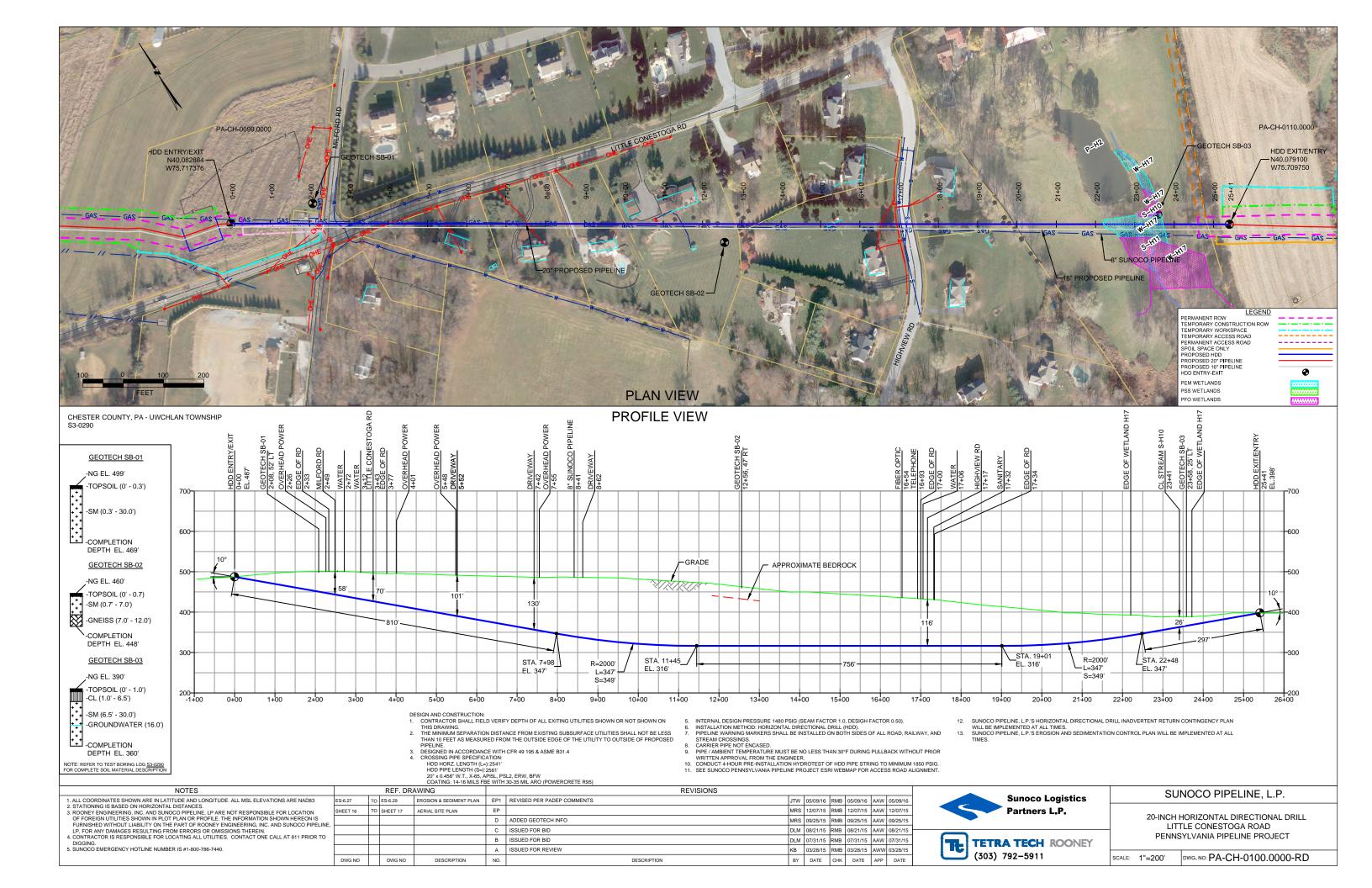
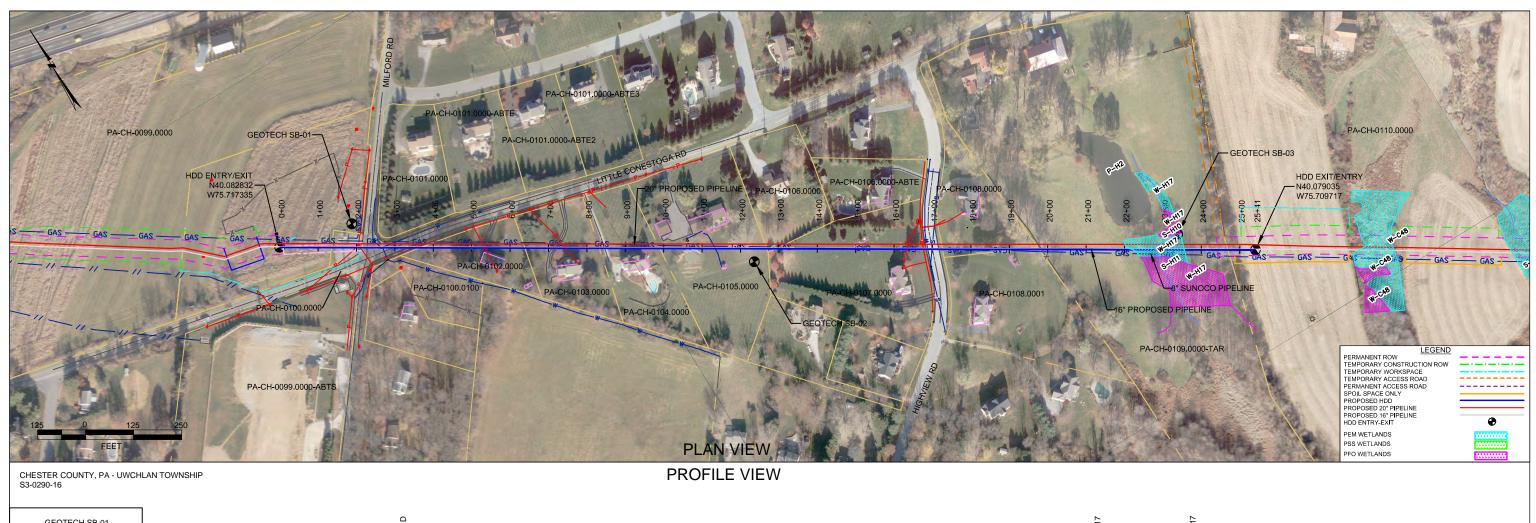
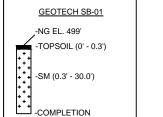
HDD PA-CH-0100.0000-RD (W-H17, S-H10)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and in this case that threat is considered to be low. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 2220 feet northwest of the northwestern most boundary of wetland H17. The other entry/exit point is 171 feet southeast of the southeastern most boundary of wetland H17. The drill will pass 50 feet under the northwestern most boundary of the wetland and 20 feet under the southeastern most boundary of the wetland. Stream H10 runs through this wetland, 200 feet northwest of the southeastern entry/exit point. The drill will pass 26 feet under the stream. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be silty sand and gneiss.







GEOTECH SB-02 -NG EL. 460'

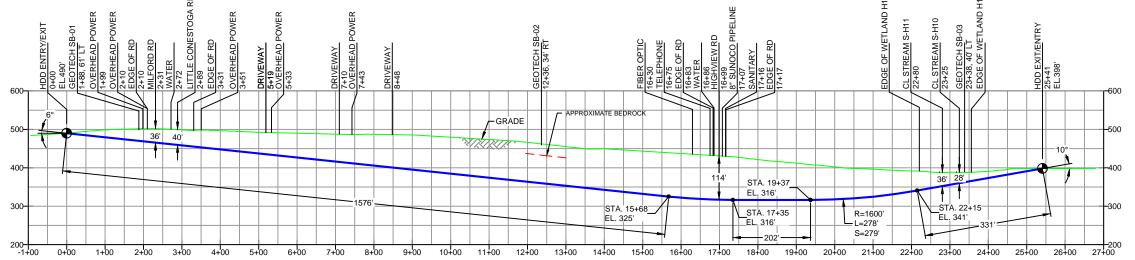
DEPTH EL. 469'

-TOPSOIL (0' - 0.7) -SM (0.7' - 7.0') -GNEISS (7.0' - 12.0')

> -COMPLETION DEPTH EL. 448' GEOTECH SB-03

-NG EL. 390' -TOPSOIL (0' - 1.0') -CL (1.0' - 6.5') -SM (6.5' - 30.0') -GROUNDWATER (16.0') -COMPLETION

DEPTH EL. 360' NOTE: REFER TO TEST BORING LOG <u>S3-0290</u> FOR COMPLETE SOIL MATERIAL DESCRIPTION



DESIGN AND CONSTRUCTION:

- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXITING UTILITIES SHOWN OR NOT SHOWN ON
- THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS
 THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED
 PIPELINE
 DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
 CROSSING PIPE SPECIFICATION:
 HDD HORZ. LENGTH (L=):2541'
 HDD PIPE LENGTH (S=):2557'
 16" x 0.438" W.T., X-70, APISL, PSL2, ERW, BFW
 COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95)

- 5. INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0. DESIGN FACTOR 0.50).
- INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND
- STREAM CROSSINGS.

 8. CARRIER PIPE NOT ENCASED.

 9. PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.

 10. CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.

 11. SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.

- 12. SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.

 13. SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL

					COATING: 14-16 MILS FE	BE WITH	30-35 MIL ARO (POWERCRETE R95)						
ſ	NOTES			REF. DR	AWING	REVISIONS							
ſ	1. ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83	ES-6.27	то	ES-6.29	EROSION & SEDIMENT PLAN								
	STATIONING IS BASED ON HORIZONTAL DISTANCES. ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION	SHEET 16	то	SHEET 17	AERIAL SITE PLAN								
	OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE.					EP1	REVISED PER PADEP COMMENTS	JTW	05/10/16	RMB	05/10/16	AAW	05/10/16
	LP, FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.					EP		MRS	03/15/16	RMB	03/15/16	AAW	03/15/16
	 CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING. 					В	ADDED GEOTECH INFO	MRS	09/25/15	RMB	09/25/15	AAW	09/25/15
	5. SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.					Α	ISSUED FOR BID	MRS	08/31/15	RMB	08/31/15	AWW	08/31/15
		DWG NO		DWG NO	DESCRIPTION	NO.	DESCRIPTION	BY	DATE	СНК	DATE	APP	DATE



(303) 792-5911

16-INCH HORIZONTAL DIRECTIONAL DRILL LITTLE CONESTOGA ROAD PENNSYLVANIA PIPELINE PROJECT

SUNOCO PIPELINE, L.P.

TETRA TECH ROONEY SCALE: 1"=250'

DWG. NO: PA-CH-0100.0000-RD-16



LEGEND:

© Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS
HDD S3-0290
CHESTER COUNTY, UPPER UWCHLAN TOWNSHIP, PA
SUNOCO PENNSYLVANIA PIPELINE PROJECT



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENN	SYLV	ANIA P	IPELINE PROJECT		Project	No.: 103IP3406	
Project Location:	MILFORD AND L	ITTLE	CONE	STOGA ROADS, DOWNINGTOWN, PA		Page 1	of 1	
HDD No.:	S3-0290			Dates(s) Drilled: 05-20-15	Inspector:	E. WA	ТТ	
Boring No.:	SB-01			Drilling Method: SPT - ASTM D1586	Driller:	S. HO	FER	
Drilling Contractor:	HAD DRILLING			Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	30.0		
Boring Location Coor	dinates:			40° 4' 57.699" N	75° 42' 59.880" W			
								\neg

Donning	Location	Coordii	iaics.				40 4 57.099 N 75 42 59.000 W					
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	Recov.	Strata	Description of Materials	6"	ncreme	ent Blo	ws *	N
No.	From	То	From	То	Re (i	(USCS)	Boompton of materials		110101110	,		
			0.0	0.3			TOPSOIL (3")					
1	3.0	5.0	0.3		19		LIGHT BROWN AND WHITE FINE TO MEDIUM SAND WITH SOME	16	30	21	18	51
							SILT.					
2	8.0	9.5			13		VARI-COLORED FINE TO MEDIUM SAND WITH SOME SILT, TRACE	3	20	50		70
							FINE GRAVEL.					
3	13.0	14.9			22		VARI-COLORED FINE TO MEDIUM SAND WITH SOME SILT, TRACE	3	9	22	50/5"	31
						SM	FINE GRAVEL.					
4	18.0	18.9			7	SIVI	LIGHT BROWN AND WHITE FINE TO MEDIUM SAND WITH SOME	6	50/5"			>50
							SILT, AND A LITTLE FINE QUARTZ GRAVEL.					
5	23.0	23.8			8		LIGHT BROWN AND WHITE FINE TO MEDIUM SAND WITH SOME	28	50/3"			>50
							SILT, AND A LITTLE FINE QUARTZ GRAVEL. (USCS: SM).					
6	28.0	28.7			5		LIGHT BROWN AND WHITE FINE TO MEDIUM SAND WITH SOME	5	50/2"			>50
				30.0			SILT, AND A LITTLE FINE QUARTZ GRAVEL.					
							AUGERED TO 30'.					
							CAVED AND DRY AT 28.5'.					
							SAMPLES 2 THRU 6 ARE HIGHLY DECOMPOSED ROCK SOILS.					
							(SOILS THAT HAVE BEEN WEATHERED IN-PLACE FROM ROCK)					
						-						<u> </u>
												

Notes/Comments:

Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA PI	PELINE PROJECT		Project No.: 103IP3406
Project Location:	465 LITTLE CONESTOGA RO	AD, DOWNINGTOWN, PA	Page 1 of 1	
HDD No.:	S3-0290	Dates(s) Drilled: 05-27-15	Inspector:	E. WATT
Boring No.:	SB-02	Drilling Method: SPT - ASTM D1586	Driller:	S. HOFFER
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): NOT ENCOUNTERED	Total Depth (ft):	12.0
Boring Location Coordin	nates:	40° 4' 51.263" N	75° 42' 49.239" W	V

Bonnig	Location	n Oooran	iatoo.				70 12 10.200 11					
Sample	Sample	Depth (ft)	Strata D	Depth (ft)	§ (=	Strata	Description of Materials	0" 1		DI	*	N
No.	From	То	From	То	Recov. (in)	(USCS)	Description of Materials	6" 1	ncreme	ent Bio	ws ^	N
			0.0	0.7			TOPSOIL (8")					
1	3.0	5.0	0.7		16		BROWN AND GRAY FINE TO MEDIUM SAND AND SILT, WITH A LITTLE	13	22	28	23	50
						SM	FINE TO COARSE UNWEATHERED ROCK GRAVEL (GNEISS).					
				7.0			(USCS: SM).					
							AUGER REFUSAL AT 7'.					
							ROCK CORING					
RUN 1	7.0	12.0	7.0	11.0	60	×	VERY INTENSELY FRACTURED GRAY GNEISS, SOME OXIDATION.	TCR: 1	00%, S0	CR: 35%	, RQD:	20%
			11.0	11.9		ROCK	MODERATELY FRACTURED GRAY GNEISS.					
			11.9	12.0		~	VERY INTENSELY FRACTURED GRAY GNEISS.					
							CORE TESTING RESULTS (RUN 1, DEPTH 7.7'):					
							COMPRESSIVE STRENGTH: 5,690 PSI					
							UNIT WEIGHT: 172.8 PCF					
							CORE TESTING RESULTS (RUN 1, DEPTH 11.5'):					
							COMPRESSIVE STRENGTH: 3,360 PSI					
							UNIT WEIGHT: 160.8 PCF					
							OBSTRUCTION AT 9' PREVENTED ROCK CORE BARRELL FROM	-				
							BEING ADVANCED TO START RUN 2.	-				
								1				

Notes/Comments:

Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments. N: Number of blows to drive spoon from 6" to 18" interval.



TETRA TECH

240 Continental Drive, Suite 200 Newark, Delaware 19713 302.738.7551 fax: 302.454.5988

TEST BORING LOG

Project Name:	SUNOCO PENNSYLVANIA PI		Project No.: 103IP3406	
Project Location:	CH-0109, DOWNINGTOWN, P	'A		Page 1 of 1
HDD No.:	S3-0290	Dates(s) Drilled: 05-27-15	Inspector:	J. COSTELLO
Boring No.:	SB-03	Drilling Method: SPT - ASTM D1586	Driller:	GREG
Drilling Contractor:	HAD DRILLING	Groundwater Depth (ft): 16.0	Total Depth (ft):	30.0
Boring Location Coordi	nates:	40° 4' 45.950" N	75° 42' 36.827" W	I

Sample			Depth (ft)	Recov. (in)	Strata	Description of Materials	6" 1	ncreme	nt Dia	*	N.	
No.	From	То	From	То	Rec (ir	(USCS)	Description of Materials	0 1	ncieme	siit Di0	ws	N
			0.0	1.0			TOPSOIL (12")					
1	3.0	5.0	1.0		24	CL	MOTTLED BROWN AND GRAY SILTY CLAY WITH A LITTLE FINE	5	7	9	11	16
				6.5		CL	SAND, TRACE FINE ROOTS. (USCS: CL).					
2	8.0	10.0	6.5		20		OR WEATHERED TO A VARI-COLORED FINE SAND AND SILT, TRACE		4	6	7	10
							FINE UNWEATHERED GRAVEL. (USCS: SM).					
3	13.0	15.0			23		DR WEATHERED TO A WHITE, GRAY, TAN FINE TO MEDIUM SAND AND	1	4	6	8	10
							SILT, TRACE TO A LITTLE UNWEATHERED FINE GRAVEL.					
4	18.0	20.0			24	CN4	DR WEATHERED TO A WHITE, GRAY, TAN FINE SAND AND SILT,	2	3	7	12	10
						SM	TRACE UNWEATHERED FINE GRAVEL.					
5	23.0	25.0			24		DR WEATHERED TO A WHITE, GRAY, TAN FINE SAND AND SILT,	1	4	6	10	10
							TRACE UNWEATHERED FINE GRAVEL.					
6	28.0	30.0			24		DR WEATHERED TO A WHITE, GRAY, TAN FINE SAND AND SILT,	1	1	6	4	7
				30.0			TRACE UNWEATHERED FINE GRAVEL. (USCS: SM).					
							WET ON SPOON AT 18'.					
							WATER LEVEL THROUGH AUGERS AT 16'.					
							DRY AND CAVED AT 14'.					
												
												-

Notes/Comments:

Pocket Pentrometer Testing

S1: > 4TSF

DR: DECOMPOSED ROCK

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.

N: Number of blows to drive spoon from 6" to 18" interval.

GEOTECHNICAL LABORATORY TESTING SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0290

	Test				Water	Percent	Atterburg	Limits (AS	STM D4318)	USCS
HDD	Boring	Sample	Depth of S	Sample (ft.)	Content, %	Silts/Clays, %	Liquid	Plastic	Plasticity	Classif.
No.	No.	No.	From	To	(ASTM D2216)	(ASTM D1140)	Limit, %	Limit, %	Index, %	(ASTM D2487)
		1	3.0	5.0	11.7	27.8	-	-	-	-
		2	8.0	9.5	6.3	22.0	-	-	-	-
	SB-01	4	18.0	18.9	7.4	31.5	-	-	-	-
		5	23.0	23.8	6.6	30.5	29	24	5	SM
		6	28.0	28.7	7.4	37.7	-	-	-	-
S3-0290	SB-02	1	3.0	5.0	13.0	38.8	30	24	6	SM
		1	3.0	5.0	20.3	83.0	41	23	18	CL
		2	8.0	10.0	21.5	48.0	36	26	10	SM
	SB-03	3	13.0	15.0	23.9	42.1	-	-	-	-
		5	23.0	25.0	22.9	47.6	-	-	-	-
		6	28.0	30.0	22.9	47.6	55	37	18	SM

	Rock Core Testing Results											
Boring	Core	Unit										
No.	Run	Weight (pcf)										
SB-02	1	7.7	5,690 172.8									
SB-02	1	11.5	3,360	160.8								

Notes:

1) Sample depths based on feet below grade at time of exploration.

ROCK CORE DESCRIPTION SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0290

			Core De	epth (ft)				Dept	h (ft)			Bedding		
Location	Boring No.	Core Run	From	То	TCR (%)	SCR (%)	RQD (%)	From	То	Weathering	Classification	Thickness (ft)	Color	Discontinuity Data
								7	8	Moderate	Gneiss	Massive	_	Fractures ranging from 0° to 45°, Avg. 29°
S3-0290	SB-2	1	7	12	100	35	20	8	10	Moderate	Metavolcanic inclusion	2	Gray, brown, black	Fractures ranging from 4° to 75°, Avg. 49°
								10	12	Moderate	Gneiss	Massive	Light gray	Fractures ranging from 30° to 75°, Avg. 51°

REGIONAL GEOLOGY SUMMARY SUNOCO PENNSYLVANIA PIPELINE PROJECT HDD S3-0290

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	GENERAL TOPOGRAPHIC SETTING	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
		SB-01	Complete falsia maisa dagladas	Gently sloping to the north				Ranges from 4 to 50 ft bgs, Avg. 27 ft bgs (.25 mile radius)	
\$3-0290	Little Conestoga S3-0290 Road, Downingtown	SB-02	Graphitic felsic gneiss - Includes Pickering Gneiss and small areas of marble; dominantly quartz and feldspar with varying amounts of graphite and various metamorphic minerals; medium grained, light to dark gray and greenish	Generally level, slightly sloping to the south		Graphitic gneiss	Unknown	Ranges from 10 to 50 ft bgs, Avg. 29 ft bgs (.25 mile radius)	
		SB-03	gray; sedimentary origin.	Generallly level				Ranges from 10 to 50 ft bgs, Avg. 31 ft bgs (.25 mile radius)	

<u>Note</u>: Source of well log data - http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm. All other sources as referenced in comments section.

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u>	N (blows)*	Darticle Si	ize Identifica:	tion
Very Loose	5 or less	Boulders	8 in. diamet	
Loose	6 to 10	Cobbles	3 to 8 in. di	
Medium Dense	11 to 30			
Dense	31to 50	Gravel	Coarse (C)	3 in. to ¾ in. sieve
Very Dense	51 or more		Fine (F)	¾ in. to No. 4 sieve
, =		Sand	Coarse (C)	No. 4 to No. 10 sieve
				(4.75mm-2.00mm)
Relative Proportion	ons		Medium	No. 10 to No. 40 sieve
Description Term	<u>Percent</u>		(M)	(2.00mm – 0.425mm)
Trace	1 - 10		Fine (F)	No. 40 to No. 200 sieve
Little	11 - 20			(0.425 – 0.074mm)
Some	21 - 35	Silt/Clay	Less Than a	No. 200 sieve (<0.074mm)
And	36 - 50	- ,, ,		,

COHESIVE SOILS

(Silt, Clay & Combinations)

Consistency	N (blows)*	Plasticity	
Very Soft	3 or less	<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
Soft	4 to 5	None to Slight	0 - 4
Medium Stiff	6 to 10	Slight	5 - 7
Stiff	11 to 15	Medium	8- 22
Very Stiff	16 to 30	High to Very High	> 22
Hard	31 or more	, ,	

ROCK (Rock Cores)

Rock	Rock		
Quality Designation	Quality <u>Descripti</u>		
(RQD), %	<u>on</u>		
0-25	Very Poor		
25-50	Poor		
50-75	Fair		
75-90	Good		
90-100	Excellent		

*N - Standard Penetration Resistance. Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

Major Divisions		Group Symbols	Typical Descriptions	Laboratory Classifications					
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Gravels More than half of coarse fraction is larger than No. 4 sieve size	Clean gravel (Little or no fines)	GW	Well-graded gravels, gravel- sand mixtures, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows: Less than 5 percent GW, GP, SW, SP More than 12 percent GM, GC, SM, SC	nbols ⁽¹⁾	$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 4: $C_{c=\frac{1}{10}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3	
			GP	Poorly graded gravels, gravel- sand mixtures, little or no fines		percent GW, GP, SW, SP percent GM. GC, SM, SC percent Borderline cases requiring dual symbols ⁽¹⁾	Not meeting C_{u} or C_{c} requirements for GW		
		Gravel with fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures			Atterberg limits below A Line or I p less than 4	Limits plotting in hatched zone with ! p between 4 and 7 are	
			GC	Clayey gravels, gravel-sand-clay mixtures			Atterberg limits above A line with I p greater than 7	borderline cases requiring use of dual symbols	
	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	sw	Well graded sands, gravely sands, little or no fines			$C_{u=\frac{D_{60}}{D_{10}}}$ greater than 6: $C_{c=\frac{1}{L}}$	(D ₃₀)2 D ₁₀ x D ₆₀ between 1 and 3	
			SP	Poorly graded sands, gravelly sands, little or no fines		Less than 5 More than 12 5 to 12	Not meeting C_u or C_c requirements for SW		
	half of coa	Sands with fines (Appreciable amount of fines)	SM	Silty sands, sand- silt mixtures	Determ Jepending		Atterberg limits below A Line or I p less than 4	Limits Plotting in hatched	
	(More than I		SC	Clayey sands, sand-clay mixtures			Atterberg limits above A line with I p greater than 7	zone with I p between 4 and 7 are borderline cases requiring use of dual symbols	
Major Divisions Group Symbols		Typical Descriptions		For soils p When w _{l.}	lotting nearly is near 50 us	on A line use dual symbols i.e ., l p e CL-CH or ML-MH. Take near as	= 29.5, w _L =60 gives CH-MH. ± 2 percent.		
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	Silts and clays (Liquid limit less than 50)	ML	sands, rock fl	s and very fine lour, silty or clayey r clayey silts with iy	60	A Line:			
		CL	plasticity, gra	ys of low to medium velly clays , sandy ays, lean clays	5(U Line:	1 1	Or I	
		OL	Organic silts clays of low	and organic silty plasticity	% (PI), %	0		, or Or	
	Silts and Clays (Liquid limit greater than 50)	MH		s, micaceous or s fine sandy or silty silts	Plasticity Index (PI), %		Juge / F	MH or OH	
		СН	Inorganic clar	ys of high plasticity,	Plasi		Character		
		ОН	Organic clays	s of medium to high anic silts	7		ML or OL	0 70 80 90 100	
	Highly organic soils	Pt	Peat and othe	er highly organic			Liquid Limit (LL		

⁽¹⁾ Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.