

HDD PA-CH-0088.0000-RD (W-Q76, S-Q83, W-Q77, and S-Q86)

Given the design, the threat of inadvertent return has been reduced to the maximum extent practicable and based on geotechnical data, the threat will otherwise be considered to be low. However, due to inadvertent return on a previous HDD in this area, the threat in this area is considered to be medium. The previous inadvertent return was 100 gallons of drilling fluid that penetrated the stream. Sand bags, a vacuum truck, wheel barrows, and hand tools were used to clean up the spill. There was no erosion and the land was restored to its original condition with 80% of the vegetation returning. For this reason, we recommend additional inspection while the drill is being performed. Implementing this design, along with adherence to the Pennsylvania Pipeline Project Inadvertent Return Contingency Plan will ensure inadvertent impacts, if they were to occur, are also minimized to the maximum extent.

The drill will enter/exit 550 feet northwest of wetland Q76. The drill will pass 40 feet under the northwestern most boundary of the wetland and 33 feet under the southeastern most boundary of the wetland. Stream Q83 runs along the southeastern most boundary of this wetland. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be silty sand and felsic gneiss.

The drill will enter/exit 1085 feet northwest of wetland Q77 and enter/exit 1480 feet southeast of this wetland. The drill will pass 30 feet under the northwestern most boundary of the wetland and 30 feet under the southeastern most boundary of the wetland. Stream Q86 runs along the southeastern most boundary of this wetland. Using the results of the geotechnical investigation, as well as several other data points, the entry/exit, angles, and depths have been configured to pass through the best substrates while maintaining pipe integrity (e.g., no large bends). The majority of the substrate that will be passed through is estimated to be silty sand and felsic gneiss.



Legend

- HDD Path
- Proposed Centerline

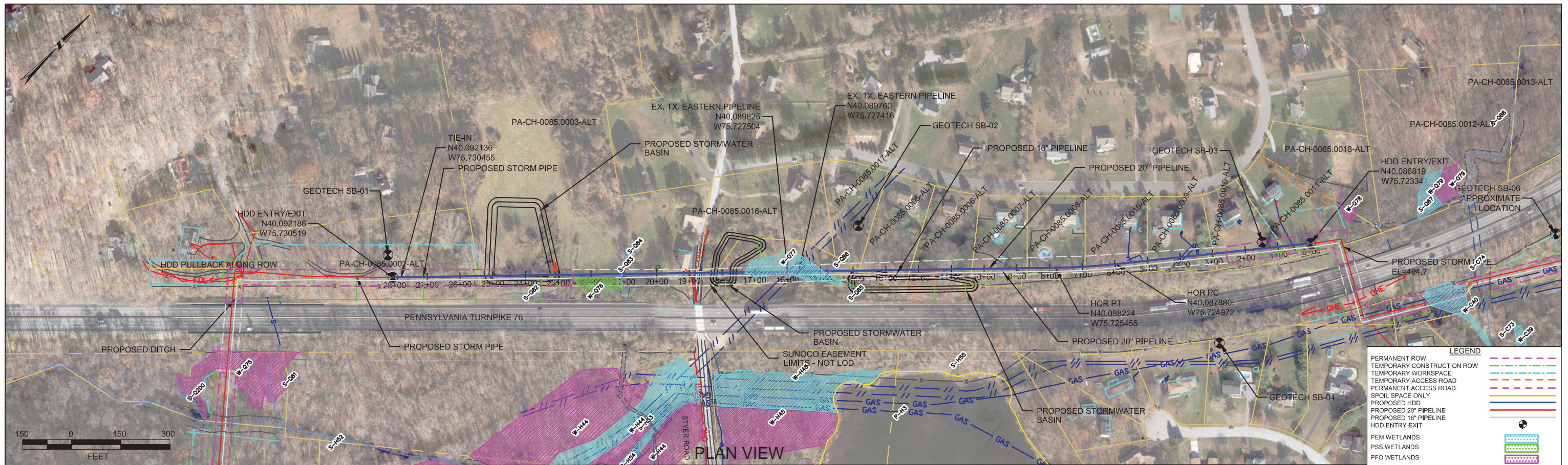
DRAWN	CHECKED	APPROVED	DATE	REV NO.	DESCRIPTION
AW	RB	RB	11/21/16	A	ISSUED FOR REVIEW

PREPARED BY:
 TETRA
 (303) 79

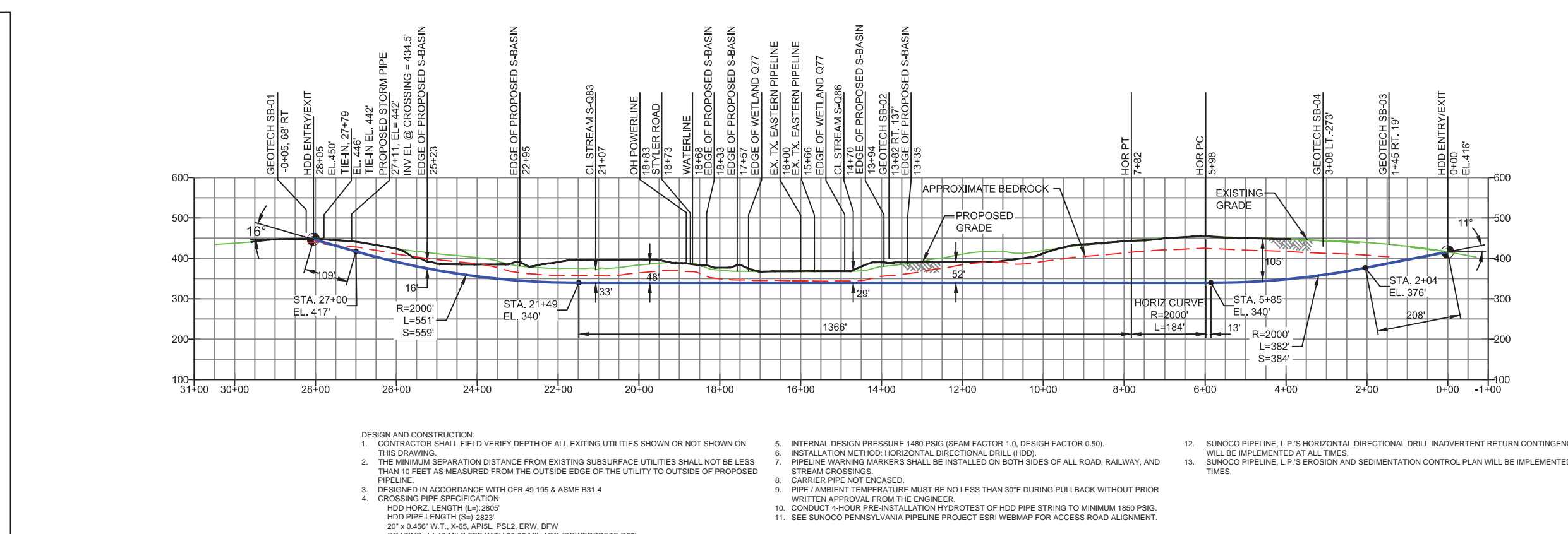
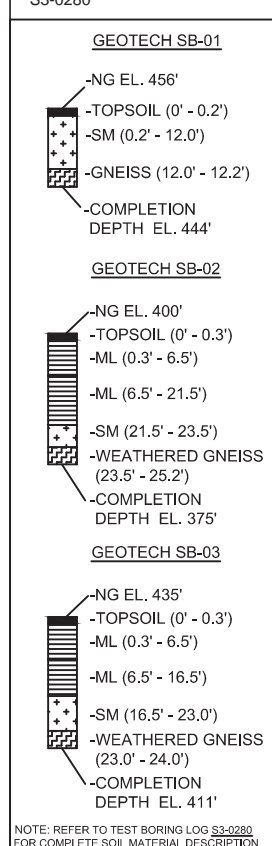
Sunoco Pipeline L.P.
 PENNSYLVANIA PIPELINE PROJECT
 INADVERTENT RETURN ASSESSMENTS

DRAWN: AW	CHECKED: RB
DATE: 11/21/16	SCALE: AS SHOWN

PA-CH-0088-RD-HDD



CHASTER COUNTY, PA - UPPER UWCHLAN TOWNSHIP
S3-0280



DESIGN AND CONSTRUCTION:

- CONTRACTOR SHALL FIELD VERIFY DEPTH OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN ON THIS DRAWING.
- THE MINIMUM SEPARATION DISTANCE FROM EXISTING SUBSURFACE UTILITIES SHALL NOT BE LESS THAN 10 FEET AS MEASURED FROM THE OUTSIDE EDGE OF THE UTILITY TO OUTSIDE OF PROPOSED PIPELINE.
- DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
- CROSSING PIPE SPECIFICATION:
 HDD HORZ. LENGTH (L-): 2805'
 HDD PIPE LENGTH (S-): 2823'
 20" x 0.456" W.T., X-65, API5L, PSL2, ERW, BFW
 COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95)
- INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
- INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
- PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
- CARRIER PIPE NOT ENCASED.
- PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
- CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
- SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
- SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY WILL BE IMPLEMENTED AT ALL TIMES.
- SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.

NOTE: REFER TO TEST BORING LOG S3-0280 FOR COMPLETE SOIL MATERIAL DESCRIPTION

NOTES

- ALL COORDINATES SHOWN ARE IN LATITUDE AND LONGITUDE. ALL MSL ELEVATIONS ARE NAD83
- STATIONING IS BASED ON HORIZONTAL DISTANCES.
- ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP ARE NOT RESPONSIBLE FOR LOCATION OF FOREIGN UTILITIES SHOWN IN PLOT PLAN OR PROFILE. THE INFORMATION SHOWN HEREON IS FURNISHED WITHOUT LIABILITY ON THE PART OF ROONEY ENGINEERING, INC. AND SUNOCO PIPELINE, LP. FOR ANY DAMAGES RESULTING FROM ERRORS OR OMISSIONS THEREIN.
- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

REF. DRAWING		REVISIONS	
DWG NO	DESCRIPTION	NO.	DESCRIPTION
ES-6.24	EROSION & SEDIMENT PLAN		
SHEET 14	AERIAL SITE PLAN		
		EP2	REVISED PER PADEP COMMENTS RECEIVED 09-06-16
		EP1	REVISED FOR PADEP COMMENTS
		EP	
		0	ISSUED FOR CONSTRUCTION

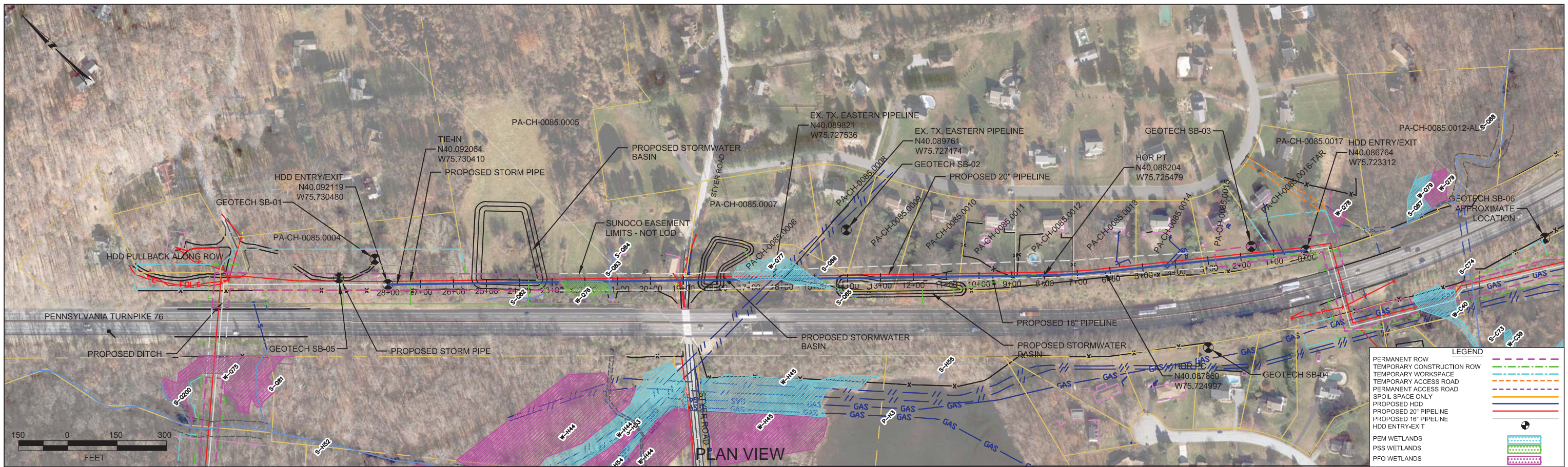
Sunoco Logistics Partners L.P.

TETRA TECH ROONEY
(303) 792-5911

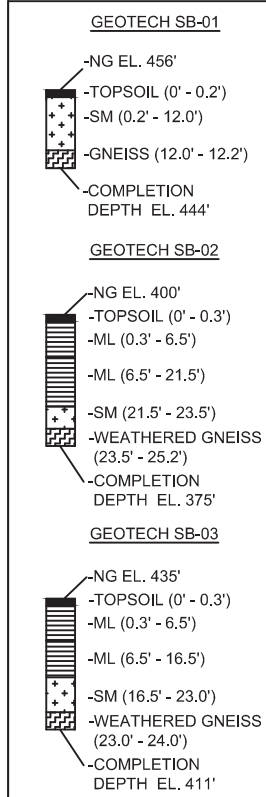
SUNOCO PIPELINE, L.P.

20-INCH HORIZONTAL DIRECTIONAL DRILL
PENNSYLVANIA TURNPIKE 76
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=300'
DWG. NO: PA-CH-0088.0000-RD

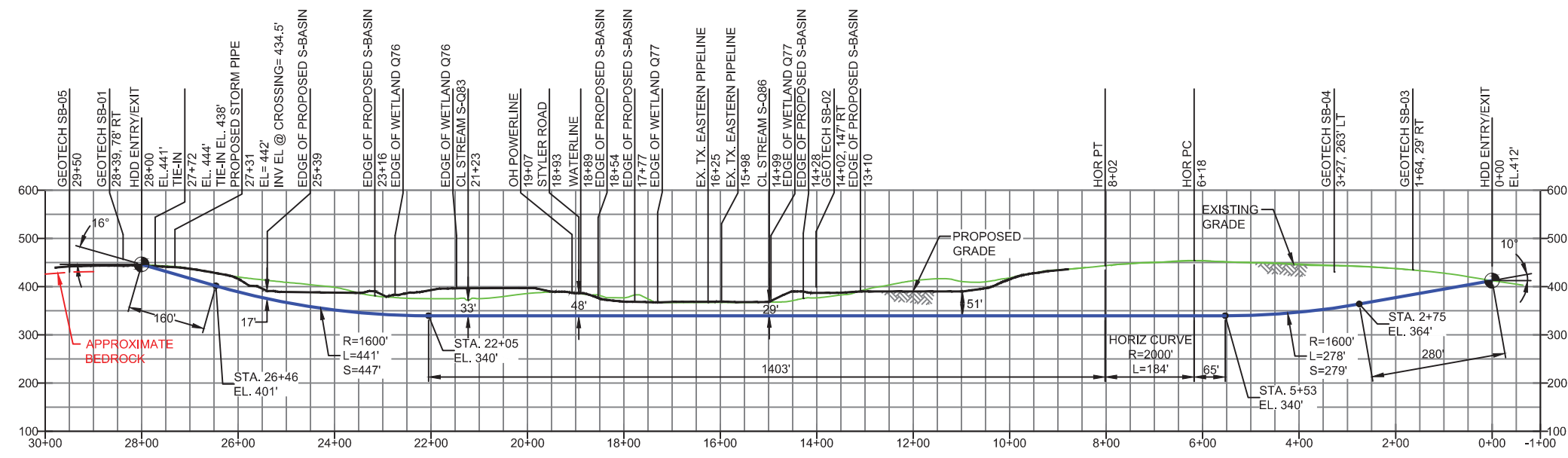


CHASTER COUNTY, PA - UPPER UWCHLAN TOWNSHIP
S3-0280-16

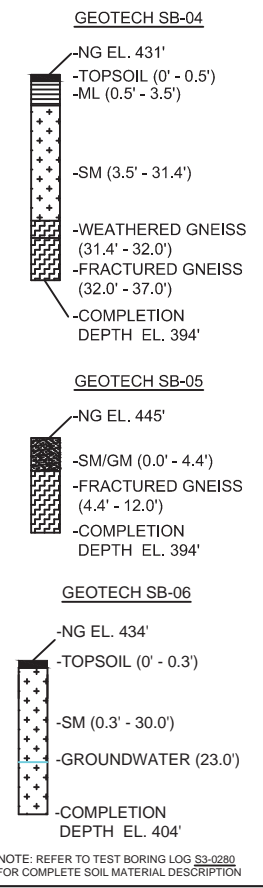


NOTE: REFER TO TEST BORING LOG S3-0280 FOR COMPLETE SOIL MATERIAL DESCRIPTION

PROFILE VIEW



- DESIGN AND CONSTRUCTION:
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 - DESIGNED IN ACCORDANCE WITH CFR 49 195 & ASME B31.4
 - CROSSING PIPE SPECIFICATION:
HDD HORZ. LENGTH (L=): 2800'
HDD PIPE LENGTH (S=): 2819'
16" x 0.438" W.T., X-70, API 5L, PSL2, ERW, BFW
COATING: 14-16 MILS FBE WITH 30-35 MIL ARO (POWERCRETE R95)
 - INTERNAL DESIGN PRESSURE 1480 PSIG (SEAM FACTOR 1.0, DESIGN FACTOR 0.50).
 - INSTALLATION METHOD: HORIZONTAL DIRECTIONAL DRILL (HDD).
 - PIPELINE WARNING MARKERS SHALL BE INSTALLED ON BOTH SIDES OF ALL ROAD, RAILWAY, AND STREAM CROSSINGS.
 - CARRIER PIPE NOT ENCASED.
 - PIPE / AMBIENT TEMPERATURE MUST BE NO LESS THAN 30°F DURING PULLBACK WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
 - CONDUCT 4-HOUR PRE-INSTALLATION HYDROTEST OF HDD PIPE STRING TO MINIMUM 1850 PSIG.
 - SEE SUNOCO PENNSYLVANIA PIPELINE PROJECT ESRI WEBMAP FOR ACCESS ROAD ALIGNMENT.
 - SUNOCO PIPELINE, L.P.'S HORIZONTAL DIRECTIONAL DRILL INADVERTENT RETURN CONTINGENCY PLAN WILL BE IMPLEMENTED AT ALL TIMES.
 - SUNOCO PIPELINE, L.P.'S EROSION AND SEDIMENTATION CONTROL PLAN WILL BE IMPLEMENTED AT ALL TIMES.



NOTE: REFER TO TEST BORING LOG S3-0280 FOR COMPLETE SOIL MATERIAL DESCRIPTION

NOTES

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- CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UTILITIES. CONTACT ONE CALL AT 811 PRIOR TO DIGGING.
- SUNOCO EMERGENCY HOTLINE NUMBER IS #1-800-786-7440.

REF. DRAWING			REVISIONS		
DWG NO	DWG NO	DESCRIPTION	NO.	DESCRIPTION	DATE
ES-6.24	TO ES-6.26	EROSION & SEDIMENT PLAN			
SHEET 14	TO SHEET 15	AERIAL SITE PLAN			
			EP2	REVISED PER PADEP COMMENTS RECEIVED 09-06-16	MRS 10/07/16 RMB 10/07/16 AAW 10/07/16
			EP1	REVISED PER PADEP COMMENTS	MRS 05/09/16 RMB 05/09/16 AAW 05/09/16
			EP		MRS 02/26/16 RMB 02/26/16 AAW 02/26/16
			0	ISSUED FOR CONSTRUCTION	MRS 02/19/16 RMB 02/19/16 AAW 02/19/16

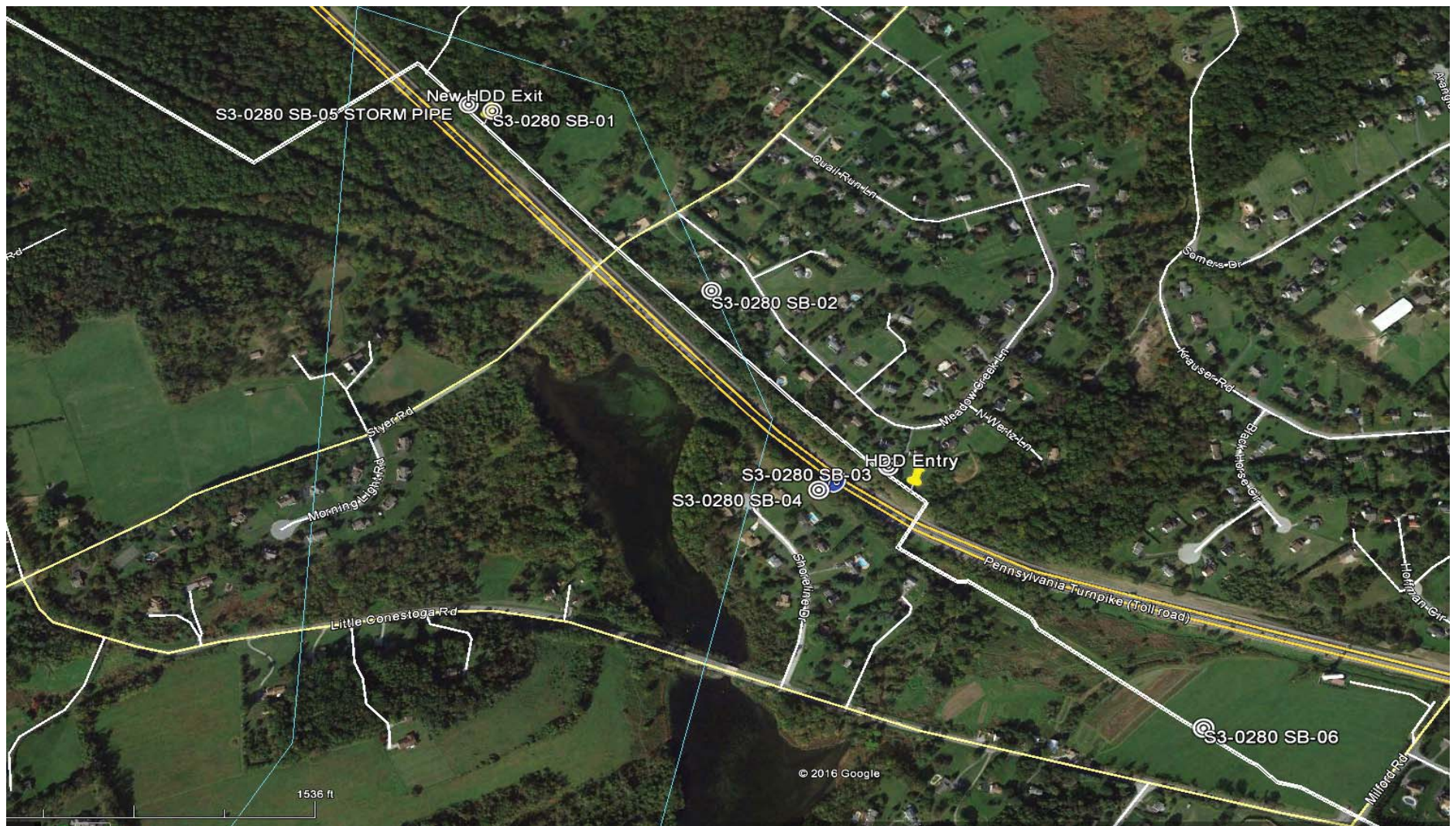
Sunoco Logistics Partners L.P.

TETRA TECH ROONEY
(303) 792-5911

SUNOCO PIPELINE, L.P.

16-INCH HORIZONTAL DIRECTIONAL DRILL
PENNSYLVANIA TURNPIKE 76
PENNSYLVANIA PIPELINE PROJECT

SCALE: 1"=300'
DWG. NO: PA-CH-0088.0000-RD-16



LEGEND:

⊙ Geotechnical Soil Boring (SB) Locations



GEOTECHNICAL BORING LOCATIONS
 HDD S3-0280
 CHESTER COUNTY, UPPER UWCHLAN TOWNSHIP, PA
 SUNOCO PENNSYLVANIA PIPELINE PROJECT



TETRA TECH

240 Continental Drive, Suite 200
 Newark, Delaware 19713
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 fax: 302.454.5988

TEST BORING LOG

Project Name: SUNOCO PENNSYLVANIA PIPELINE PROJECT			Project No.: 103IP3406		
Project Location: SHORELINE DRIVE, DOWNINGTOWN, PA			Page 1 of 1		
HDD No.: S3-0280		Dates(s) Drilled: 05-21-15		Inspector: E. WATT	
Boring No.: SB-04		Drilling Method: SPT - ASTM D1586		Driller: S. HOFFER	
Drilling Contractor: HAD DRILLING		Groundwater Depth (ft): NOT ENCOUNTERED		Total Depth (ft): 37.0	
Boring Location Coordinates:			40° 5' 12.456" N		75° 43' 29.328" W

Sample No.	Sample Depth (ft)		Strata Depth (ft)		Recov. (in)	Strata (USCS)	Description of Materials	6" Increment Blows *				N	
	From	To	From	To									
			0.0	0.5			TOPSOIL (6")						
			0.5	3.5		ML	BROWN SILTY CLAY WITH SOME FINE SAND.						
1	3.0	5.0	3.5		24	SM	ORANGE BROWN FINE TO MEDIUM SAND WITH SOME SILT, TRACE FINE GNEISS GRAVEL.	9	7	8	10	15	
2	8.0	10.0			13		LIGHT BROWN TO GREENISH BROWN FINE TO MEDIUM SAND WITH SOME SILT, TRACE FINE OXIDIZED GNEISS GRAVEL.	9	32	50		82	
3	13.0	14.9			17		BROWN AND LIGHT GRAY FINE TO MEDIUM SAND WITH SOME SILT, TRACE FINE GNEISS GRAVEL (USCS: SM).	11	43	34	50/5"	77	
4	18.0	18.9			8		DECOMPOSED GNEISS WEATHERED TO A FINE TO MEDIUM SAND WITH A LITTLE SILT, WITH A LITTLE UNWEATHERED F-C GRAVEL.	3	50/5"			>50	
5	23.0	23.6			5		DECOMPOSED GNEISS WEATHERED TO A FINE TO MEDIUM SAND WITH A LITTLE SILT, WITH A LITTLE UNWEATHERED F-C GRAVEL.	12	50/1"			>50	
6	28.0	28.6			6		DECOMPOSED GNEISS WEATHERED TO A FINE TO MEDIUM SAND WITH SOME SILT, WITH A LITTLE UNWEATHERED F-C GRAVEL.	15	50/1"			>50	
7	31.4	32.0	31.4	32.0	7		WHITE AND LIGHT GRAY PARTIALLY WEATHERED GNEISS.	3	50/2"			>50	
							AUGER REFUSAL AT 31.4'.						
							ROCK CORING						
RUN 1	32.0	32.7			8.5		ROCK	MODERATELY FRACTURED GNEISS	TCR: 100%, SCR: 94%, RQD: 94%				
RUN 2	32.7	34.9			52	VERTY INTENSELY FRACTURED GNEISS.		TCR: 100%, SCR: 19%, RQD: 13%					
	34.9	35.8				MODERATELY FRACTURED GNEISS							
	35.8	37.0				VERTY INTENSELY FRACTURED GNEISS.							
						USED HIGH VOLUME OF WATER DURING CORING. ALL WATER WAS LOST IN FORMATION (I.E., NO WATER CAME OUT OF AUGERS).							
						CORE TESTING RESULTS (RUN 2, DEPTH 35-35.7'):							
						COMPRESSIVE STRENGTH: 1,520 PSI							
						UNIT WEIGHT: 173.8 PCF							

Notes/Comments:
Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.
 N: Number of blows to drive spoon from 6" to 18" interval.



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TEST BORING LOG

Project Name: SUNOCO PENNSYLVANIA PIPELINE PROJECT			Project No.: 103IP3406		
Project Location: GREENRIDGE ROAD, GLENMORE, PA			Page 1 of 1		
HDD No.: S3-0280		Dates(s) Drilled: 02-23-16		Inspector: E. WATT	
Boring No.: SB-05		Drilling Method: SPT - ASTM D1586		Driller: D. BOLZE	
Drilling Contractor: CGC		Groundwater Depth (ft): NOT ENCOUNTERED		Total Depth (ft): 12.0	
Boring Location Coordinates:			40° 5' 32.84" N		75° 43' 50.87" W

Sample No.	Sample Depth (ft)		Strata Depth (ft)		Recov. (in)	Strata (USCS)	Description of Materials	6" Increment Blows *			N
	From	To	From	To							
							INITIALLY ENCOUNTERED AUGER REFUSAL AT 2.5', OFFSET 9' WEST AND AUGERED TO 3'.				
1	3.0	3.8	0.0		7	SM/GM	GRAY FINE TO MEDIUM SAND WITH A LITTLE SILT, SOME FINE TO COARSE UNWEATHERED GNEISS GRAVEL.	13	50/3"		>50
							AUGER REFUSAL AT 4.2'. OFF-SET 13' NORTH FROM ORIGINAL LOCATION.				
2	3.0	4.2			10	SM/GM	GRAY AND BROWN FINE TO MEDIUM SAND, SOME SILT, TRACE FINE TO COARSE UNWEATHERED GNEISS GRAVEL. (USCS: SM)	12	13	50/3"	>50
				4.4			AUGER REFUSAL AT 4.4'.				
							<u>ROCK CORING</u>				
RUN 1	4.4	8.0	4.4		34	FRACTURED ROCK	GRAY MODERATELY FRACTURED FELSIC GNEISS	TCR: 79%, SCR: 58%, RQD: 44%			
RUN 2	8.0	12.0			12		GRAY MODERATELY FRACTURED FELSIC GNEISS. NO RECOVERY FROM 10' TO 12' (LIKELY SOIL MATERIAL).	TCR: 25%, SCR: 17%, RQD: 10%			
				12.0							
							<u>CORE TESTING RESULTS (RUN 1, DEPTH 7'-8'):</u>				
							COMPRESSIVE STRENGTH: 7,688 PSI				
							UNIT WEIGHT: 161.1				

Notes/Comments:
Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.
 N: Number of blows to drive spoon from 6" to 18" interval.



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TEST BORING LOG

Project Name: SUNOCO PENNSYLVANIA PIPELINE PROJECT			Project No.: 103IP3406		
Project Location: LITTLE CONESTOGA ROAD, DOWNINGTOWN, PA			Page 1 of 1		
HDD No.: S3-0280		Dates(s) Drilled: 05-20-15		Inspector: E. WATT	
Boring No.: SB-06		Drilling Method: SPT - ASTM D1586		Driller: S. HOFFER	
Drilling Contractor: HAD DRILLING		Groundwater Depth (ft): 23.0		Total Depth (ft): 30.0	
Boring Location Coordinates:			40° 5' 1.030" N		75° 43' 7.844" W

Sample No.	Sample Depth (ft)		Strata Depth (ft)		Recov. (ft)	Strata (USCS)	Description of Materials	6" Increment Blows *				N	
	From	To	From	To									
			0.0	0.3			TOPSOIL (4")						
1	3.0	5.0	0.3		19	SM	LIGHT BROWN AND BROWN FINE TO MEDIUM SAND WITH SOME SILT, A LITTLE FINE TO COARSE GRAVEL.	1	5	12	17	17	
2	8.0	10.0			22		LIGHT GRAY MICACEOUS FINE TO MEDIUM SAND AND SILT	2	5	5	8	10	
3	13.0	15.0			24		LIGHT GRAY MICACEOUS FINE TO MEDIUM SAND AND SILT	1	2	4	6	6	
							(USCS: SM)						
4	18.0	20.0			24		ALTERNATING BANDS OF BROWN, LIGHT GRAY, AND WHITE FINE SAND AND SILT, TRACE FINE GRAVEL.	WH	WH	3	13	3	
5	23.0	25.0			24		ALTERNATING BANDS OF BROWN, LIGHT GRAY, AND WHITE FINE SAND AND SILT, TRACE FINE GRAVEL. (USCS: SM).	1	2	7	11	9	
6	28.0	30.0			24		BROWN AND WHITE MICACEOUS FINE SAND AND SILT, WITH A LITTLE FINE TO COARSE GRAVEL.	2	8	14	18	22	
				30.0									
								WATER LEVEL THROUGH AUGERS AT 23'.					
							CAVED AT 27', WATER LEVEL ON CAVE AT 23'.						
							SAMPLES 2 THRU 6 ARE HIGHLY DECOMPOSED ROCK SOILS. (SOILS THAT HAVE BEEN WEATHERED IN-PLACE FROM ROCK)						

Notes/Comments:
Pocket Pentrometer Testing

Strata (USCS) Designations are approximated based on visual review, except where indicated in Description of Materials.

* Number of blows of 140 lb. Hammer dropped 30 in. required to drive 2 in. split-spoon sampler in 6 in. increments.
 N: Number of blows to drive spoon from 6" to 18" interval.

**ROCK CORE DESCRIPTION SUMMARY
SUNOCO PENNSYLVANIA PIPELINE PROJECT
HDD S3-0280**

Location	Boring No.	Core Run	Core Depth (ft)		TCR (%)	SCR (%)	RQD (%)	Depth (ft)		Weathering	Classification	Bedding Thickness (ft)	Color	Discontinuity Data	
			From	To				From	To						
S3-0280	SB-4	1	32	32.7	100	94	94	32	34.5	Slight	Graphitic Gneiss	2.5	Light Gray	Fractures ranging from 10° to 70°, Avg. 47.5°	
		2	32.7	37	100	19	13								
	SB-5	1	1	4.4	8	79	58	44	4.4	12	Slight	Gneiss	Massive	Light gray/gray	Fractures ranging from 2° to 46°, Avg. 26°
			2	8	12	25	17	10							
		2	1	4.4	8	79	58	44	4.4	12	Slight	Gneiss	Massive	Light gray/gray	Fractures ranging from 2° to 46°, Avg. 26°
			2	8	12	25	17	10							

GEOTECHNICAL LABORATORY TESTING SUMMARY
SUNOCO PENNSYLVANIA PIPELINE PROJECT
HDD S3-0280

HDD No.	Test Boring No.	Sample No.	Depth of Sample (ft.)		Water Content, % (ASTM D2216)	Percent Silts/Clays, % (ASTM D1140)	Atterburg Limits (ASTM D4318)			USCS Classif. (ASTM D2487)	
			From	To			Liquid Limit, %	Plastic Limit, %	Plasticity Index, %		
S3-0280	SB-01	1	3.0	4.6	8.2	23.6	NV	NP	NP	SM	
		2	8.0	8.6	5.7	18.2	-	-	-	-	
	SB-02	2	8.0	10.0	21.9	71.5	-	-	-	-	
		3	13.0	14.8	14.0	84.7	36	27	9	ML	
		4	18.0	20.0	19.2	73.4	-	-	-	-	
	SB-03	5	23.0	23.8	7.0	43.0	32	25	7	SM	
		1	3.0	5.0	19.9	57.4	-	-	-	-	
		2	8.0	10.0	11.8	52.3	-	-	-	-	
		3	13.0	14.8	13.1	59.4	34	26	8	ML	
	SB-04	4	18.0	19.2	10.2	42.0	30	23	7	SM	
		2	8.0	10.0	11.6	34.1	-	-	-	-	
		3	13.0	14.9	12.4	34.0	31	24	7	SM	
		4	18.0	18.9	8.3	19.6	-	-	-	-	
		5	23.0	23.6	3.7	16.5	-	-	-	-	
	SB-05	6	28.0	28.6	5.1	24.6	-	-	-	-	
		1	3.0	3.8	5.4	16.9	-	-	-	-	
	SB-06	2	3.0	4.2	15.7	36.1	30	23	7	SM	
		2	8.0	10.0	25.2	44.9	-	-	-	-	
		3	13.0	15.0	31.8	46.2	36	25	11	SM	
		4	18.0	20.0	21.7	39.9	-	-	-	-	
		5	23.0	25.0	20.9	44.9	37	26	11	SM	
			6	28.0	30.0	18.3	44.3	-	-	-	-

Rock Core Testing Results				
Boring No.	Core Run	Approximate Depth (ft)	Compressive Strength (psi)	Unit Weight (pcf)
SB-04	2	35.0-35.7	1,520	173.8
SB-05	1	7.0 to 8.0	7,688	161.1

Notes:

- 1) Sample depths based on feet below grade at time of exploration.

**REGIONAL GEOLOGY SUMMARY
SUNOCO PENNSYLVANIA PIPELINE PROJECT
HDD S3-0280**

HDD No.	NAME	BORING NO.	REGIONAL GEOLOGY DESCRIPTION	BEDROCK FORMATION	GENERAL ROCK TYPE	APPROX MAX FM THICKNESS (FT)	DEPTH TO ROCK (Ft bgs) based on nearby well drilling logs	NOTES / COMMENTS
S3-0280	Marsh Creek State Park and Park Road	SB-01	Graphitic felsic gneiss - Medium grained, medium to dark gray; locally gneissic, predominantly feldspar and quartz, plus dark accessories and various alteration minerals.	Graphitic felsic gneiss (PreCambrian)	Graphitic felsic gneiss	No information found during literature review	Ranges from 4 to 46 ft bgs, avg 26 ft bgs (.25 mile radius)	southwest to northeast oriented fault south of the SB-01 and SB-05
		SB-02					Widely ranges from 6 to 102 ft bgs (.25 mile radius)	
		SB-05					Ranges from 4 to 46 ft bgs, avg 26 ft bgs (.25 mile radius)	southwest to northeast oriented fault south of the SB-01 and SB-05
		SB-03	Banded mafic gneiss - Dark, fine to medium grained; includes rocks of probable sedimentary origin; may be equivalent to "PZmgh."	Banded mafic gneiss (PreCambrian)	Mafic gneiss	No information found during literature review	Ranges from 6 to 75 ft bgs, avg 32 ft bgs (.25 mile radius)	
		SB-04						Mafic gneiss
		SB-06	Metadiabase - Dark-gray, fine-grained intrusives; locally, mineralogy is altered and unit has greenish color.	Metadiabase (PreCambrian)	Mafic metavolcanic rock	Mostly in thin dikes, but a few over 100 ft	Ranges from 4 to 50 ft bgs, avg 23ft bgs (.25 mile radius)	Mapped as a mafic dike, may not be encountered. Difficult to excavate except where fractured. Occurs as a limited isolated intrusion; not a formation per say.

Note : Source of well log data - <http://www.dcnr.state.pa.us/topogeo/groundwater/pagwis/records/index.htm>. All other sources as referenced in comments section.

FIELD DESCRIPTION AND LOGGING SYSTEM FOR SOIL EXPLORATION

GRANULAR SOILS

(Sand, Gravel & Combinations)

<u>Density</u>	<u>N (blows)*</u>
Very Loose	5 or less
Loose	6 to 10
Medium Dense	11 to 30
Dense	31 to 50
Very Dense	51 or more

Particle Size Identification

Boulders	8 in. diameter or more
Cobbles	3 to 8 in. diameter
Gravel	Coarse (C) 3 in. to ¾ in. sieve Fine (F) ¾ in. to No. 4 sieve
Sand	Coarse (C) No. 4 to No. 10 sieve (4.75mm-2.00mm) Medium (M) No. 10 to No. 40 sieve (2.00mm – 0.425mm) Fine (F) No. 40 to No. 200 sieve (0.425 – 0.074mm)
Silt/Clay	Less Than a No. 200 sieve (<0.074mm)

Relative Proportions

<u>Description Term</u>	<u>Percent</u>
Trace	1 - 10
Little	11 - 20
Some	21 - 35
And	36 - 50

COHESIVE SOILS

(Silt, Clay & Combinations)

<u>Consistency</u>	<u>N (blows)*</u>
Very Soft	3 or less
Soft	4 to 5
Medium Stiff	6 to 10
Stiff	11 to 15
Very Stiff	16 to 30
Hard	31 or more

Plasticity

<u>Degree of Plasticity</u>	<u>Plasticity Index</u>
None to Slight	0 - 4
Slight	5 - 7
Medium	8- 22
High to Very High	> 22

ROCK

(Rock Cores)

<u>Rock Quality Designation (RQD), %</u>	<u>Rock Quality Description</u>
0-25	Very Poor
25-50	Poor
50-75	Fair
75-90	Good
90-100	Excellent

***N - Standard Penetration Resistance.** Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 18 inches into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. The number of hammer blows to drive the sampler through each 6 inch interval is recorded; the number of blows required to drive the sampler through the final 12 inch interval is termed the Standard Penetration Resistance (SPR) N-value. For example, blow counts of 6/8/9 (through three 6-inch intervals) results in an SPR N-value of 17 (8+9).

Groundwater observations were made at the times indicated. Groundwater elevations fluctuate throughout a given year, depending on actual field porosity and variations in seasonal and annual precipitation.

UNIFIED SOIL CLASSIFICATION SYSTEM [Casagrande (1948)]

Major Divisions		Group Symbols	Typical Descriptions	Laboratory Classifications				
Coarse Grained Soils (More than half of material is larger than No. 200 sieve)	Gravels (More than half of coarse fraction is larger than No. 4 sieve size)	Clean gravel (Little or no fines)	GW Well-graded gravels, gravel-sand mixtures, little or no fines	Determine Percentage of sand and gravel from grain size curve. Depending on Percentage of fines (fraction smaller than No. 200 sieve), coarse-grained soils are classified as follows: Less than 5 percent GW, GP, SW, SP More than 12 percent GM, GC, SM, SC 5 to 12 percent Borderline cases requiring dual symbols ⁽¹⁾	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3			
		GP Poorly graded gravels, gravel-sand mixtures, little or no fines	Not meeting C_u or C_c requirements for GW					
		Gravel with fines (Appreciable amount of fines)	GM Silty gravels, gravel-sand-silt mixtures		Atterberg limits below A Line or I_p less than 4	Limits plotting in hatched zone with I_p between 4 and 7 are borderline cases requiring use of dual symbols		
			GC Clayey gravels, gravel-sand-clay mixtures		Atterberg limits above A line with I_p greater than 7			
	Sands (More than half of coarse fraction is smaller than No. 4 Sieve)	Clean sands (Little or no fines)	SW Well graded sands, gravelly sands, little or no fines		$C_u = \frac{D_{60}}{D_{10}}$ greater than 6: $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3			
			SP Poorly graded sands, gravelly sands, little or no fines		Not meeting C_u or C_c requirements for SW			
		Sands with fines (Appreciable amount of fines)	SM Silty sands, sand-silt mixtures		Atterberg limits below A Line or I_p less than 4	Limits Plotting in hatched zone with I_p between 4 and 7 are borderline cases requiring use of dual symbols		
			SC Clayey sands, sand-clay mixtures		Atterberg limits above A line with I_p greater than 7			
						For soils plotting nearly on A line use dual symbols i.e., $I_p = 29.5$, $w_L = 60$ gives CH-MH. When w_L is near 50 use CL-CH or ML-MH. Take near as ± 2 percent.		
		Fine-grained soils (More than half of material is smaller than No. 200 sieve)	Silt and clays (Liquid limit less than 50)		ML Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity			
CL Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays								
OL Organic silts and organic silty clays of low plasticity								
Silt and Clays (Liquid limit greater than 50)	MH Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts							
	CH Inorganic clays of high plasticity, fat clays							
	OH Organic clays of medium to high plasticity, organic silts							
Highly organic soils	Pt Peat and other highly organic soils							

(1) Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC. well-graded gravel-sand mixture with clay binder.