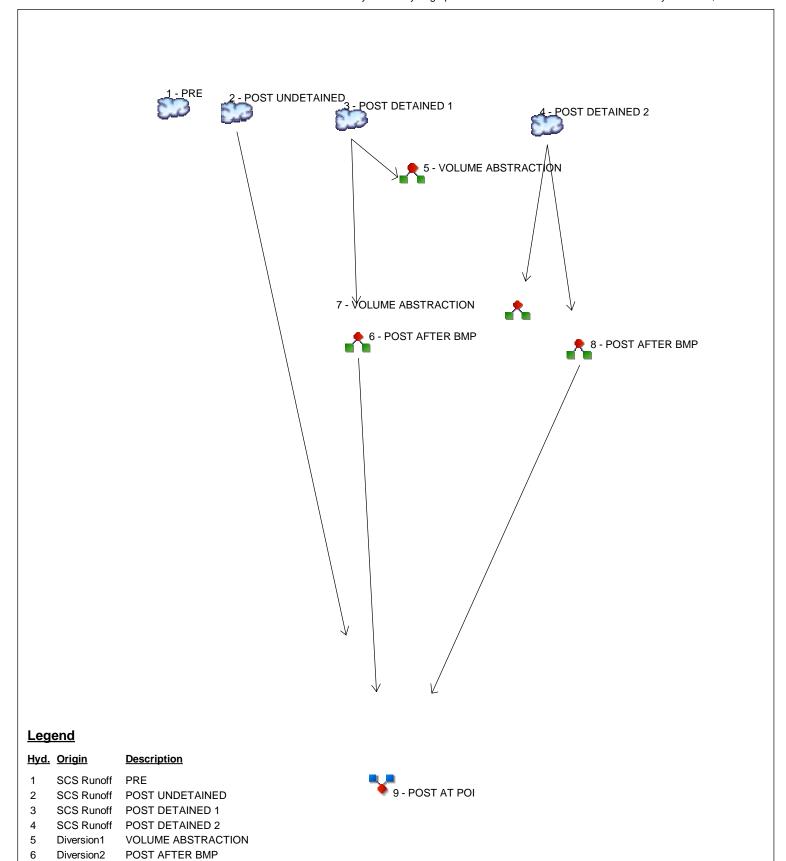
Diversion2

Combine

VOLUME ABSTRACTION

POST AFTER BMP

POST AT POI



Project: \\nuss010fp2\shared\Marcellus_M-Z - Mark Sladic\Sunoco Logistics (SXL)\\112I\cup05952sd\\\mathbf{M}\)ar\\mathbf{n}\)er\\mathbf{E}\text{a}\text{200P}\text{6}\text{ase 2}\\\mathbf{E}\text{SCG}\mathbf{P}-2\\\mathbf{F}\)

Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

lyd. lo.		Inflow	Peak Outflow (cfs)								Hydrograph
о.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff						9.557				PRE
2	SCS Runoff						2.274				POST UNDETAINED
3	SCS Runoff						1.844				POST DETAINED 1
4	SCS Runoff						5.064				POST DETAINED 2
5	Diversion1	3					1.844				VOLUME ABSTRACTION
6	Diversion2	3					1.538				POST AFTER BMP
7	Diversion1	4					3.912				VOLUME ABSTRACTION
3	Diversion2	4					5.064				POST AFTER BMP
9	Combine	2, 6, 8					7.173				POST AT POI

Proj. file: \\nuss010fp2\shared\Marcellus_M-Z - Mark Sladic\Sunoco Logistics (\$XIL)\est2\\C05958 /(201958 / (201958 - East Phase 2)\ESC\GP-2\

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

	•		•	•	пушаном п	yurugrapris Exter	ISION TO AUTOCA	D® Civil 3D® 2015 by Autodesk, Inc. v10.4
Hyd. Hydrograph No. type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1 SCS Runoff	9.557	2	720	25,212				PRE
2 SCS Runoff	2.274	2	720	5,853				POST UNDETAINED
3 SCS Runoff	1.844	2	726	6,801				POST DETAINED 1
4 SCS Runoff	5.064	2	722	15,510				POST DETAINED 2
5 Diversion1	1.844	2	726	1,577	3			VOLUME ABSTRACTION
6 Diversion2	1.538	2	732	5,223	3			POST AFTER BMP
7 Diversion1	3.912	2	718	1,293	4			VOLUME ABSTRACTION
8 Diversion2	5.064	2	722	14,217	4			POST AFTER BMP
9 Combine	7.173	2	722	25,293	2, 6, 8			POST AT POI

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Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

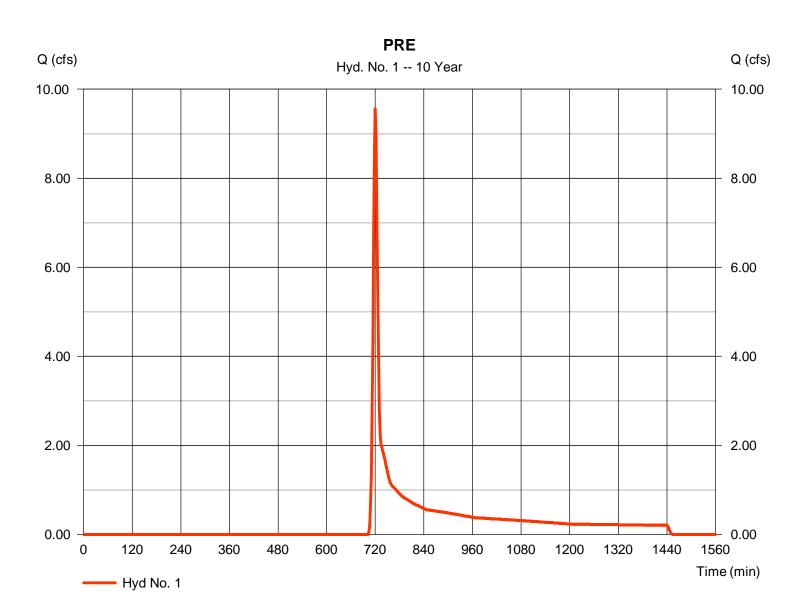
Tuesday, 11 / 1 / 2016

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 9.557 cfsStorm frequency Time to peak = 10 yrs= 720 min Time interval = 2 min Hyd. volume = 25.212 cuftDrainage area Curve number = 8.230 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 4.72 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(7.780 x 55) + (0.450 x 58)] / 8.230



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 3.22 = 4.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 6.19	+	0.00	+	0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpaved =5.18	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})71.0		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							8.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

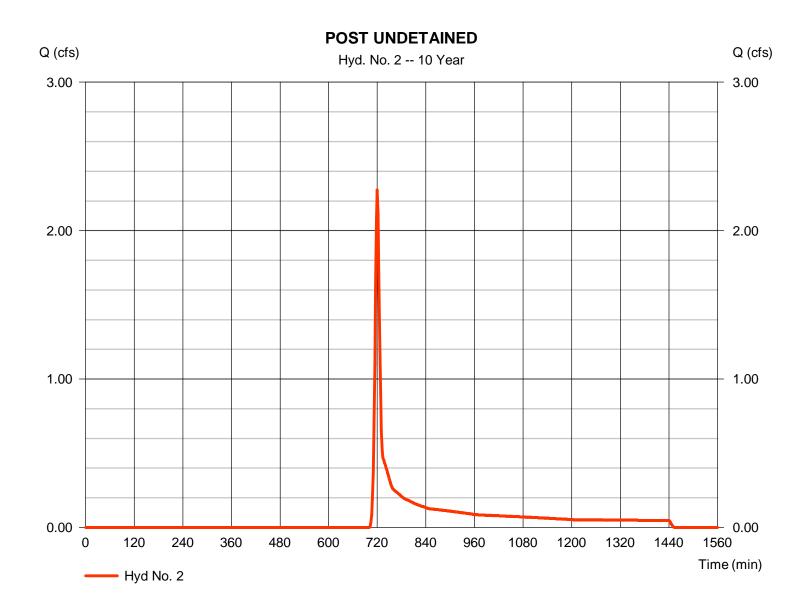
Tuesday, 11 / 1 / 2016

Hyd. No. 2

POST UNDETAINED

= SCS Runoff Hydrograph type Peak discharge = 2.274 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 2 min Hyd. volume = 5.853 cuftCurve number Drainage area = 1.790 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 4.72 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.030 \times 85) + (0.430 \times 58) + (1.330 \times 55)] / 1.790$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 2POST UNDETAINED

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 3.22 = 4.00	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpaved =5.18	d	0.00 0.00 Unpave 0.00	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})71.0		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							8.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

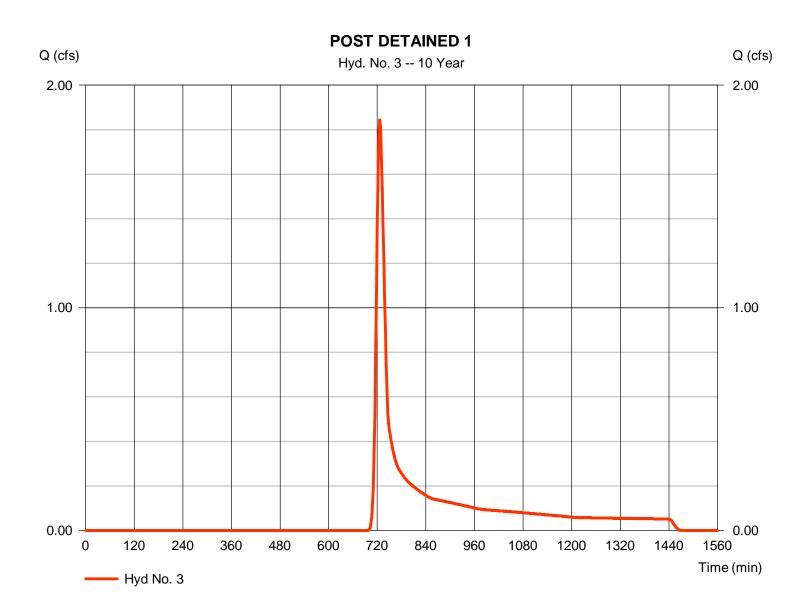
Tuesday, 11 / 1 / 2016

Hyd. No. 3

POST DETAINED 1

= SCS Runoff Hydrograph type Peak discharge = 1.844 cfsStorm frequency Time to peak = 726 min = 10 yrsTime interval = 2 min Hyd. volume = 6.801 cuftDrainage area Curve number = 1.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) = 18.70 min Total precip. = 4.72 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 85) + (0.460 \times 58) + (1.240 \times 55)] / 1.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

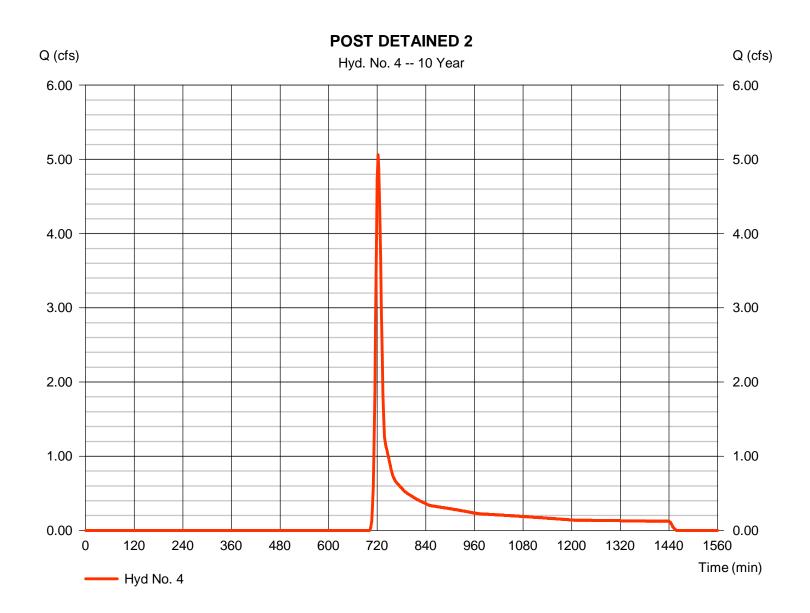
Tuesday, 11 / 1 / 2016

Hyd. No. 4

POST DETAINED 2

Hydrograph type = SCS Runoff Peak discharge = 5.064 cfsStorm frequency Time to peak = 722 min = 10 yrsTime interval = 2 min Hyd. volume = 15.510 cuftCurve number Drainage area = 4.600 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 13.10 \, \text{min}$ Total precip. = 4.72 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.040 \times 85) + (0.750 \times 58) + (3.810 \times 55)] / 4.600$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

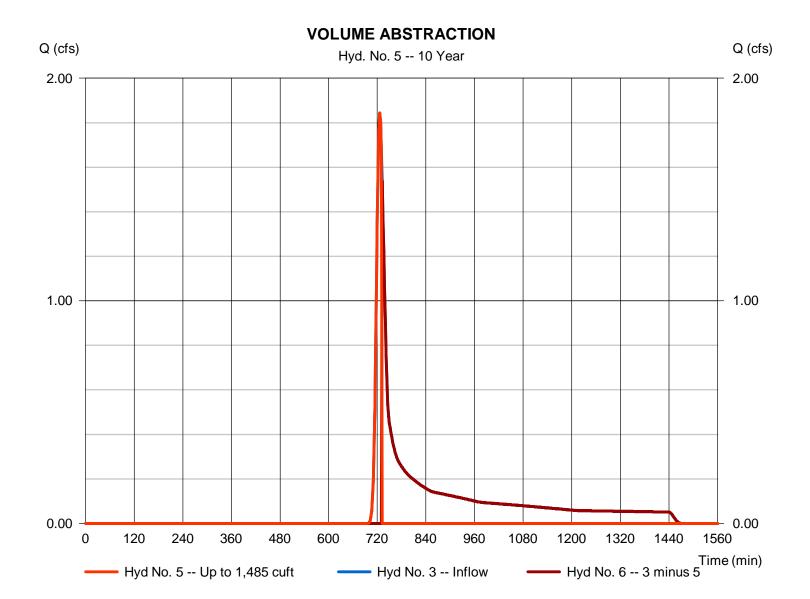
Hyd. No. 5

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 1.844 cfsStorm frequency= 10 yrsTime to peak= 726 minTime interval= 2 minHyd. volume= 1,577 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 6

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

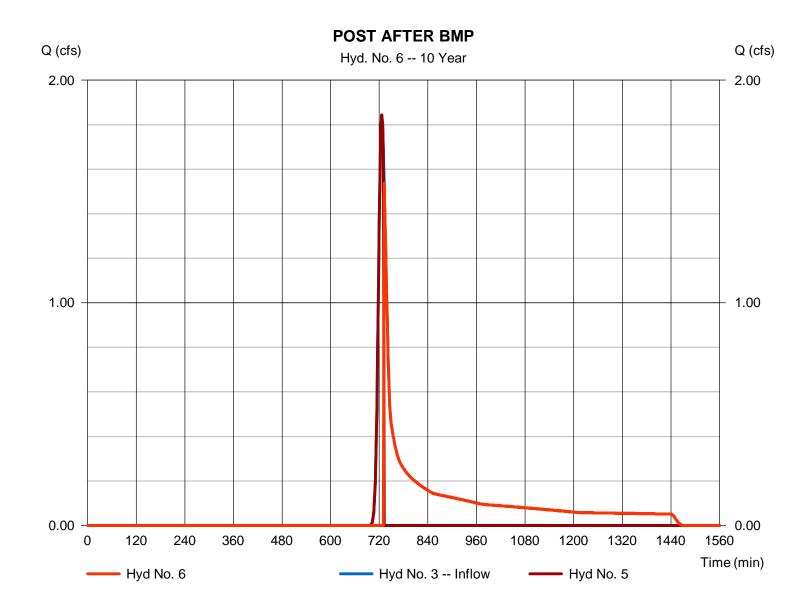
Hyd. No. 6

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 1.538 cfsStorm frequency= 10 yrsTime to peak= 732 minTime interval= 2 minHyd. volume= 5,223 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 5

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

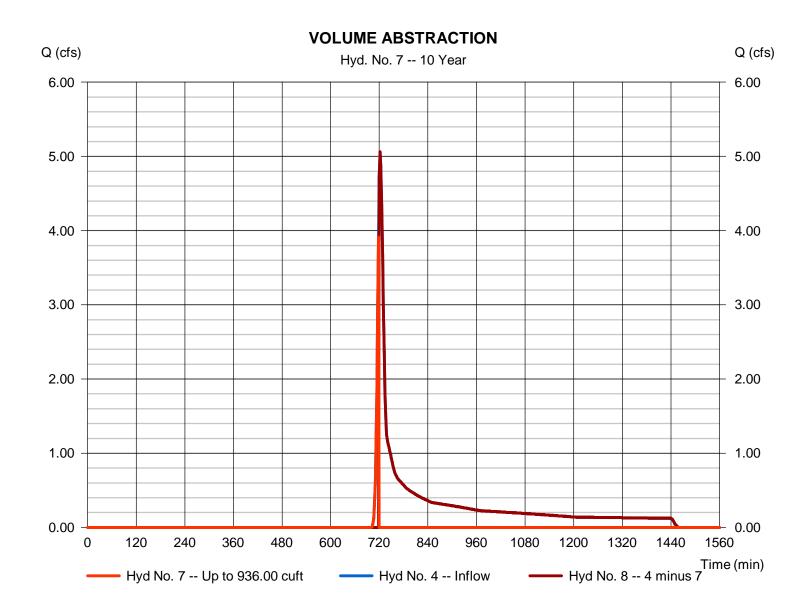
Hyd. No. 7

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 3.912 cfsStorm frequency= 10 yrsTime to peak= 718 minTime interval= 2 minHyd. volume= 1,293 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 8

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

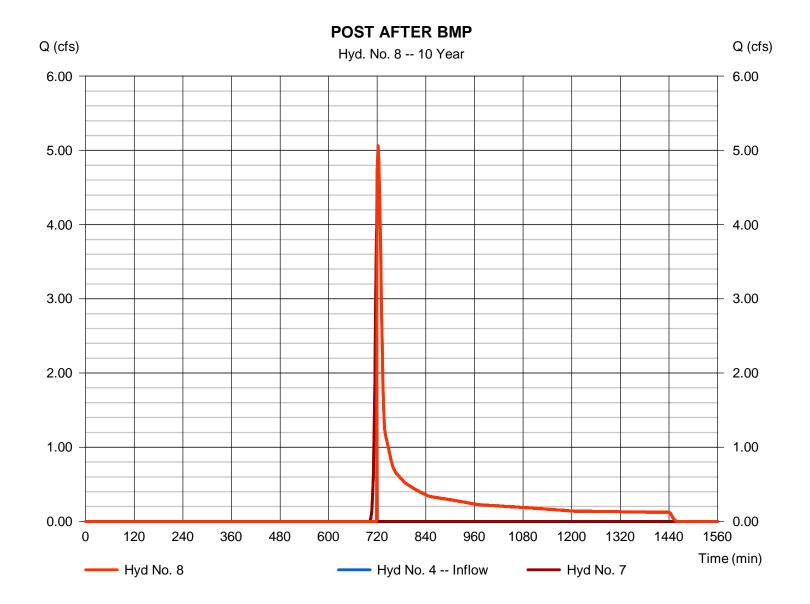
Hyd. No. 8

POST AFTER BMP

Hydrograph type = Diversion2 Peak discharge = 5.064 cfs
Storm frequency = 10 yrs Time to peak = 722 min
Time interval = 2 min Hyd. volume = 14,217 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 7

Diversion method = First Flush Volume Volume Up To = 936.00 cuft



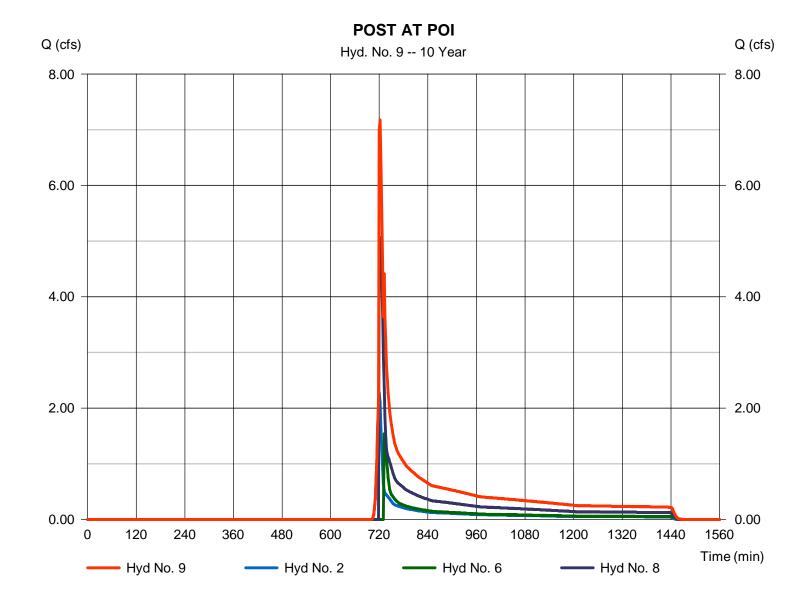
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Hyd. No. 9

POST AT POI

Hydrograph type = Combine = 7.173 cfsPeak discharge Storm frequency = 10 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 25,293 cuftInflow hyds. Contrib. drain. area = 2, 6, 8= 1.790 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)							
1	51.6351	12.5000	0.8881								
2	53.0270	11.7000	0.8506								
3	0.0000	0.0000	0.0000								
5	61.9255	12.6000	0.8350								
10	65.3595	12.8000	0.8167								
25	51.9840	10.7000	0.7342								
50	50.0341	10.3000	0.7046								
100	42.0647	8.6000	0.6468								

File name: Morgantown IDF.IDF

Intensity = $B/(Tc + D)^E$

Return					Intens	ity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	4.06	3.25	2.72	2.35	2.07	1.85	1.67	1.53	1.41	1.31	1.23	1.15
2	4.84	3.87	3.24	2.80	2.47	2.22	2.02	1.85	1.71	1.59	1.49	1.40
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5.65	4.58	3.88	3.38	3.00	2.70	2.46	2.26	2.10	1.96	1.84	1.73
10	6.22	5.08	4.32	3.78	3.36	3.04	2.78	2.56	2.38	2.22	2.09	1.97
25	6.88	5.62	4.79	4.21	3.77	3.42	3.14	2.91	2.72	2.55	2.41	2.28
50	7.32	6.00	5.14	4.52	4.06	3.70	3.41	3.17	2.96	2.79	2.63	2.50
100	7.78	6.35	5.44	4.81	4.33	3.96	3.66	3.41	3.20	3.02	2.87	2.73

Tc = time in minutes. Values may exceed 60.

P-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Morgantown Road\Hydraflow Rev 1\Morgantown Precip.pc

		Rainfall Precipitation Table (in)											
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr					
SCS 24-hour	2.68	3.22	0.00	4.04	4.72	5.70	6.53	7.41					
SCS 6-Hr	1.88	2.27	0.00	2.83	3.28	3.92	4.43	4.98					
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					

7

8

9

Diversion1

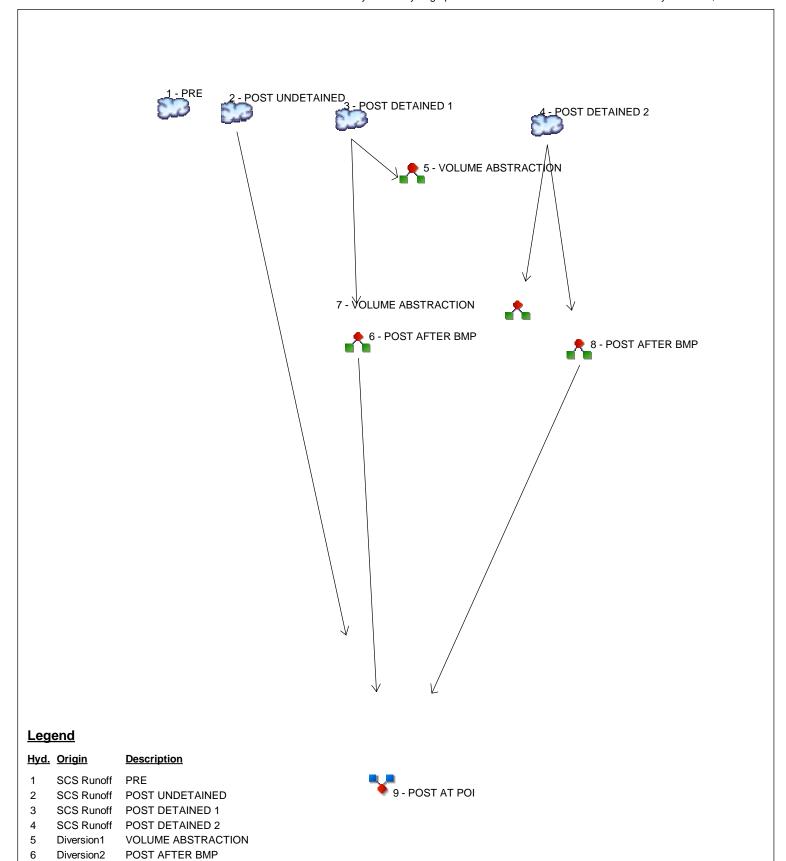
Diversion2

Combine

VOLUME ABSTRACTION

POST AFTER BMP

POST AT POI



Project: \\nuss010fp2\shared\Marcellus_M-Z - Mark Sladic\Sunoco Logistics (SXL)\\112I\cup05952sd\\\mathbf{M}\)ar\\mathbf{n}\)er\\mathbf{E}\text{a}\text{200P}\text{6}\text{ase 2}\\\mathbf{E}\text{SCG}\mathbf{P}-2\\\mathbf{F}\)

Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

lyd. lo.	Hydrograph	Inflow byd(s)				Hydrograph Description					
о.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff								23.26		PRE
2	SCS Runoff								5.332		POST UNDETAINED
3	SCS Runoff								4.654		POST DETAINED 1
4	SCS Runoff								12.18		POST DETAINED 2
5	Diversion1	3							3.657		VOLUME ABSTRACTION
6	Diversion2	3							4.654		POST AFTER BMP
7	Diversion1	4							2.351		VOLUME ABSTRACTION
3	Diversion2	4							12.18		POST AFTER BMP
)	Combine	2, 6, 8							21.57		POST AT POI

Proj. file: \\nuss010fp2\shared\Marcellus_M-Z - Mark Sladic\Sunoco Logistics (\$XIL)\est2\\C05958 /(201958 / (201958 - East Phase 2)\ESC\GP-2\

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	23.26	2	720	54,716				PRE
2	SCS Runoff	5.332	2	720	12,466				POST UNDETAINED
3	SCS Runoff	4.654	2	724	13,647				POST DETAINED 1
4	SCS Runoff	12.18	2	722	33,037				POST DETAINED 2
5	Diversion1	3.657	2	718	1,841	3			VOLUME ABSTRACTION
6	Diversion2	4.654	2	724	11,805	3			POST AFTER BMP
7	Diversion1	2.351	2	708	1,002	4			VOLUME ABSTRACTION
8	Diversion2	12.18	2	722	32,035	4			POST AFTER BMP
9	Combine	21.57	2	722	56,306	2, 6, 8			POST AT POI

\\nuss010fp2\shared\Marcellus_M-Z - Mark \$lakbowenimerioblogistikesa(SXL)\112|C05@56a(Matinder/E20s16Phase 2)\ESCGP-2\PPP\02 S

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

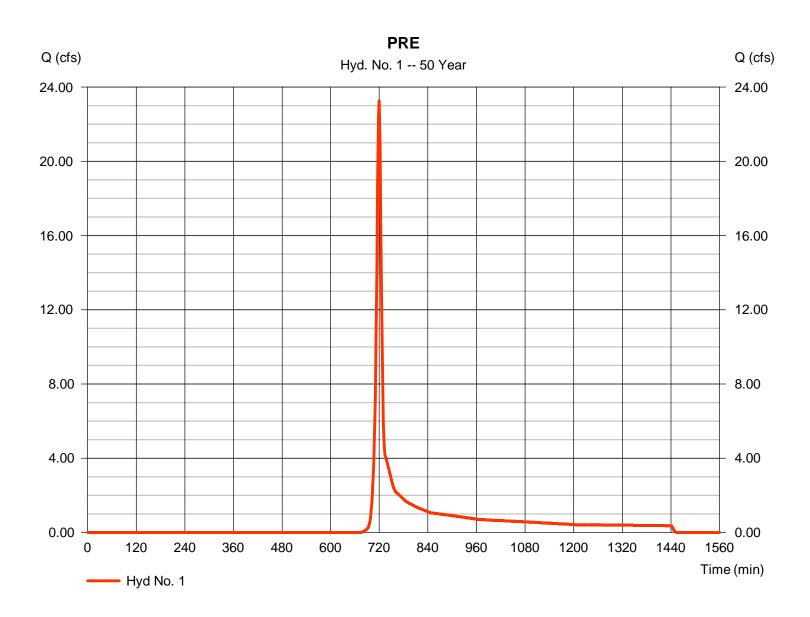
Tuesday, 11 / 1 / 2016

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 23.26 cfs= 720 min Storm frequency Time to peak = 50 yrsTime interval = 2 min Hyd. volume = 54.716 cuft Drainage area Curve number = 8.230 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 6.53 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(7.780 x 55) + (0.450 x 58)] / 8.230



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>		
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 3.22 = 4.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00				
Travel Time (min)	= 6.19	+	0.00	+	0.00	=	6.19		
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpaved =5.18		0.00 0.00 Unpaved 0.00		0.00 0.00 Paved 0.00				
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42		
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015				
Flow length (ft)	({0})71.0		0.0		0.0				
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16		
Total Travel Time, Tc	Total Travel Time, Tc								

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

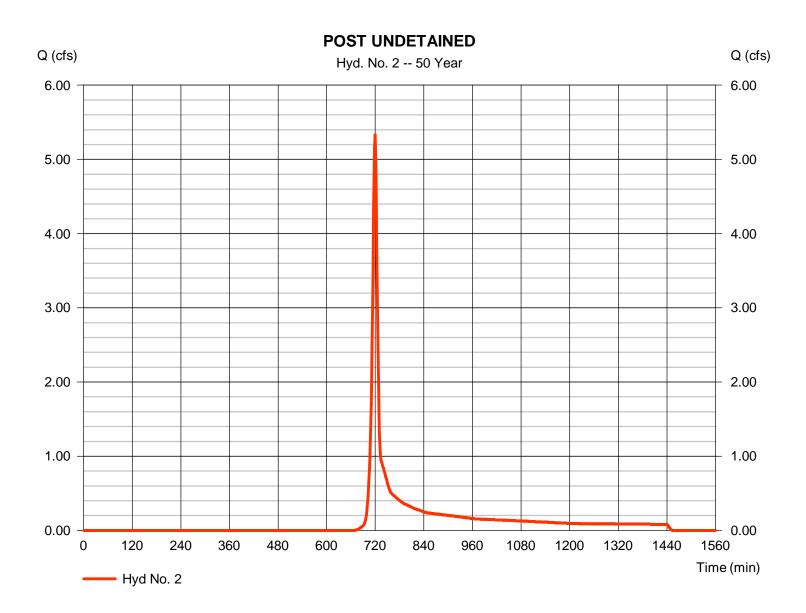
Tuesday, 11 / 1 / 2016

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 5.332 cfsStorm frequency Time to peak = 720 min = 50 yrsTime interval = 2 min Hyd. volume = 12.466 cuft Curve number Drainage area = 1.790 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 6.53 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.030 \times 85) + (0.430 \times 58) + (1.330 \times 55)] / 1.790$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 2POST UNDETAINED

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 3.22 = 4.00	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpaved =5.18	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})71.0		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							8.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

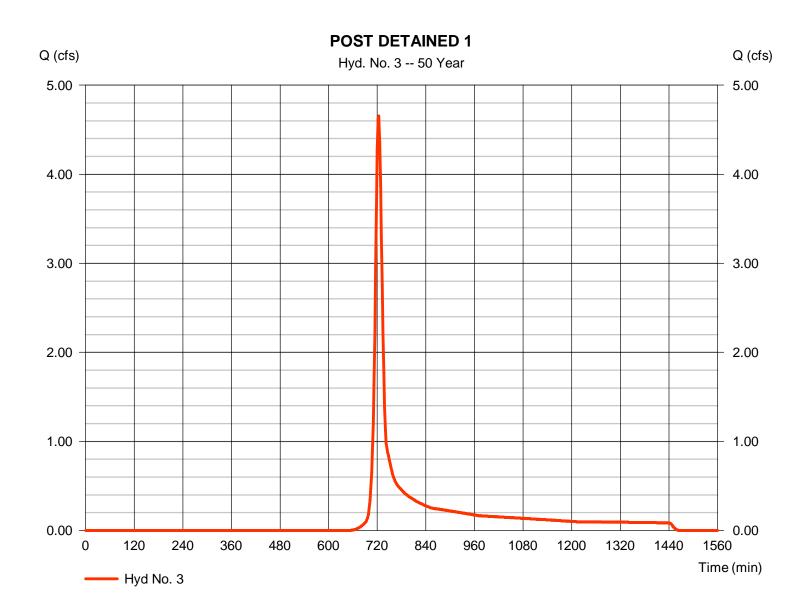
Tuesday, 11 / 1 / 2016

Hyd. No. 3

POST DETAINED 1

Hydrograph type = SCS Runoff Peak discharge = 4.654 cfsStorm frequency Time to peak = 724 min = 50 yrsTime interval = 2 min Hyd. volume = 13.647 cuft Curve number Drainage area = 1.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. = 6.53 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.140 \times 85) + (0.460 \times 58) + (1.240 \times 55)] / 1.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

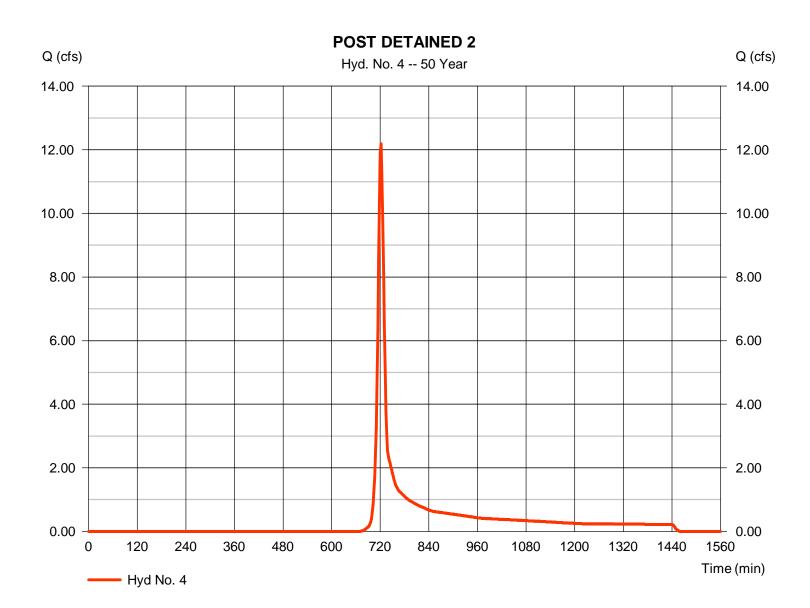
Tuesday, 11 / 1 / 2016

Hyd. No. 4

POST DETAINED 2

Hydrograph type = SCS Runoff Peak discharge = 12.18 cfsStorm frequency Time to peak = 722 min = 50 yrsTime interval = 2 min Hyd. volume = 33.037 cuft Drainage area Curve number = 4.600 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) = 11.30 min Total precip. = 6.53 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.040 \times 85) + (0.750 \times 58) + (3.810 \times 55)] / 4.600$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

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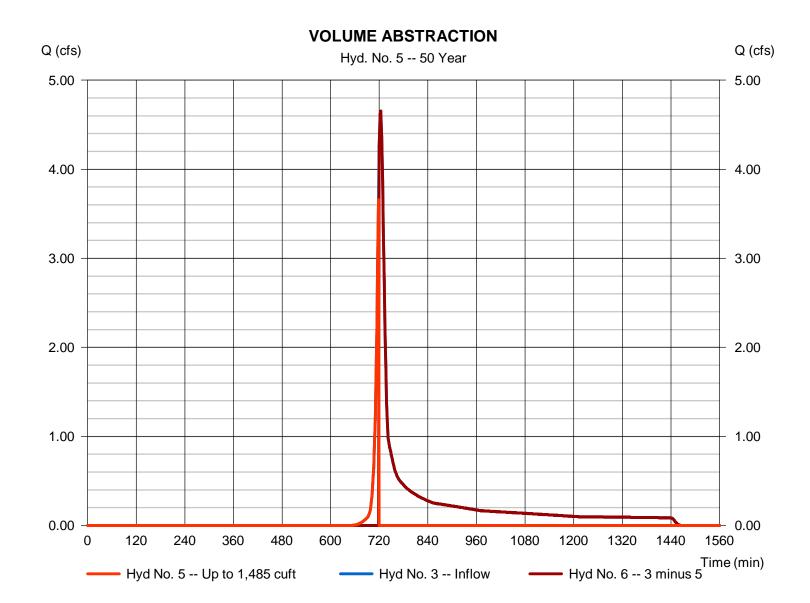
Hyd. No. 5

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 3.657 cfsStorm frequency= 50 yrsTime to peak= 718 minTime interval= 2 minHyd. volume= 1,841 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 6

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

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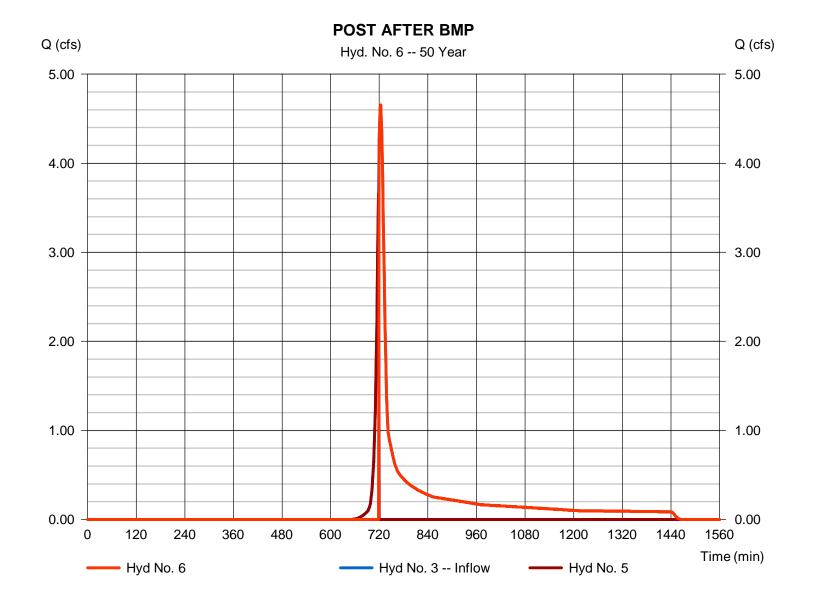
Hyd. No. 6

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 4.654 cfsStorm frequency= 50 yrsTime to peak= 724 minTime interval= 2 minHyd. volume= 11,805 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 5

Diversion method = First Flush Volume Volume Up To = 1,485 cuft



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

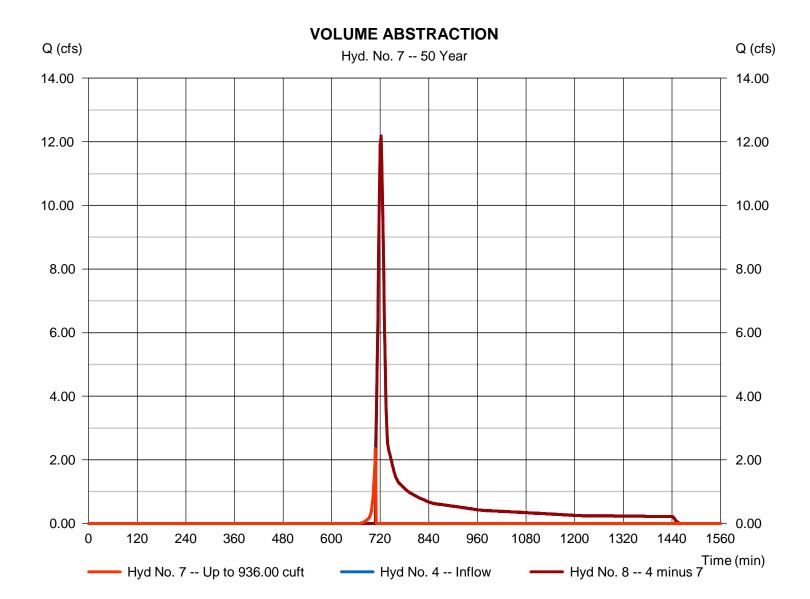
Tuesday, 11 / 1 / 2016

Hyd. No. 7

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 2.351 cfsStorm frequency= 50 yrsTime to peak= 708 minTime interval= 2 minHyd. volume= 1,002 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 8



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

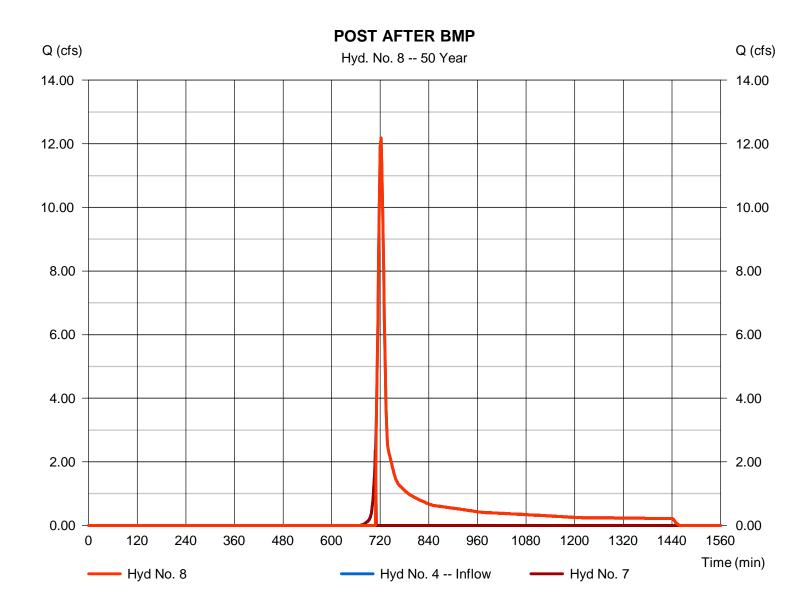
Tuesday, 11 / 1 / 2016

Hyd. No. 8

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 12.18 cfsStorm frequency= 50 yrsTime to peak= 722 minTime interval= 2 minHyd. volume= 32,035 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 7



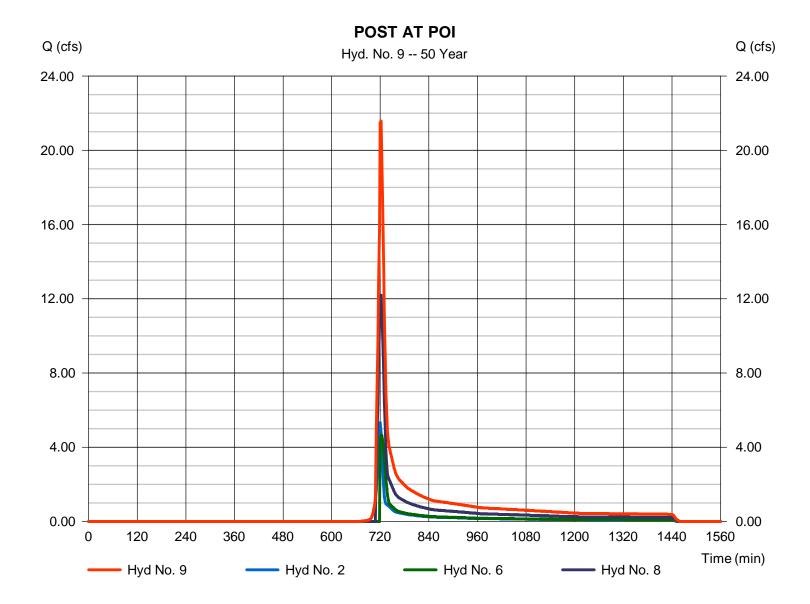
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Hyd. No. 9

POST AT POI

Hydrograph type = Combine Peak discharge = 21.57 cfsStorm frequency = 50 yrsTime to peak = 722 min Time interval = 2 min Hyd. volume = 56,306 cuft Inflow hyds. Contrib. drain. area = 1.790 ac= 2, 6, 8



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)							
1	51.6351	12.5000	0.8881								
2	53.0270	11.7000	0.8506								
3	0.0000	0.0000	0.0000								
5	61.9255	12.6000	0.8350								
10	65.3595	12.8000	0.8167								
25	51.9840	10.7000	0.7342								
50	50.0341	10.3000	0.7046								
100	42.0647	8.6000	0.6468								

File name: Morgantown IDF.IDF

Intensity = $B/(Tc + D)^E$

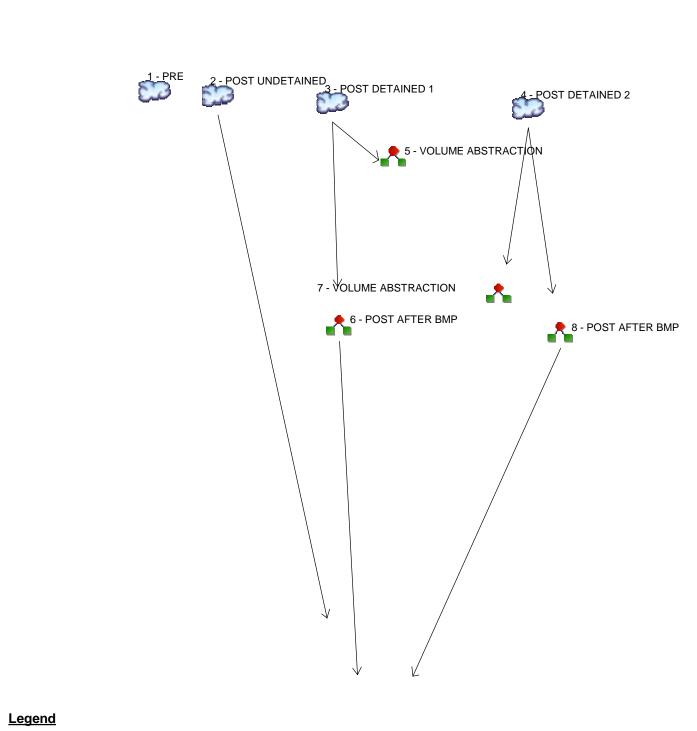
Return					Intens	ity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	4.06	3.25	2.72	2.35	2.07	1.85	1.67	1.53	1.41	1.31	1.23	1.15
2	4.84	3.87	3.24	2.80	2.47	2.22	2.02	1.85	1.71	1.59	1.49	1.40
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5.65	4.58	3.88	3.38	3.00	2.70	2.46	2.26	2.10	1.96	1.84	1.73
10	6.22	5.08	4.32	3.78	3.36	3.04	2.78	2.56	2.38	2.22	2.09	1.97
25	6.88	5.62	4.79	4.21	3.77	3.42	3.14	2.91	2.72	2.55	2.41	2.28
50	7.32	6.00	5.14	4.52	4.06	3.70	3.41	3.17	2.96	2.79	2.63	2.50
100	7.78	6.35	5.44	4.81	4.33	3.96	3.66	3.41	3.20	3.02	2.87	2.73

Tc = time in minutes. Values may exceed 60.

P-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Morgantown Road\Hydraflow Rev 1\Morgantown Precip.pc

		F	tainfall F	Precipitat	tion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	2.68	3.22	0.00	4.04	4.72	5.70	6.53	7.41
SCS 6-Hr	1.88	2.27	0.00	2.83	3.28	3.92	4.43	4.98
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Watershed Model Schematic



Hyd.	<u>Origin</u>	<u>Description</u>	
1	SCS Runoff	f PRE	
2	SCS Runoff	f POST UNDETAINED ▼ 9 - POST AT POI	
3	SCS Runoff	f POST DETAINED 1	
4	SCS Runoff	f POST DETAINED 2	
5	Diversion1	VOLUME ABSTRACTION	
6	Diversion2	POST AFTER BMP	
7	Diversion1	VOLUME ABSTRACTION	
8	Diversion2	POST AFTER BMP	
9	Combine	POST AT POI	

Project: Morgantown.gpw

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Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

lyd. lo.	Hydrograph	Inflow byd(s)		Peak Outflow (cfs)							Hydrograph
υ.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
	SCS Runoff									30.82	PRE
	SCS Runoff									7.008	POST UNDETAINED
3	SCS Runoff									7.021	POST DETAINED 1
ļ	SCS Runoff									16.10	POST DETAINED 2
	Diversion1	3								3.019	VOLUME ABSTRACTION
	Diversion2	3								7.021	POST AFTER BMP
	Diversion1	4								1.491	VOLUME ABSTRACTION
	Diversion2	4								16.10	POST AFTER BMP
	Combine	2, 6, 8								29.83	POST AT POI

Proj. file: Morgantown.gpw

Tuesday, 11 / 1 / 2016

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	30.82	2	720	71,361				PRE
2	SCS Runoff	7.008	2	720	16,173				POST UNDETAINED
3	SCS Runoff	7.021	2	722	18,542				POST DETAINED 1
4	SCS Runoff	16.10	2	722	42,860				POST DETAINED 2
5	Diversion1	3.019	2	712	1,668	3			VOLUME ABSTRACTION
6	Diversion2	7.021	2	722	16,874	3			POST AFTER BMP
7	Diversion1	1.491	2	702	942	4			VOLUME ABSTRACTION
8	Diversion2	16.10	2	722	41,918	4			POST AFTER BMP
9	Combine	29.83	2	720	74,965	2, 6, 8			POST AT POI
Моі	gantown.gpv	v			Return F	Period: 100	Year	Tuesday, 1	1 / 1 / 2016

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

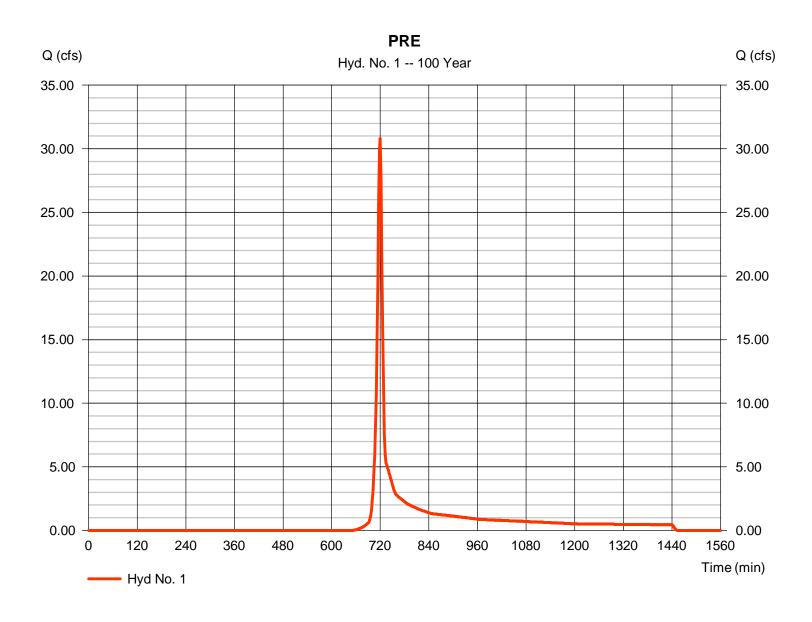
Tuesday, 11 / 1 / 2016

Hyd. No. 1

PRE

Hydrograph type = SCS Runoff Peak discharge = 30.82 cfsStorm frequency Time to peak = 720 min = 100 yrsTime interval = 2 min Hyd. volume = 71,361 cuftDrainage area Curve number = 8.230 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 7.41 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(7.780 x 55) + (0.450 x 58)] / 8.230



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Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 3.22 = 4.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 6.19	+	0.00	+	0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpave =5.18	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})71.0		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							8.80 min

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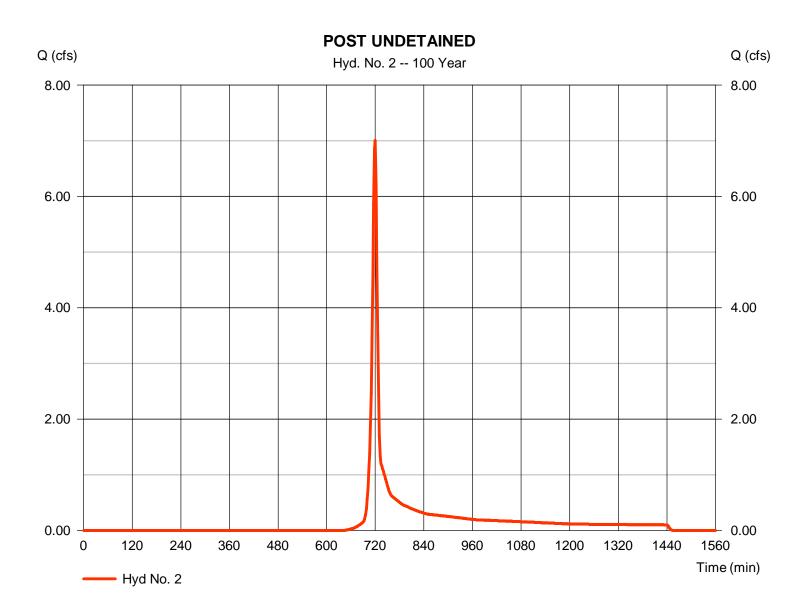
Tuesday, 11 / 1 / 2016

Hyd. No. 2

POST UNDETAINED

= 7.008 cfsHydrograph type = SCS Runoff Peak discharge = 720 min Storm frequency Time to peak = 100 yrsTime interval = 2 min Hyd. volume = 16.173 cuft Drainage area Curve number = 1.790 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.80 \, \text{min}$ Total precip. = 7.41 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.030 \times 85) + (0.430 \times 58) + (1.330 \times 55)] / 1.790$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 2POST UNDETAINED

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 3.22 = 4.00	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	6.19
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 751.00 = 10.30 = Unpaved =5.18	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.42	+	0.00	+	0.00	=	2.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.00 = 6.32 = 6.20 = 0.030 =7.51		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})71.0		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							8.80 min

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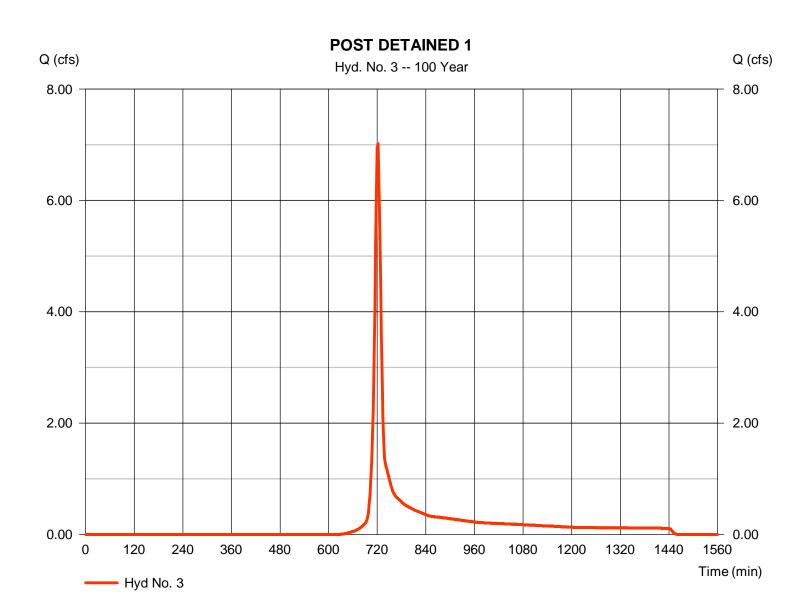
Tuesday, 11 / 1 / 2016

Hyd. No. 3

POST DETAINED 1

= SCS Runoff = 7.021 cfsHydrograph type Peak discharge Storm frequency Time to peak = 722 min = 100 yrsTime interval = 2 min Hyd. volume = 18.542 cuft Drainage area Curve number = 1.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 12.90 \, \text{min}$ Total precip. = 7.41 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 85) + (0.460 \times 58) + (1.240 \times 55)] / 1.840$



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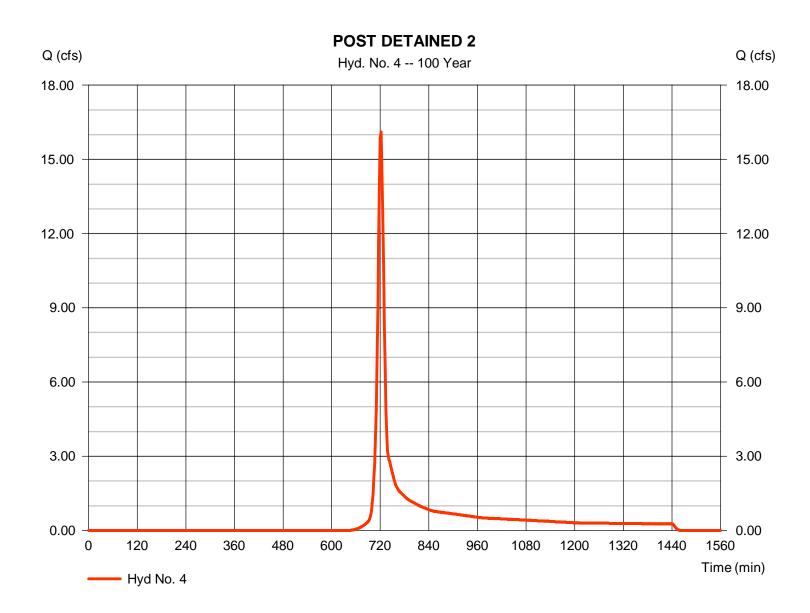
Tuesday, 11 / 1 / 2016

Hyd. No. 4

POST DETAINED 2

Hydrograph type = SCS Runoff Peak discharge = 16.10 cfsStorm frequency Time to peak = 722 min = 100 yrsTime interval = 2 min Hyd. volume = 42.860 cuftDrainage area Curve number = 4.600 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) = 11.00 min Total precip. = 7.41 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.040 \times 85) + (0.750 \times 58) + (3.810 \times 55)] / 4.600$



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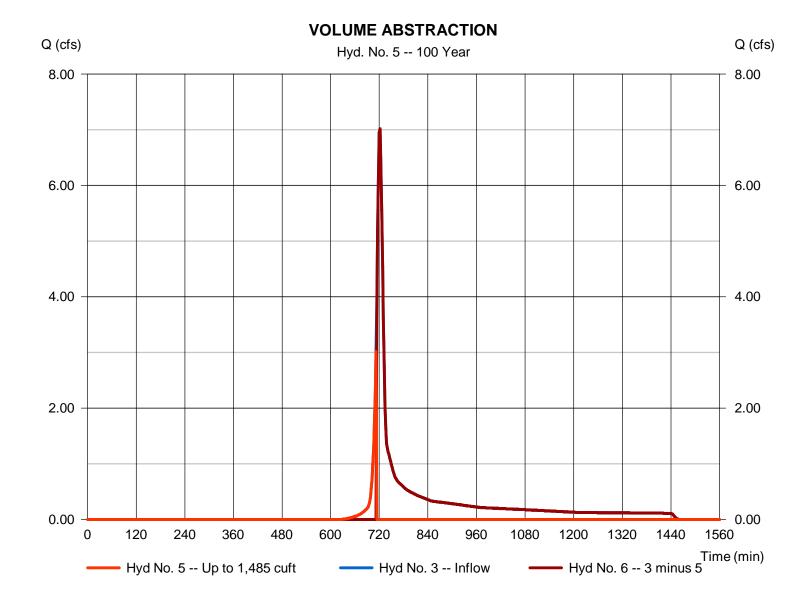
Tuesday, 11 / 1 / 2016

Hyd. No. 5

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 3.019 cfsStorm frequency= 100 yrsTime to peak= 712 minTime interval= 2 minHyd. volume= 1,668 cuft

Inflow hydrograph = 3 - POST DETAINED 1 2nd diverted hyd. = 6



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Tuesday, 11 / 1 / 2016

Hyd. No. 6

POST AFTER BMP

Hydrograph type = Diversion2 Storm frequency = 100 yrs Time interval = 2 min

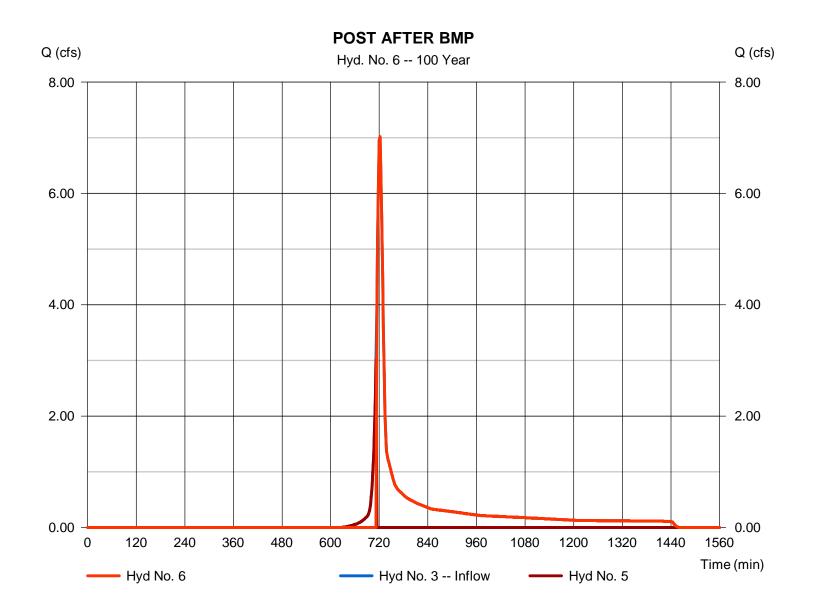
Inflow hydrograph = 3 - POST DETAINED 1

Diversion method = First Flush Volume

Peak discharge = 7.021 cfs Time to peak = 722 min Hyd. volume = 16,874 cuft

2nd diverted hyd. = 5

Volume Up To = 1,485 cuft



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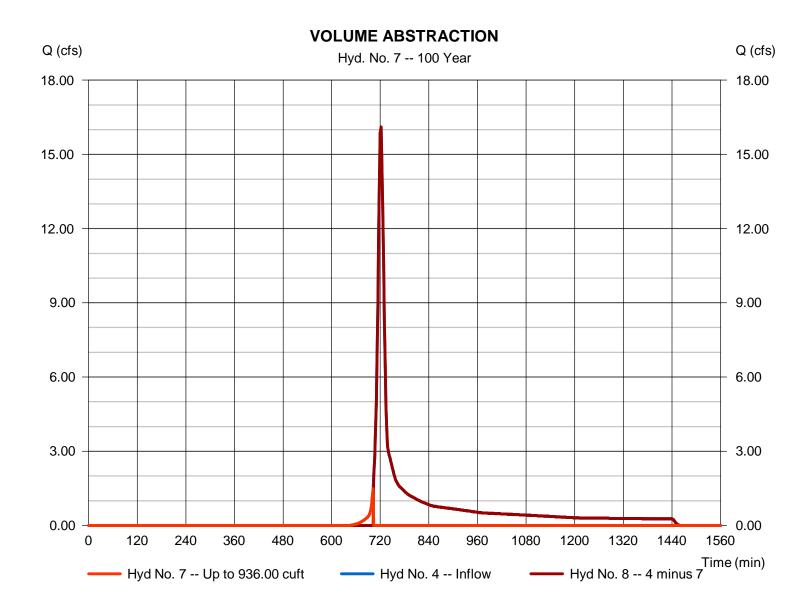
Tuesday, 11 / 1 / 2016

Hyd. No. 7

VOLUME ABSTRACTION

Hydrograph type= Diversion1Peak discharge= 1.491 cfsStorm frequency= 100 yrsTime to peak= 702 minTime interval= 2 minHyd. volume= 942 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 8



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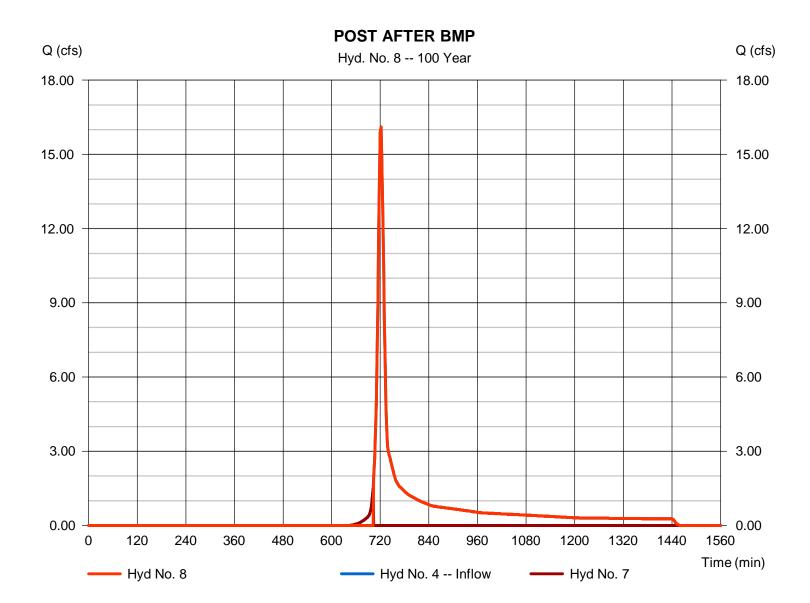
Tuesday, 11 / 1 / 2016

Hyd. No. 8

POST AFTER BMP

Hydrograph type= Diversion2Peak discharge= 16.10 cfsStorm frequency= 100 yrsTime to peak= 722 minTime interval= 2 minHyd. volume= 41,918 cuft

Inflow hydrograph = 4 - POST DETAINED 2 2nd diverted hyd. = 7



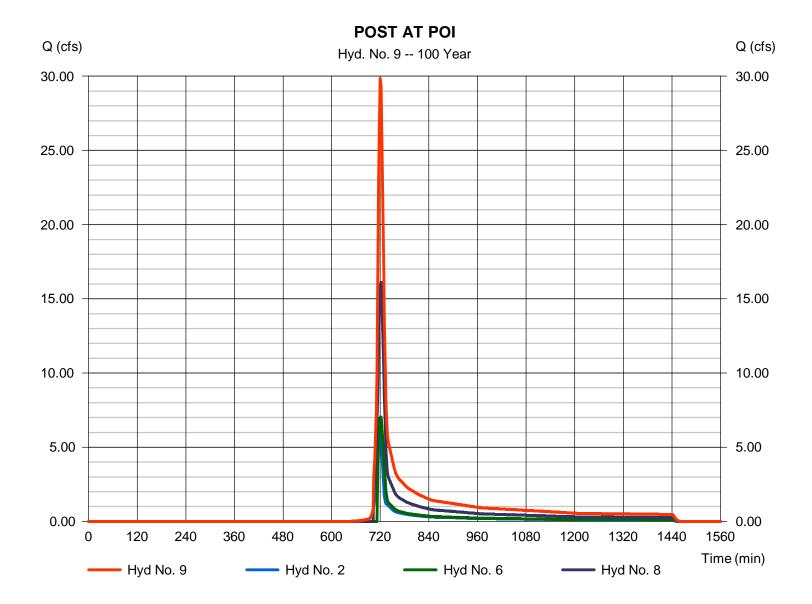
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Tuesday, 11 / 1 / 2016

Hyd. No. 9

POST AT POI

Hydrograph type = Combine Peak discharge = 29.83 cfsStorm frequency = 100 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 74,965 cuftInflow hyds. Contrib. drain. area = 1.790 ac= 2, 6, 8



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Tuesday, 11 / 1 / 2016

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)							
1	51.6351	12.5000	0.8881								
2	53.0270	11.7000	0.8506								
3	0.0000	0.0000	0.0000								
5	61.9255	12.6000	0.8350								
10	65.3595	12.8000	0.8167								
25	51.9840	10.7000	0.7342								
50	50.0341	10.3000	0.7046								
100	42.0647	8.6000	0.6468								

File name: Morgantown IDF.IDF

Intensity = $B/(Tc + D)^E$

Return					Intens	ity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	4.06	3.25	2.72	2.35	2.07	1.85	1.67	1.53	1.41	1.31	1.23	1.15
2	4.84	3.87	3.24	2.80	2.47	2.22	2.02	1.85	1.71	1.59	1.49	1.40
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5.65	4.58	3.88	3.38	3.00	2.70	2.46	2.26	2.10	1.96	1.84	1.73
10	6.22	5.08	4.32	3.78	3.36	3.04	2.78	2.56	2.38	2.22	2.09	1.97
25	6.88	5.62	4.79	4.21	3.77	3.42	3.14	2.91	2.72	2.55	2.41	2.28
50	7.32	6.00	5.14	4.52	4.06	3.70	3.41	3.17	2.96	2.79	2.63	2.50
100	7.78	6.35	5.44	4.81	4.33	3.96	3.66	3.41	3.20	3.02	2.87	2.73

Tc = time in minutes. Values may exceed 60.

P-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Morgantown Road\Hydraflow Rev 1\Morgantown Precip.pc

		F	tainfall F	Precipitat	tion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	2.68	3.22	0.00	4.04	4.72	5.70	6.53	7.41
SCS 6-Hr	1.88	2.27	0.00	2.83	3.28	3.92	4.43	4.98
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00