

TETRA TECH, INC.

By: RH Date: 1/30/2017 Subject: Gates Road

Checked By: JB Date: 2/1/2017 PCSM Design and Evaluation

PURPOSE:

The purpose of these calculations is to design a Post-Construction Stormwater Management (PCSM) Plan for the Gates Road block valve as part of the Sunoco Pipeline L.P. Pennsylvania Pipeline Project. The site is located within Conewago Township, Dauphin County, Pennsylvania. Permanent stormwater controls will be developed to satisfy PADEP and Dauphin County's approved Act 167 Plan.

PCSM DESIGN REQUIREMENTS:

The PCSM design for this project follows the PA Department of Environmental Protection's (PADEP) Pennsylvania Stormwater Best Management Practices Manual (BMP Manual), December 2006; and the standard design criteria from PA Title 25, Chapter 102.8.(g)(2) and (3). The design criteria evaluated for the site are summarized below.

Act 167 Consistency

Dauphin County has a County-wide Act 167 plan with specific requirements for Spring Creek (East), where the block valve is located. The PCSM design at the Gates Road block valve site has been designed for consistency with Dauphin County's approved Act 167 Plan.

Recommended Volume Control Guideline

Use of Control Guideline 1 is recommended where site conditions offer the opportunity to reduce the increase in runoff volume as follows:

- Do not increase the post-development total runoff volume for all storms equal to or less than the two-year/24-hour event;
- Existing (pre-development) non-forested pervious areas must be considered meadow (good condition) or its equivalent; and
- 20 percent of existing impervious area, when present, shall be considered meadow (good condition) or its equivalent.

This site will utilize an infiltration berm to manage the two-year/24-hour volume increase.

Recommended Peak Rate Control Guideline

The recommended control guideline for peak rate control is:

- Do not increase the peak rate of discharge for the 2-year through 100-year events (at minimum); as necessary, provide additional peak rate control as required by applicable and approved Act 167 plan.
- The Gates Road block valve site is located in Spring Creek (East) watershed, subwatershed
 SP5. This particular subwatershed has the release rate requirements listed below.

Subwatershed	1-Year	2-Year	10-Year	25-Year	50-Year	100-Year
SP5	100%	90%	90%	90%	100%	100%

This site will utilize an infiltration berm to manage the two-year through 100-year peak rate increases. The requirements from both PADEP and Dauphin County's Act 167 Plan are met with this proposed design. This BMP will also help to increase the time of concentration for the drainage area encompassing the block valve.

Recommended Water Quality Control Guideline

Control Guideline 1 will provide water quality control and stream channel protection as well as flood control protection.

Infiltration

Infiltration rates for the PCSM BMPs have been determined from site infiltration testing conducted in accordance of the PA BMP Manual. Documentation for infiltration testing and design infiltration rates can be found in Attachment 5 of the Site Restoration/Post Construction Stormwater Management Plan. Infiltration test locations and recommended design rates are also labeled on the PCSM Plan Drawings in Attachment 6.

During the onsite infiltration tests, the depth to seasonal high groundwater and shallow bedrock or another confining layer were evaluated. The post-construction stormwater management facility for the site has been designed to maintain 2 feet of separation between the ponding elevation of the facility and the seasonal high water table and bedrock.

The post-construction stormwater management design will utilize onsite infiltration to meet Volume Control Guideline 1.

Loading Ratio

Loading ratios have been considered for the design of infiltration BMPs. In general, the following Loading Ratio guidelines are recommended:

- Maximum Impervious Loading Ratio of 5:1 relating impervious drainage area to infiltration area.
- Maximum Drainage Area Loading Ratio of 8:1 relating total drainage area to infiltration area.

The maximum impervious loading ratio of 5:1 has been met. The impervious loading ratio for the site is 0.7:1.

The maximum drainage area loading ratio of 8:1 has been met. The drainage area loading ratio for the site is 3.2:1.

Disturbed Area

To meet Standard Worksheet 10 guidelines, 90% of the disturbed area is contained by the proposed PCSM BMPs.

Karst Topography

The Gates Road block valve is not located in an area of karst terrain.

Special Protection Watershed

Gates Road block valve is located within a siltation impaired watershed. The project site was designed to minimize the total amount of impervious area. The impervious area for the Gates Road valve was limited to the amount that is required to safely construct and operate the block valve. In addition, the previously proposed gravel turn-around was eliminated, and replaced with a grass area. Finally, an existing access road was utilized for the site, thus eliminating the need to create a larger impervious area.

Non-discharge alternatives were analyzed for this block valve site. The location of the Gates Road block valve site was evaluated by ASME B31.4 Valve Spacing 434.15.2(e) which states that mainline valves should not be more than 7.5 miles apart. The valve sites were located in such a way that they avoided environmentally sensitive areas (such as wetlands and floodplains), were close to an existing road, and close to power. Land owner preference was also accounted for while locating the block valve sites. Once all of these factors were taken into account, several block valve sites, including Gates Road, were located in special protection or siltation impaired watersheds.

Non-discharge alternatives were also considered when determining the type of BMP proposed. Gates Road block valve site utilizes an infiltration berm to manage stormwater. Stormwater runoff is infiltrated to the maximum extent possible. Stormwater runoff is spread out to flow through areas that have been restored to meadow conditions, to an infiltration berm, or to undisturbed area. There will not be an increase in stormwater runoff rate or volume to prevent the physical degradation of the receiving water, such as scour, and stream bank destabilization. Stormwater runoff volume is not increasing throughout post-construction, and any post-construction stormwater discharge is managed so that it will not degrade the physical, chemical or biological characteristics of the receiving stream.

Runoff from the site will be managed by a downslope infiltration berm. Ponded runoff will be temporarily stored upslope of the berm until it infiltrates and filters through the soil media. Due to the design of the berm, which maintains a constant elevation through the entire berm length, the stormwater runoff will be released in sheet flow down a stabilized slope, without causing erosion, rather than concentrating the flow. Filtration through the existing vegetation and soil is an efficient way to remove suspended stormwater pollutants such as sediment, as the suspended particles are physically filtered from the stormwater as it flows through the vegetation and percolates into the soil.

The extent of the disturbed area will be minimized, and the duration of disturbance will be minimized by stabilizing disturbed areas as soon as practicable. Cut and fill for the project site has been minimized. Where possible based on the criteria listed above, sites were located in areas with shallow slopes to minimize the amount of cut and fill required. There is minimal cut and fill required at the Gates Road block valve site. All of the block valve sites were graded towards the natural slope. No direct discharge to surface water occurs at the site. The site will be restored promptly with proper vegetative cover techniques.

Antidegradation requirements for the special protection watershed are met because the post-construction stormwater infiltration volume equals or exceeds the pre-construction stormwater infiltration volume, and post-construction stormwater discharge is pretreated via infiltration berms. The runoff is managed so that it will not degrade the physical, chemical, or biological characteristics of the receiving stream.



NOAA Atlas 14, Volume 2, Version 3 Location name: Conewago Twp, Pennsylvania, USA*

Latitude: 40.247°, Longitude: -76.6182°
Elevation: 474.34 ft**

* source: ESRI Maps

** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

Durotion		Average recurrence interval (years)								
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.325 (0.293-0.359)	0.386 (0.349-0.428)	0.454 (0.409-0.503)	0.504 (0.453-0.558)	0.563 (0.504-0.623)	0.605 (0.541-0.669)	0.647 (0.576-0.714)	0.685 (0.606-0.757)	0.732 (0.644-0.809)	0.768 (0.671-0.850
10-min	0.518 (0.468-0.574)	0.617 (0.557-0.685)	0.726 (0.655-0.806)	0.803 (0.722-0.889)	0.893 (0.800-0.988)	0.960 (0.857-1.06)	1.02 (0.911-1.13)	1.08 (0.956-1.19)	1.15 (1.01-1.27)	1.20 (1.05-1.33)
15-min	0.646 (0.583-0.715)	0.773 (0.698-0.858)	0.916 (0.825-1.02)	1.01 (0.912-1.12)	1.13 (1.01-1.25)	1.21 (1.08-1.34)	1.29 (1.15-1.43)	1.36 (1.21-1.50)	1.45 (1.28-1.60)	1.51 (1.32-1.67)
30-min	0.883 (0.797-0.978)	1.06 (0.962-1.18)	1.30 (1.17-1.44)	1.46 (1.32-1.62)	1.67 (1.49-1.84)	1.82 (1.63-2.01)	1.97 (1.75-2.18)	2.11 (1.87-2.33)	2.29 (2.02-2.54)	2.43 (2.12-2.69)
60-min	1.10 (0.993-1.22)	1.33 (1.21-1.48)	1.66 (1.50-1.84)	1.90 (1.71-2.11)	2.22 (1.99-2.45)	2.46 (2.20-2.72)	2.71 (2.41-2.99)	2.95 (2.61-3.26)	3.28 (2.89-3.63)	3.54 (3.09-3.92)
2-hr	1.29 (1.17-1.44)	1.57 (1.42-1.74)	1.98 (1.78-2.19)	2.30 (2.07-2.54)	2.75 (2.47-3.04)	3.13 (2.79-3.45)	3.53 (3.12-3.89)	3.95 (3.47-4.36)	4.56 (3.96-5.04)	5.06 (4.36-5.60)
3-hr	1.41 (1.27-1.57)	1.71 (1.55-1.91)	2.16 (1.95-2.40)	2.51 (2.26-2.80)	3.02 (2.70-3.34)	3.43 (3.05-3.80)	3.87 (3.42-4.29)	4.34 (3.81-4.81)	5.02 (4.35-5.57)	5.58 (4.79-6.20)
6-hr	1.74 (1.57-1.95)	2.11 (1.90-2.37)	2.65 (2.38-2.97)	3.10 (2.77-3.46)	3.76 (3.34-4.18)	4.32 (3.81-4.79)	4.92 (4.31-5.46)	5.59 (4.85-6.19)	6.57 (5.62-7.27)	7.39 (6.25-8.20)
12-hr	2.13 (1.91-2.41)	2.57 (2.30-2.91)	3.25 (2.90-3.67)	3.83 (3.40-4.32)	4.70 (4.14-5.28)	5.45 (4.77-6.10)	6.30 (5.45-7.04)	7.24 (6.20-8.07)	8.66 (7.29-9.65)	9.90 (8.21-11.0)
24-hr	2.46 (2.25-2.72)	2.97 (2.72-3.28)	3.77 (3.45-4.17)	4.47 (4.08-4.93)	5.55 (5.01-6.09)	6.50 (5.83-7.11)	7.58 (6.72-8.25)	8.79 (7.71-9.56)	10.7 (9.21-11.6)	12.3 (10.5-13.3)
2-day	2.85 (2.61-3.16)	3.44 (3.15-3.82)	4.38 (4.00-4.85)	5.17 (4.71-5.72)	6.37 (5.75-7.02)	7.40 (6.64-8.15)	8.56 (7.62-9.41)	9.86 (8.68-10.8)	11.8 (10.2-13.0)	13.5 (11.6-14.8)
3-day	3.02 (2.78-3.33)	3.65 (3.35-4.02)	4.63 (4.24-5.10)	5.46 (4.99-6.01)	6.71 (6.09-7.37)	7.81 (7.03-8.55)	9.02 (8.06-9.86)	10.4 (9.19-11.3)	12.5 (10.8-13.6)	14.2 (12.2-15.6)
4-day	3.19 (2.94-3.51)	3.85 (3.55-4.23)	4.88 (4.48-5.35)	5.75 (5.27-6.30)	7.07 (6.43-7.71)	8.21 (7.42-8.95)	9.49 (8.51-10.3)	10.9 (9.70-11.9)	13.1 (11.5-14.2)	15.0 (12.9-16.3)
7-day	3.76 (3.48-4.10)	4.51 (4.18-4.93)	5.65 (5.22-6.17)	6.63 (6.11-7.22)	8.09 (7.40-8.79)	9.35 (8.51-10.2)	10.7 (9.71-11.7)	12.3 (11.0-13.3)	14.6 (12.9-15.9)	16.6 (14.5-18.1)
10-day	4.31 (4.02-4.66)	5.17 (4.82-5.59)	6.40 (5.95-6.91)	7.43 (6.89-8.01)	8.92 (8.23-9.61)	10.2 (9.35-11.0)	11.6 (10.5-12.4)	13.0 (11.8-14.0)	15.2 (13.6-16.4)	17.1 (15.1-18.4)
20-day	5.88 (5.53-6.27)	6.98 (6.56-7.44)	8.38 (7.87-8.94)	9.51 (8.91-10.1)	11.1 (10.4-11.8)	12.4 (11.6-13.2)	13.8 (12.8-14.7)	15.2 (14.0-16.2)	17.2 (15.7-18.3)	18.8 (17.1-20.1)
30-day	7.28 (6.87-7.73)	8.60 (8.12-9.13)	10.1 (9.57-10.8)	11.4 (10.7-12.1)	13.1 (12.3-13.9)	14.5 (13.6-15.4)	15.9 (14.8-16.9)	17.4 (16.1-18.5)	19.4 (17.9-20.7)	21.0 (19.3-22.4)
45-day	9.16 (8.70-9.65)	10.8 (10.2-11.4)	12.5 (11.9-13.2)	13.8 (13.1-14.6)	15.6 (14.8-16.4)	17.0 (16.1-17.9)	18.3 (17.3-19.3)	19.7 (18.5-20.7)	21.5 (20.1-22.7)	22.9 (21.3-24.1)
60-dav	11.0	12.8	14.8	16.2	18.1	19.6	21.0	22.4	24.1	25.5

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

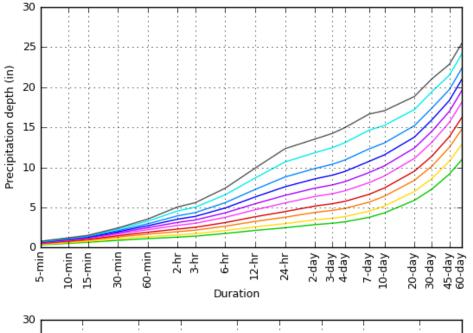
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

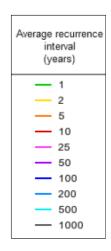
Please refer to NOAA Atlas 14 document for more information.

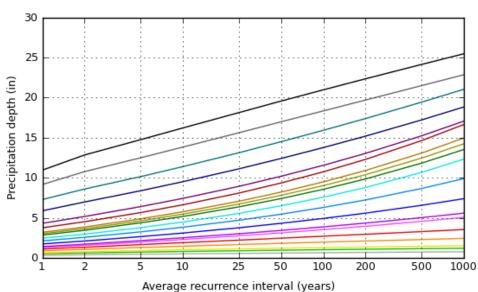
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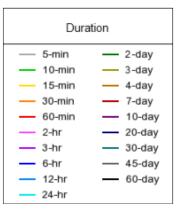
PF graphical

PDS-based depth-duration-frequency (DDF) curves Latitude: 40.2470°, Longitude: -76.6182°







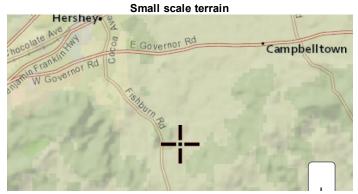


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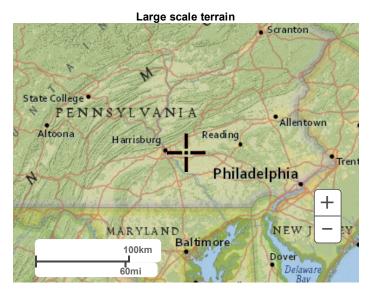
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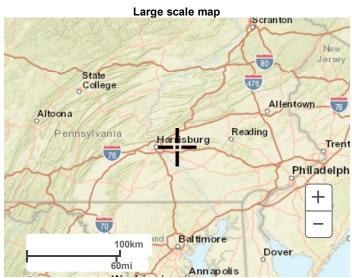
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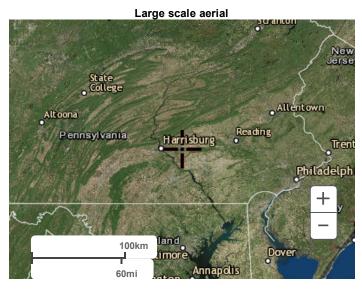
Maps & aerials











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WORKSHEET 1. GENERAL SITE INFORMATION

November 11, 2016				
Gates Road				
Conewago				
Dauphin				
1.81				
Susquehanna River				
Little Swatara and Quittapahilla Creeks				
Spring Creek (East)				
ter to Receive Runoff: Trib 09509 To Spring Creek				
ated Water Use: WWF				
mpaired according to Chapter 303(d) list? List Causes of Impairment: Source Unknown - Pathogens, Agriculture - Organic Enrichment/Low D.O.; Agriculture - Siltation				
Project Subject to, or Part of:				
eparate Storm Sewer System (MS4) Requirements	YES			
Existing or Planned drinking water supply? Yes				
If yes, distance from proposed discharge (miles):				
ct 167 Plan?	YES X			
er Conservation Plan?	NO YES X NO			
	Conewago Dauphin 1.81 Susquehanna River Little Swatara and Quittapahilla Creeks Spring Creek (East) ter to Receive Runoff: Trib 09509 To Spring Creek ated Water Use: WWF to Chapter 303(d) list? of Impairment: n - Pathogens, Agriculture - Organic Enrichment/Low D.O.; Agriculture o, or Part of: eparate Storm Sewer System (MS4) Requirements Planned drinking water supply? ce from proposed discharge (miles):			

Worksheet 2. Sensitive Natural Resources

INSTRUCTIONS

1. Provide Sensitive Resources Map according to non-structural BMP 5.4.1 in Chapter 5. This map should identify wetlands, woodlands, natural drainage ways, steep slopes, and other sensitive natural areas.

See pre-development drainage area map

2. Summarize the existing extent of each sensitive resource in the Existing Sensitive Resources Table (below, using Acres). If none present, insert 0.

0.00 acres

3. Summarize Total Protected Area as defined under BMPs in Chapter 5.

0.00 acres

4. Do not count any area twice. For example, an area that is both a floodplain and a wetland may only be considered once.

EXISTING NATURAL SENSITIVE RESOURCE	MAPPED? Yes/no/n/a	TOTAL AREA (Ac.)	PROTECTED AREA (Ac.)
Waterbodies	N/A		
Floodplains	N/A		
Riparian Areas	N/A		
Wetlands	N/A		
Woodlands	N/A		
Natural Drainage Ways	N/A		
Steep Slopes, 15% - 25%	N/A		
Steep Slopes, over 25%	N/A		
Other:			
Other:			
TOTAL EXISTING:		0.00	0.00

Wor	ksheet 3. Nonst	ructural BMP Cr	edits			
PROTECTED AREA						
1.1 Area of Protected Sensit	1.1 Area of Protected Sensitive/Special Value Features (see WS 2) <u>0.00</u> Ac.					
1.2 Area of Riparian Forest B	Buffer Protection			<u>0.00</u> Ac.		
3.1 Area of Minimum Disturb	oance/Reduced Grad	ling		<u>0.00</u> Ac		
			T	OTAL <u>0.00</u> Ac		
Site Area 0.89	Minus Protected Area - 0 This is the area that re stormwater manager	= 0.89	vater Manag	ement Area		
VOLUME CREDITS						
3.1 Minimum Soil Compaction	on (See Chapter 8, page	e 22 – SW BMP Manua	ıl)			
Lawn	ft²	x 1/4" x 1/12	=	ft³		
Meadow	ft²	x 1/3" x 1/12	=	ft³		
3.3 Protect Existing Trees (S	See Chapter 8, page 23	- SW BMP Manual)				
For Trees within 100 feet of	of impervious area:					
Tree Canopy	ft²	x 1/2" x 1/12	=	ft³		
5.1 Disconnect Roof Leaders For runoff directed to area	•		25 – SW BMF	P Manual)		
Roof Area	s <i>protected under 5.</i> 6 ft²	x 1/3" x 1/12	_	ft ³		
For all other disconnected		X 1/3 X 1/12	_			
Roof Area	ft ²	x 1/4" x 1/12	=	ft ³		
5.2 Disconnect Non-Roof im			er 8. page 26 ·			
For Runoff directed to area	•		-,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	,		
Impervious Area	, ft ²	x 1/3" x 1/12	=	ft ³		
For all other disconnected	roof areas					
Impervious Area	ft ²	x 1/4" x 1/12	=	ft ³		
	TOTAL NON-STR	JCTURAL VOLUME	CREDIT*	ft³		
	*For use on Worksheet	5	L			

WORKSHEET 4. CHANGE IN RUNOFF VOLUME FOR 2-YR STORM EVENT

PROJECT: Gates Road

Drainage Area: 1.81 acres

2-Year Rainfall: 2.97 in

Total Site Area:0.89 acresProtected Site Area:N/A acresManaged Site Area:0.89 acres

Existing Conditions

Cover Type/Condition	Soil Type	Area (sf)	Area (ac)	CN	S	la (0.2*S)	Q Runoff ¹ (in)	Runoff Volume ³ (ft ³)
Woods	В	0.00	0.00	55	8.18	1.64	0.19	0
Meadow	В	38768	0.89	58	7.24	1.45	0.26	854
TOTAL:		38768	0.89					854

Developed Conditions

Cover Type/Condition	Soil Type	Area (sf)	Area (ac)	CN	S	la (0.2*S)	Q Runoff ¹ (in)	Runoff Volume ³ (ft ³)
Impervious - Gravel	В	5663	0.13	85	1.76	0.35	1.56	738
Meadow	В	33106	0.76	58	7.24	1.45	0.26	729
TOTAL:		38768	0.89					1,467

2-Year Volume Increase (ft3):	613

2-Year Volume Increase = Developed Conditions Runoff Volume - Existing Conditions Runoff Volume

1. Runoff (in) = Q = (P - 0.2S)2 / (P + 0.8S) where

P = 2-Year Rainfall (in)

S = (1000/CN)-10

2. Runoff Volume (CF) = Q x Area x 1/12

Q = Runoff (in)

Area = Land use area (sq. ft.)

Note: Runoff Volume must be calculated for EACH land use type/condition and HSGI. The use of a weighted CN value for volume calculations is not acceptable.

PROJECT SUB-BAS							
	Required Control Volume (ft ³) - from Worksheet 4:						
	Non-structural Volume Credit (ft ³) - from Worksheet 3:						
	(maximum is 25% of require	N/A					
		·					
	Structural Volume Reqmt		613				
	(Required Control Volume minus Non-structural C	Credit)					
Propos	ed BMPs from PA Stormwater Best Management Practices Manual Chapter 6	Area (ft²)	Volume Reduction Permanently Removed (ft ³)				
6.4.1	Porous Pavement						
6.4.2	Infiltration Basin						
6.4.3	Infiltration Bed						
6.4.4	Infiltration Trench						
6.4.5	Rain Garden/Bioretention						
6.4.6	Dry Well/Seepage Pit						
6.4.7	Constructed Filter						
6.4.8	Vegetated Swale						
6.4.9	Vegetated Filter Strip	1 607	011				
6.4.10 6.5.1	Berm Vegetated Roof	1,607	811				
6.5.2	Capture and Re-Use						
6.6.1	Constructed Wetlands						
6.6.2	Wet Pond/Retention Basin						
6.7.1	Riparian Buffer/Riparian Forest Buffer Restoration						
6.7.2	Landscape Restoration/Reforestation						
6.7.3	Soil Amendment						
6.8.1	Level Spreader						
6.8.2	Special Storage Areas						
Other:	-						
	Total Str	uctural Volume (ft³):	811				
	Structural Volun	ne Requirement (ft ³):	613				
		DIFFERENCE:	-198				
VOLUME	CREDIT DETERMINATION	JII I EILEITOL.	100				
1	Detained area runoff volume from Hydraflow	= 811	cf				
2a	Storage volume of the BMPs	= 2,343	cf				
2b	Infiltrated volume within 2 hours after the 2-yr/24-hr e (Infiltration Rate/12) x Infiltration Area x 2 hrs	vent = 3,362	cf				
2 (total)	2a + 2b	= 5,705	cf				

WORKSHEET 10. WATER QUALITY COMPLIANCE FOR	RNITRATE
Does the site design incorporate the following BMPs to address nitrate pollution? A rating is achieved if at least 2 Primary BMPs for nitrate are provided across the site o BMPs for nitrate are provided across the site (or the	
PRIMARY BMPs FOR NITRATE:	YES NO
NS BMP 5.4.2 - Protect / Conserve / Enhance Riparian Buffers	
NS BMP 5.5.4 - Cluster Uses at Each Site	
NS BMP 5.6.1 - Minimize Total Disturbed Area	\boxtimes
NS BMP 5.6.3 - Re-Vegetate / Re-Forest Disturbed Areas (Native Species)	\triangleright
NS BMP 5.9.1 - Street Sweeping / Vacuuming	
Structural BMP 6.7.1 - Riparian Buffer Restoration	
Structural BMP 6.7.2 - Landscape Restoration	
SECONDARY BMPs FOR NITRATE:	
NS BMP 5.4.1 - Protect Sensitive / Special Value Features	
NS BMP 5.4.3 - Protect / Utilize Natural Drainage Features	
NS BMP 5.6.2 - Minimize Soil Compaction	\boxtimes
Structural BMP 6.4.5 - Rain Garden / Bioretention	
Structural BMP 6.4.8 - Vegetated Swale	
Structural BMP 6.4.9 - Vegetated Filter Strip	
Structural BMP 6.6.1 - Constructed Wetland	
Structural BMP 6.7.1 - Riparian Buffer Restoration	
Structural BMP 6.7.2 - Landscape Restoration	
Structural BMP 6.7.3 - Soils Amendment/Restoration	\square

TIME OF CONCENTRATION ADJUSTMENT

POST CONSTRUCTION TC TO BMP (DETAINED TC) BEFORE ADJUSTMENT

7.0 MIN

STRUCTURAL VOLUME PROVIDED BY BMP

811 CF FOR 2-YR/24-HR STORM EVENT 2,219 CF FOR 10-YR/24-HR STORM EVENT 2,343 CF FOR ALL REMANING STORM EVENTS

RATES OF RUNOFF TO THE BMP (FROM HYDRAFLOW REPORT)

Storm Event	Q (CFS)
2 YR/24 HR	0.285
10 YR/24 HR	0.943
50 YR/24 HR	2.041
100 YR/24 HR	2.678

ADDITIONAL RESIDENCE TIME (MIN) = (STRUCTURAL VOLUME PROVIDED BY BMP / RATE OF RUNOFF TO BMP) / 60

Storm Event	Q (CFS)	Additional Residence Time (min.)
2 YR/24 HR	0.285	47.427
10 YR/24 HR	0.943	39.219
50 YR/24 HR	2.041	19.133
100 YR/24 HR	2.678	14.582

ADJUSTED TC = POST CONSTRUCTION TC TO BMP BEFORE ADJUSTMENT + ADDITIONAL RESIDENCE TIME

		Additional	Adjusted Time of	
Storm Event	Q (CFS)	Residence Time	Concentration	
		(min.)	(min.)	
2 YR/24 HR	0.285	47.43	54.43	
10 YR/24 HR	0.943	39.22	46.22	
50 YR/24 HR	2.041	19.13	26.13	
100 YR/24 HR	2.678	14.58	21.58	

INFILTRATION BERM DEWATERING CALCULATION SITE NAME: GATES ROAD

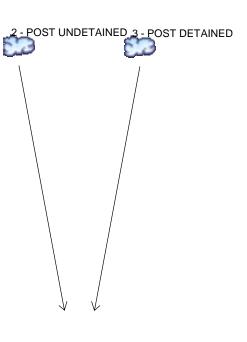
STORAGE VOLUME 811 CF AFTER 2-YEAR/24-HOUR STORM DESIGN INFILTRATION RATE 0.10 IN/HR IT-A RECOMMENDED RATE

INFILTRATION AREA 2,343 SF

DEWATERING TIME = STORAGE VOLUME / ((DESIGN INFILTRATION RATE /12) * INFILTRATION AREA)

DEWATERING TIME = 41.5 HOURS







Legend

Hyd.OriginDescription1SCS RunoffPRE2SCS RunoffPOST UNDETAINED3SCS RunoffPOST DETAINED4CombinePOST AT POI

Project: Gates Road.gpw

Monday, 01 / 23 / 2017

Hydrograph Return Period Recap

		Inflow	Peak Outflow (cfs)								Hydrograph
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff			0.471			2.571		6.376	8.660	PRE
2	SCS Runoff			0.344			1.875		4.650	6.316	POST UNDETAINED
3	SCS Runoff			0.285			0.943		2.041	2.678	POST DETAINED
4	Combine	2, 3		0.629			2.782		6.673	8.992	POST AT POI

Proj. file: Gates Road.gpw

Monday, 01 / 23 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.471	2	720	1,625				PRE
2	SCS Runoff	0.344	2	720	1,185				POST UNDETAINED
3	SCS Runoff	0.285	2	720	811				POST DETAINED
3 4	SCS Runoff Combine	0.285	2 2	720 720	811 1,996	2, 3			POST AT POI
Ga	tes Road.gpv	V			Return F	Period: 2 Ye	ear	Monday, 01	/ 23 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

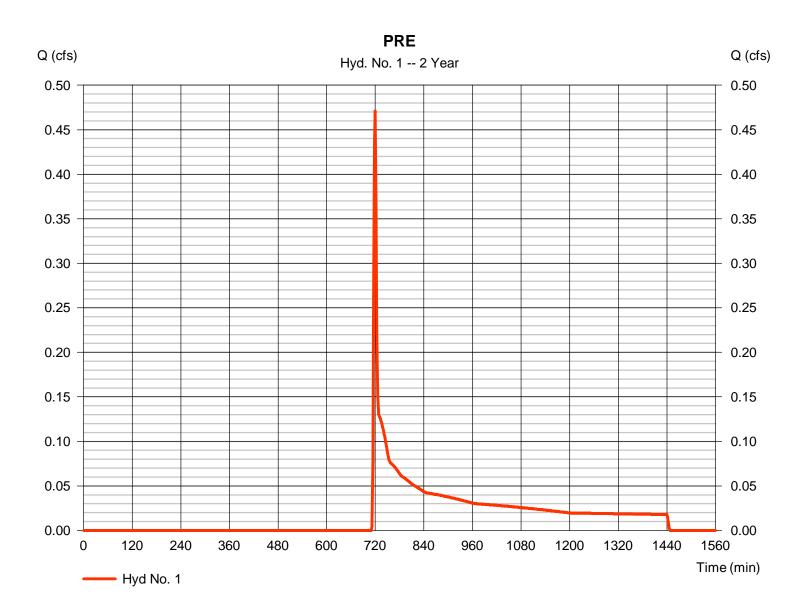
Monday, 01 / 23 / 2017

Hyd. No. 1

PRE

Hydrograph type = SCS Runoff Peak discharge = 0.471 cfsStorm frequency Time to peak = 720 min = 2 yrsTime interval = 2 min Hyd. volume = 1,625 cuftDrainage area Curve number = 1.810 ac= 58*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 2.97 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.810 x 58)] / 1.810



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 2.97 = 8.00	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	d	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

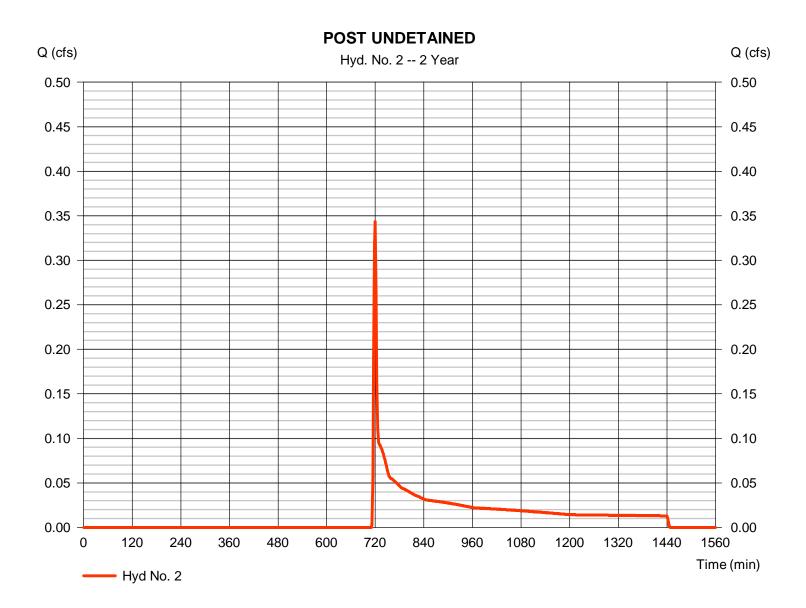
Monday, 01 / 23 / 2017

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 0.344 cfsStorm frequency Time to peak = 2 yrs= 720 min Time interval = 2 min Hyd. volume = 1,185 cuftDrainage area Curve number = 1.320 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 2.97 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.020 x 85) + (1.300 x 58)] / 1.320



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 2POST UNDETAINED

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00	_	4.89
Traver Time (Illiii)	= 4.03	+	0.00	+	0.00	=	4.03
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	d	123.00 4.50 Unpave 3.42	d	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

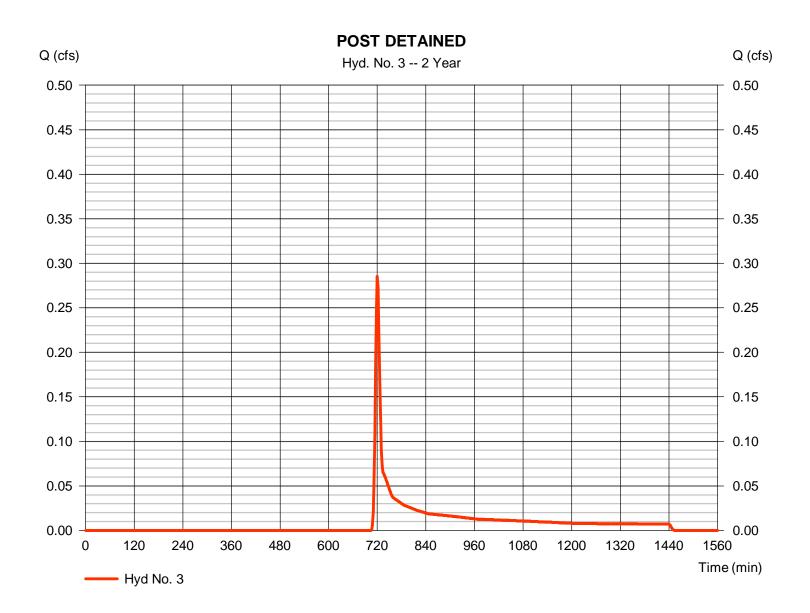
Monday, 01 / 23 / 2017

Hyd. No. 3

POST DETAINED

Hydrograph type = SCS Runoff Peak discharge = 0.285 cfsStorm frequency Time to peak = 2 yrs= 720 min Time interval = 2 min Hyd. volume = 811 cuft Drainage area Curve number = 0.490 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 7.00 \, \text{min}$ Total precip. = 2.97 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.110 x 85) + (0.380 x 58)] / 0.490



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4 $\,$

Hyd. No. 3POST DETAINED

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.240 = 50.0 = 2.97 = 5.00	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	5.90
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 179.00 = 7.30 = Unpaved =4.36	d	55.00 5.50 Paved 4.77		32.00 3.10 Unpave 2.84	d	
Travel Time (min)	= 0.68	+	0.19	+	0.19	=	1.06
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							7.00 min

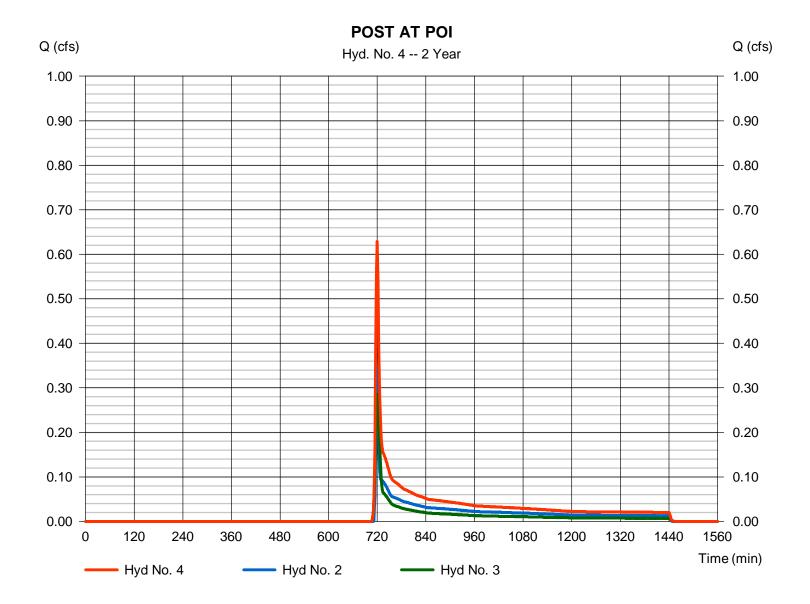
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Hyd. No. 4

POST AT POI

Hydrograph type = Combine Peak discharge = 0.629 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 1,996 cuftInflow hyds. = 2, 3Contrib. drain. area = 1.810 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.571	2	718	5,480				PRE
2	SCS Runoff	1.875	2	718	3,996				POST UNDETAINED
3	SCS Runoff	0.943	2	720	2,219				POST DETAINED
3 4	SCS Runoff Combine	0.943	2 2	720 718	2,219 6,215	2, 3			POST AT POI
Ga	tes Road.gpv	V			Return F	Period: 10 \	⁄ear	Monday, 01	/ 23 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

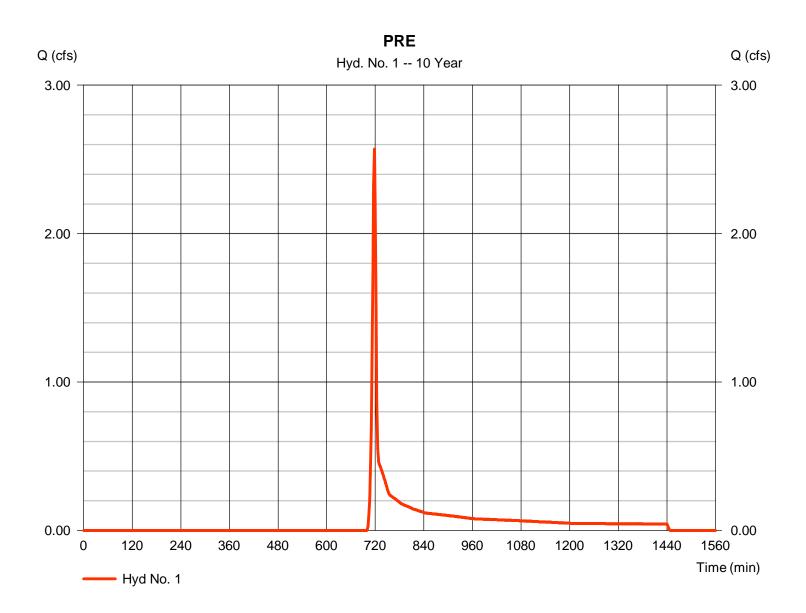
Monday, 01 / 23 / 2017

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 2.571 cfsStorm frequency Time to peak = 718 min = 10 yrsTime interval = 2 min Hyd. volume = 5,480 cuftCurve number Drainage area = 1.810 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 4.47 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.810 x 58)] / 1.810



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

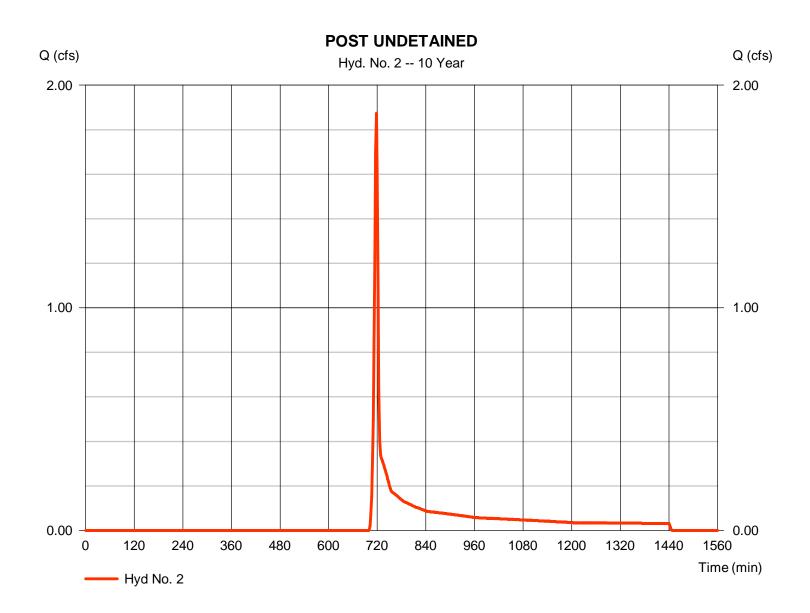
Monday, 01 / 23 / 2017

Hyd. No. 2

POST UNDETAINED

= SCS Runoff Hydrograph type Peak discharge = 1.875 cfsStorm frequency Time to peak = 718 min = 10 yrsTime interval = 2 min Hyd. volume = 3,996 cuftDrainage area Curve number = 1.320 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 4.47 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.020 \times 85) + (1.300 \times 58)] / 1.320$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

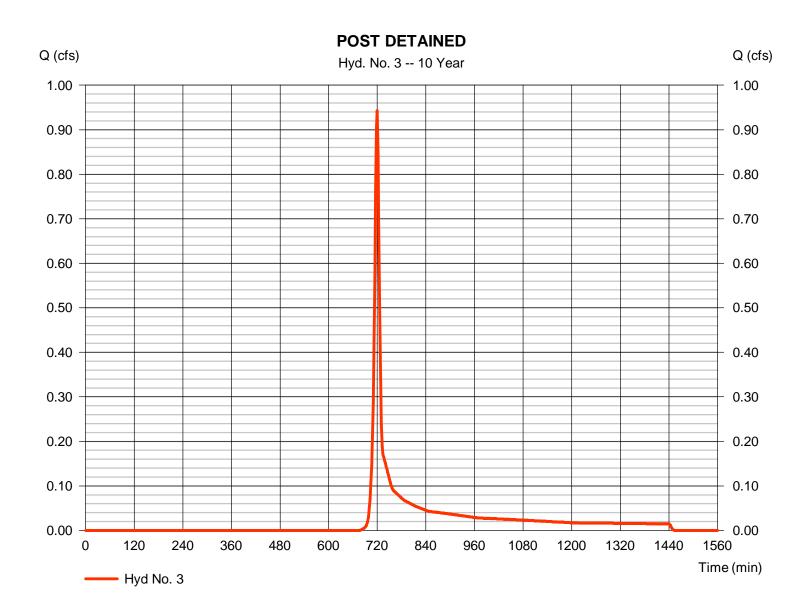
Monday, 01 / 23 / 2017

Hyd. No. 3

POST DETAINED

= SCS Runoff Hydrograph type Peak discharge = 0.943 cfsStorm frequency Time to peak = 10 yrs= 720 min Time interval = 2 min Hyd. volume = 2.219 cuftCurve number Drainage area = 0.490 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 7.00 \, \text{min}$ Total precip. = 4.47 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.110 x 85) + (0.380 x 58)] / 0.490



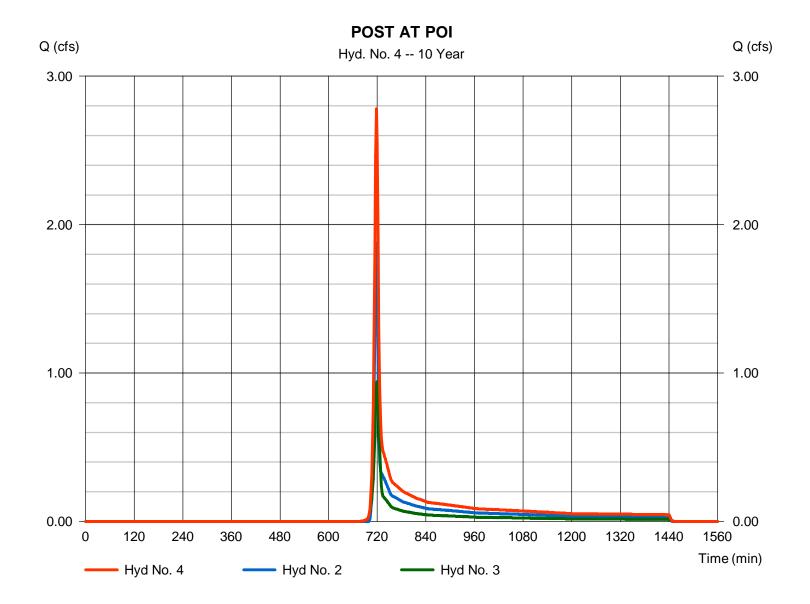
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Hyd. No. 4

POST AT POI

Hydrograph type = Combine = 2.782 cfsPeak discharge Storm frequency = 10 yrsTime to peak = 718 min = 2 min Time interval Hyd. volume = 6,215 cuftInflow hyds. = 2, 3Contrib. drain. area = 1.810 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	6.376	2	718	12,787				PRE
2	SCS Runoff	4.650	2	718	9,325				POST UNDETAINED
3	SCS Runoff	2.041	2	720	4,672				POST DETAINED
	Combine	6.673	2	718	13,997	2, 3			POST AT POI
Gat	tes Road.gpw				Return F	Period: 50 \	⁄ear	Monday, 0°	1 / 23 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

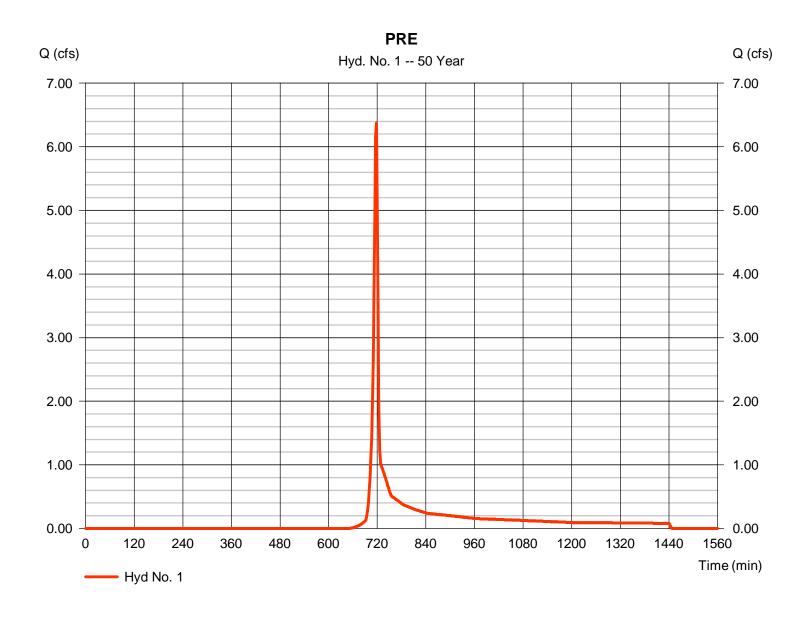
Monday, 01 / 23 / 2017

Hyd. No. 1

PRE

Hydrograph type = SCS Runoff Peak discharge = 6.376 cfsStorm frequency Time to peak = 718 min = 50 yrsTime interval = 2 min Hyd. volume = 12.787 cuftDrainage area Curve number = 1.810 ac= 58*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 6.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.810 x 58)] / 1.810



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

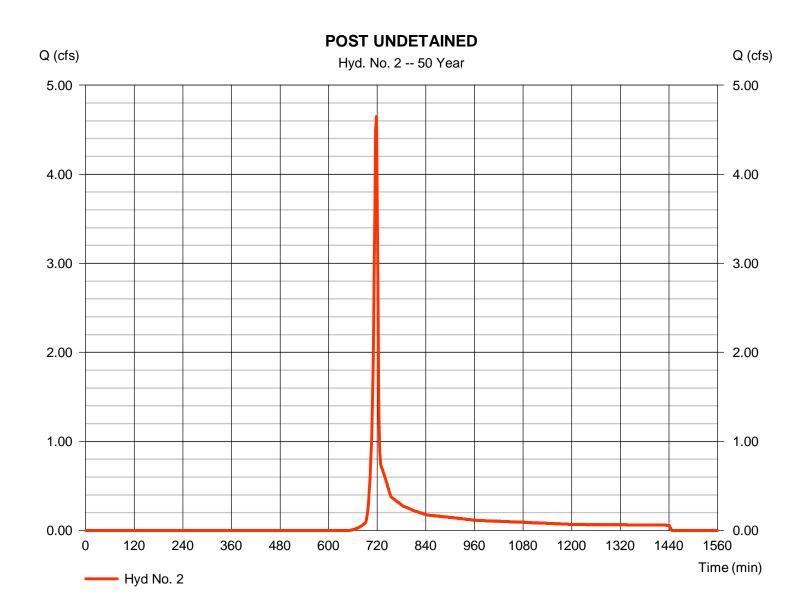
Monday, 01 / 23 / 2017

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 4.650 cfsStorm frequency Time to peak = 718 min = 50 yrsTime interval = 2 min Hyd. volume = 9.325 cuftDrainage area = 1.320 acCurve number = 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 6.50 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.020 x 85) + (1.300 x 58)] / 1.320



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

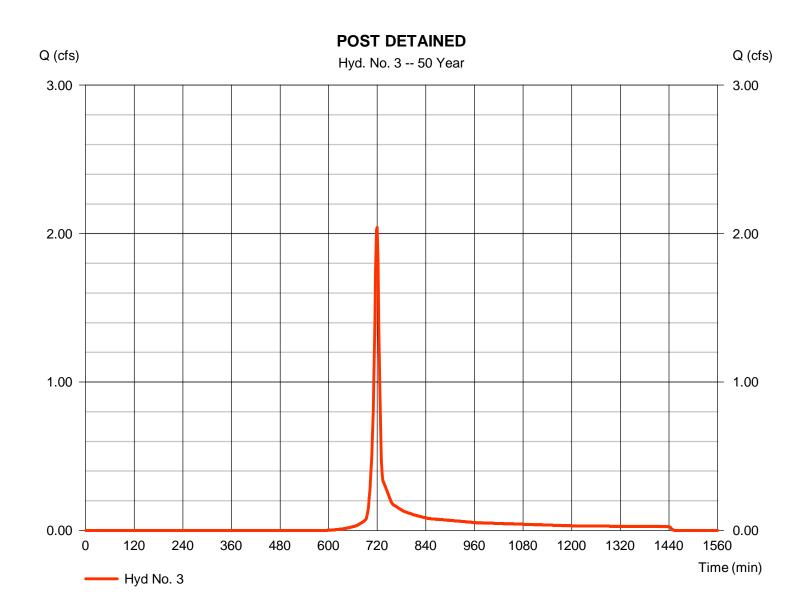
Monday, 01 / 23 / 2017

Hyd. No. 3

POST DETAINED

= SCS Runoff Hydrograph type Peak discharge = 2.041 cfsStorm frequency Time to peak = 720 min = 50 yrsTime interval = 2 min Hyd. volume = 4,672 cuftCurve number Drainage area = 0.490 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 7.00 \, \text{min}$ Total precip. = 6.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.110 \times 85) + (0.380 \times 58)] / 0.490$



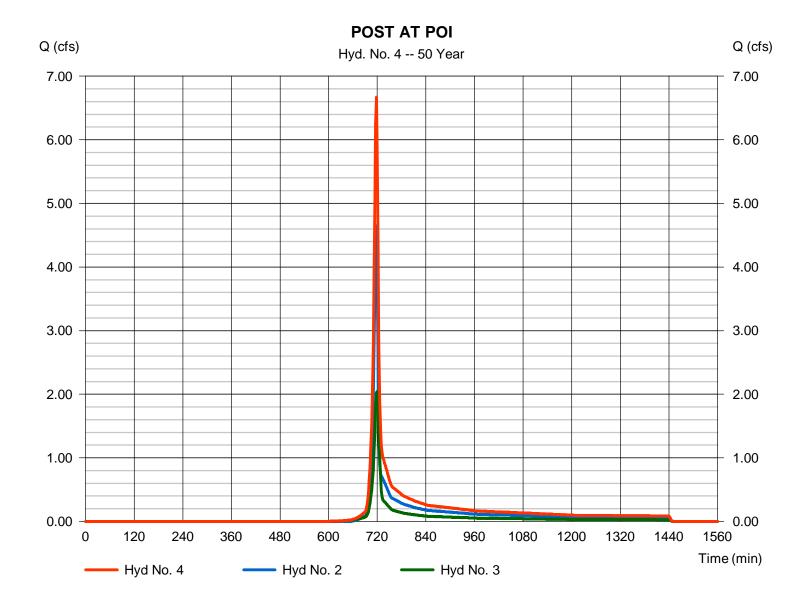
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Hyd. No. 4

POST AT POI

Hydrograph type = Combine Peak discharge = 6.673 cfsStorm frequency = 50 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 13,997 cuftInflow hyds. Contrib. drain. area = 2, 3= 1.810 ac



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	8.660	2	718	17,318				PRE
2	SCS Runoff	6.316	2	718	12,629				POST UNDETAINED
3	SCS Runoff	2.678	2	720	6,135				POST DETAINED
	SCS Runoff Combine	2.678	2 2	720 718	6,135	2, 3			POST AT POI
Gat	es Road.gpw			Return F	Period: 100	Year	Monday, 01	/ 23 / 2017	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

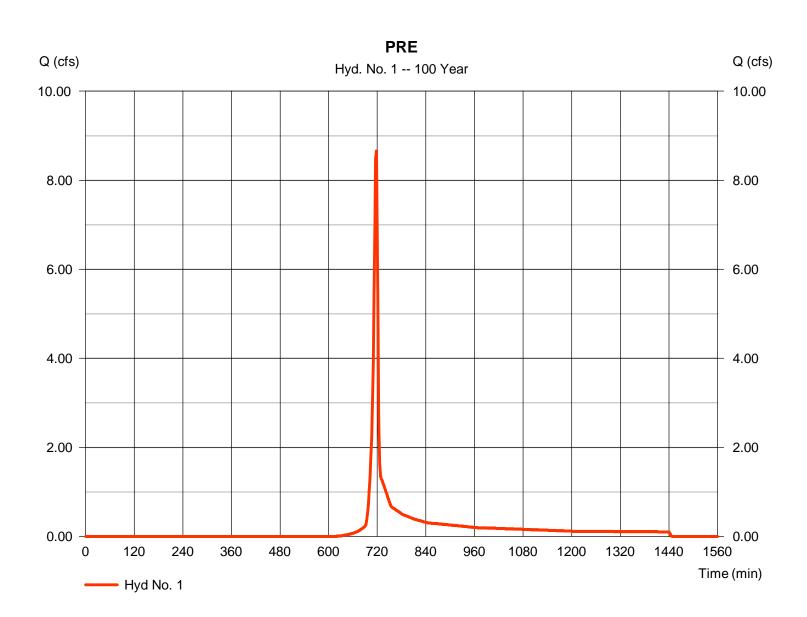
Monday, 01 / 23 / 2017

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 8.660 cfsStorm frequency Time to peak = 718 min = 100 yrsTime interval = 2 min Hyd. volume = 17.318 cuft Drainage area Curve number = 1.810 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 7.58 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(1.810 \times 58)] / 1.810$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

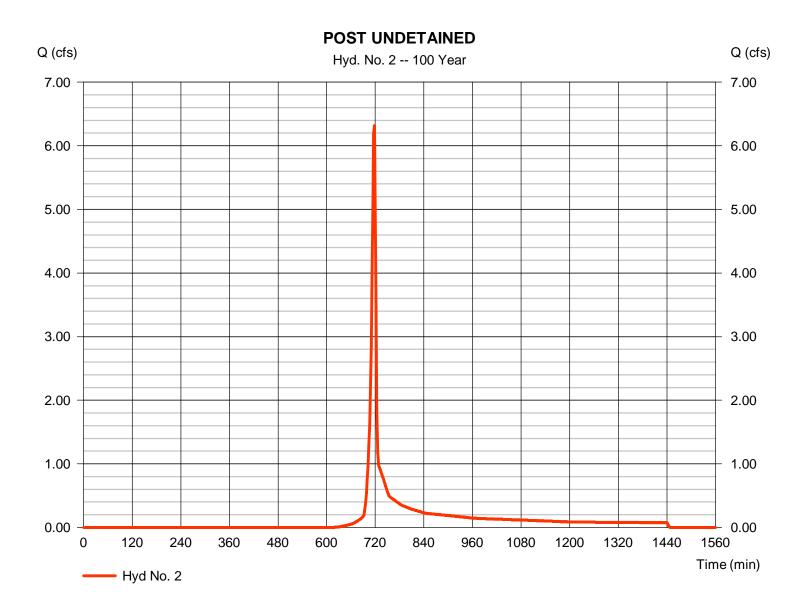
Monday, 01 / 23 / 2017

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 6.316 cfsStorm frequency Time to peak = 718 min = 100 yrsTime interval = 2 min Hyd. volume = 12.629 cuftCurve number Drainage area = 1.320 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 7.58 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.020 x 85) + (1.300 x 58)] / 1.320



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

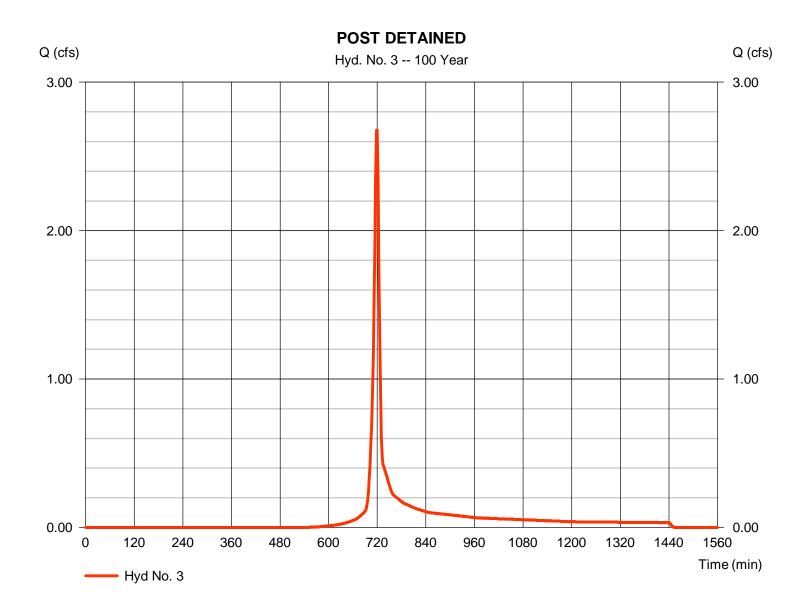
Monday, 01 / 23 / 2017

Hyd. No. 3

POST DETAINED

= SCS Runoff Hydrograph type Peak discharge = 2.678 cfsStorm frequency Time to peak = 720 min = 100 yrsTime interval = 2 min Hyd. volume = 6,135 cuftCurve number Drainage area = 0.490 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 7.00 \, \text{min}$ Total precip. = 7.58 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.110 \times 85) + (0.380 \times 58)] / 0.490$



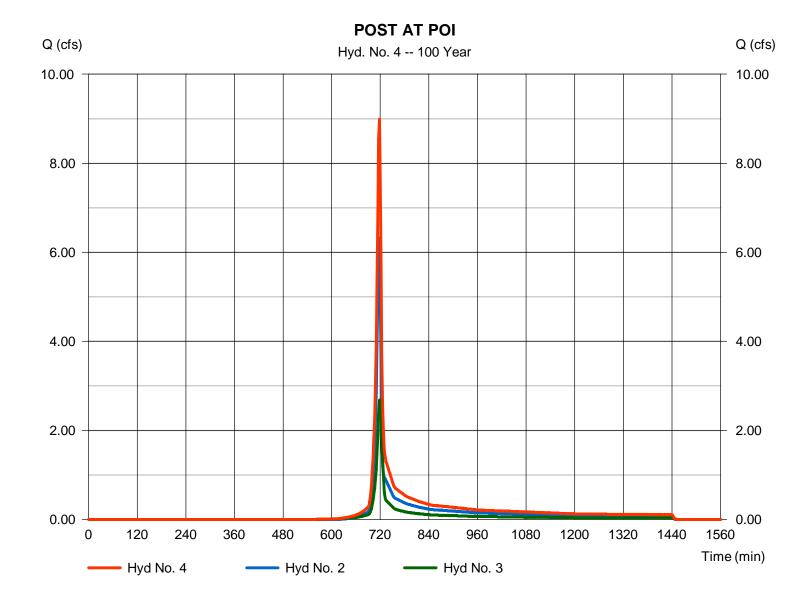
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Hyd. No. 4

POST AT POI

Hydrograph type = Combine Peak discharge = 8.992 cfsStorm frequency = 100 yrsTime to peak = 718 min Time interval = 2 min Hyd. volume = 18,765 cuftInflow hyds. = 2, 3Contrib. drain. area = 1.810 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	47.8383	12.2000	0.8815	
2	54.1927	12.1000	0.8666	
3	0.0000	0.0000	0.0000	
5	62.5798	12.9000	0.8462	
10	55.8141	11.6000	0.7914	
25	52.1635	10.8000	0.7411	
50	47.5325	9.8000	0.6975	
100	42.7369	8.7000	0.6521	

File name: Gates Road IDF.IDF

Intensity = $B/(Tc + D)^E$

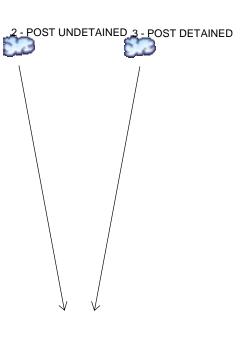
Return		Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	3.90	3.11	2.60	2.24	1.97	1.77	1.60	1.46	1.35	1.25	1.17	1.10			
2	4.63	3.71	3.11	2.68	2.37	2.12	1.92	1.76	1.63	1.51	1.42	1.33			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	5.45	4.42	3.74	3.25	2.89	2.60	2.37	2.18	2.02	1.88	1.76	1.66			
10	6.04	4.91	4.16	3.63	3.23	2.92	2.67	2.46	2.29	2.14	2.01	1.90			
25	6.75	5.50	4.69	4.11	3.68	3.34	3.07	2.84	2.65	2.49	2.34	2.22			
50	7.26	5.92	5.06	4.45	4.00	3.64	3.35	3.11	2.91	2.74	2.59	2.46			
100	7.75	6.33	5.42	4.79	4.31	3.94	3.64	3.39	3.18	3.00	2.85	2.71			

Tc = time in minutes. Values may exceed 60.

\ESCGP-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Gates Road\Hydraflow Rev 1\Gates Road Precip.pc

		R	ainfall P	recipitat	tion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	2.46	2.97	0.00	3.77	4.47	5.55	6.50	7.58
SCS 6-Hr	1.74	2.11	0.00	2.65	3.10	3.76	4.32	4.92
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00







Legend

Hyd.OriginDescription1SCS RunoffPRE2SCS RunoffPOST UNDETAINED3SCS RunoffPOST DETAINED4CombinePOST AT POI

Project: 2-year.gpw

Monday, 01 / 23 / 2017

Hydrograph Return Period Recap

	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff			0.471							PRE
2	SCS Runoff			0.344							POST UNDETAINED
3	SCS Runoff			0.087							POST DETAINED
4	Combine	2, 3		0.358							POST AT POI

Proj. file: 2-year.gpw

Monday, 01 / 23 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.471	2	720	1,625				PRE
2	SCS Runoff	0.344	2	720	1,185				POST UNDETAINED
3	SCS Runoff	0.087	2	752	805				POST DETAINED
4	Combine	0.358	2	720	1,990	2, 3			POST AT POI
2-y	ear.gpw				Return I	Period: 2 Ye	ear	Monday, 0	1 / 23 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

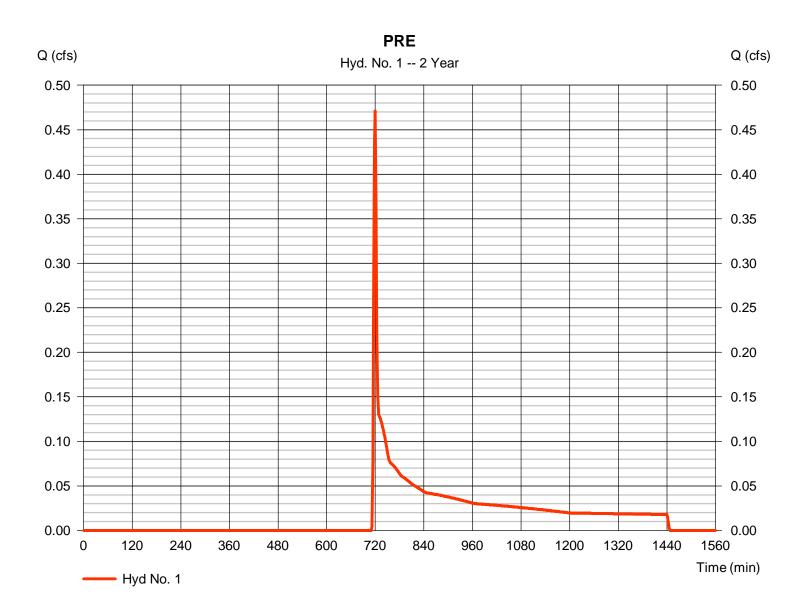
Monday, 01 / 23 / 2017

Hyd. No. 1

PRE

Hydrograph type = SCS Runoff Peak discharge = 0.471 cfsStorm frequency Time to peak = 720 min = 2 yrsTime interval = 2 min Hyd. volume = 1,625 cuftDrainage area Curve number = 1.810 ac= 58*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 2.97 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.810 x 58)] / 1.810



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 4.89	+	0.00	+	0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	ed	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

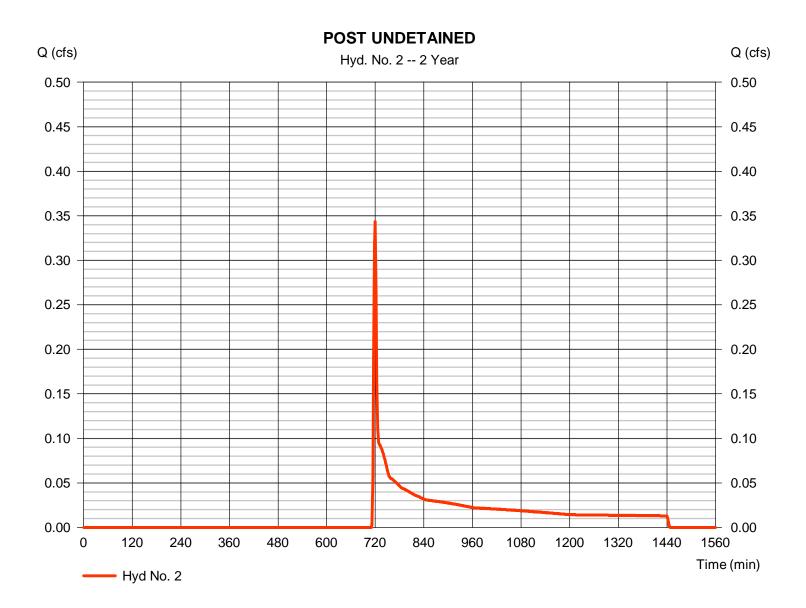
Monday, 01 / 23 / 2017

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 0.344 cfsStorm frequency Time to peak = 2 yrs= 720 min Time interval = 2 min Hyd. volume = 1,185 cuftDrainage area Curve number = 1.320 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 2.97 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.020 x 85) + (1.300 x 58)] / 1.320



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4 $\,$

Hyd. No. 2POST UNDETAINED

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 4.89	+	0.00	+	0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	ed	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc						ı	6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

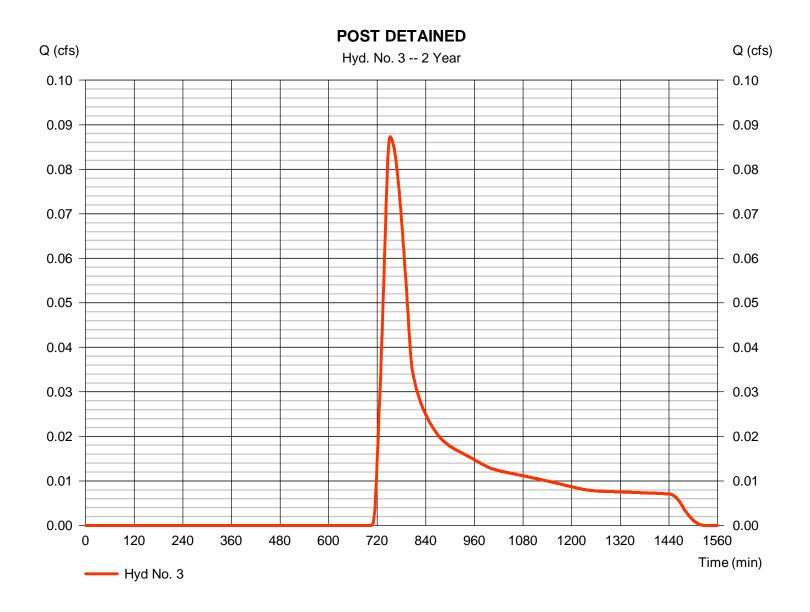
Monday, 01 / 23 / 2017

Hyd. No. 3

POST DETAINED

Hydrograph type = SCS Runoff Peak discharge = 0.087 cfsStorm frequency Time to peak = 752 min = 2 yrsTime interval = 2 min Hyd. volume = 805 cuft Drainage area Curve number = 0.490 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 54.43 \, \text{min}$ Total precip. = 2.97 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.110 \times 85) + (0.380 \times 58)] / 0.490$



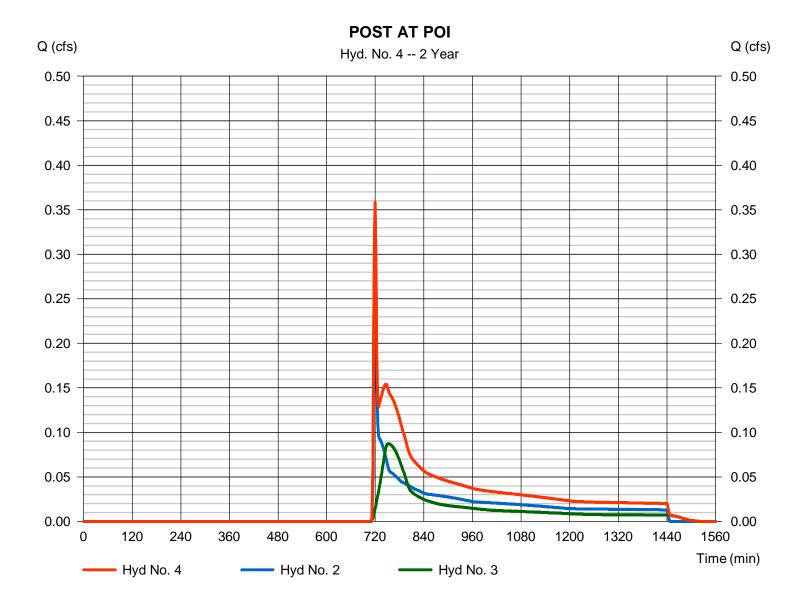
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Hyd. No. 4

POST AT POI

Hydrograph type = Combine Peak discharge = 0.358 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 2 min Hyd. volume = 1,990 cuftInflow hyds. = 2, 3Contrib. drain. area = 1.810 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	47.8383	12.2000	0.8815	
2	54.1927	12.1000	0.8666	
3	0.0000	0.0000	0.0000	
5	62.5798	12.9000	0.8462	
10	55.8141	11.6000	0.7914	
25	52.1635	10.8000	0.7411	
50	47.5325	9.8000	0.6975	
100	42.7369	8.7000	0.6521	

File name: Gates Road IDF.IDF

Intensity = $B/(Tc + D)^E$

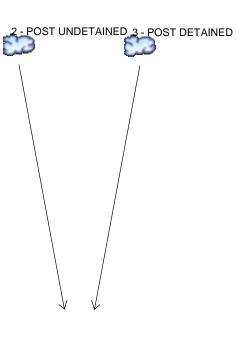
Return		Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	3.90	3.11	2.60	2.24	1.97	1.77	1.60	1.46	1.35	1.25	1.17	1.10			
2	4.63	3.71	3.11	2.68	2.37	2.12	1.92	1.76	1.63	1.51	1.42	1.33			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	5.45	4.42	3.74	3.25	2.89	2.60	2.37	2.18	2.02	1.88	1.76	1.66			
10	6.04	4.91	4.16	3.63	3.23	2.92	2.67	2.46	2.29	2.14	2.01	1.90			
25	6.75	5.50	4.69	4.11	3.68	3.34	3.07	2.84	2.65	2.49	2.34	2.22			
50	7.26	5.92	5.06	4.45	4.00	3.64	3.35	3.11	2.91	2.74	2.59	2.46			
100	7.75	6.33	5.42	4.79	4.31	3.94	3.64	3.39	3.18	3.00	2.85	2.71			

Tc = time in minutes. Values may exceed 60.

\ESCGP-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Gates Road\Hydraflow Rev 1\Gates Road Precip.pc

		R	ainfall P	recipitat	ion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	2.46	2.97	0.00	3.77	4.47	5.55	6.50	7.58
SCS 6-Hr	1.74	2.11	0.00	2.65	3.10	3.76	4.32	4.92
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00





4 - POST AT POI

Legend

Hyd.OriginDescription1SCS RunoffPRE2SCS RunoffPOST UNDETAINED3SCS RunoffPOST DETAINED4CombinePOST AT POI

Project: 10-year.gpw

Monday, 01 / 23 / 2017

Hydrograph Return Period Recap

	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff						2.571				PRE
2	SCS Runoff						1.875				POST UNDETAINED
3	SCS Runoff						0.363				POST DETAINED
4	Combine	2, 3					1.973				POST AT POI

Proj. file: 10-year.gpw Monday, 01 / 23 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description			
1	SCS Runoff	2.571	2	718	5,480				PRE			
2	SCS Runoff	1.875	2	718	3,996				POST UNDETAINED			
3	SCS Runoff	0.363	2	742	2,199				POST DETAINED			
3 4	SCS Runoff Combine	0.363	2 2	742	2,199 6,195	2, 3			POST AT POI			
10-	10-year.gpw					Period: 10 Y	'ear	Monday, 01	Monday, 01 / 23 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

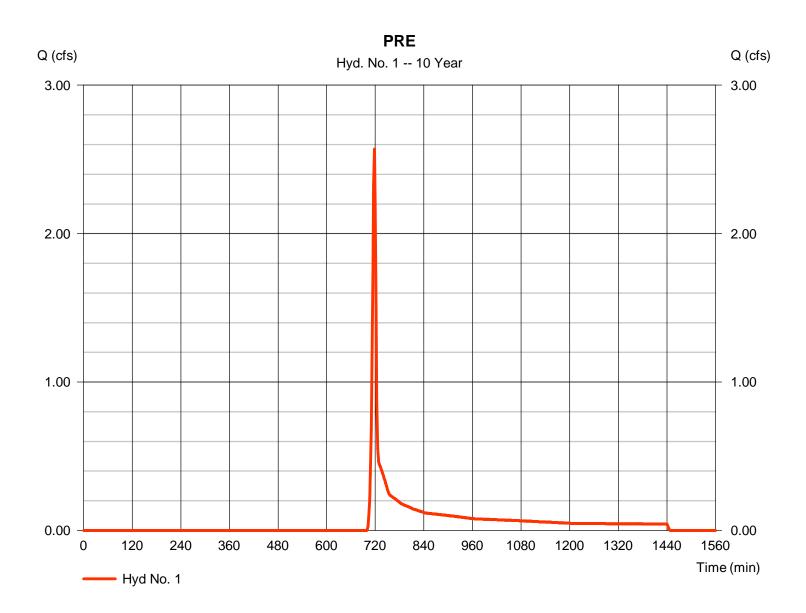
Monday, 01 / 23 / 2017

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 2.571 cfsStorm frequency Time to peak = 718 min = 10 yrsTime interval = 2 min Hyd. volume = 5,480 cuftCurve number Drainage area = 1.810 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 4.47 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.810 x 58)] / 1.810



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 4.89	+	0.00	+	0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	ed	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

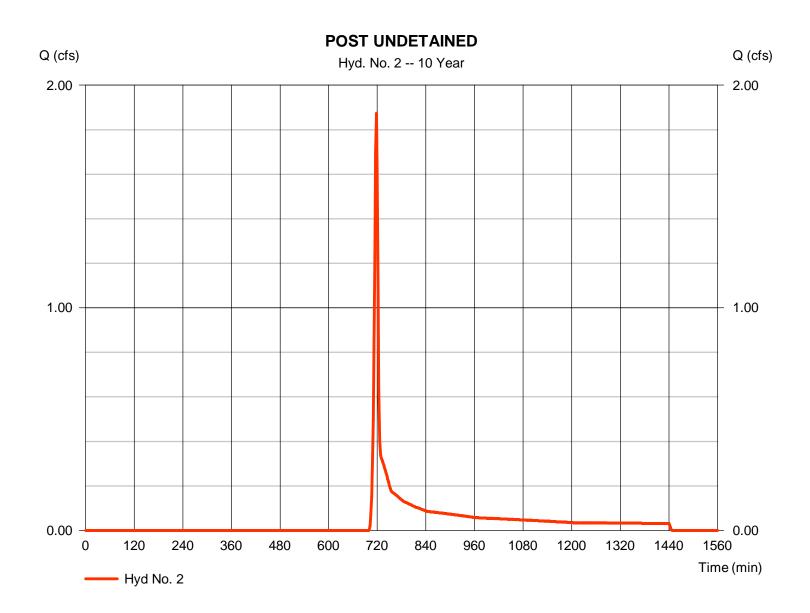
Monday, 01 / 23 / 2017

Hyd. No. 2

POST UNDETAINED

= SCS Runoff Hydrograph type Peak discharge = 1.875 cfsStorm frequency Time to peak = 718 min = 10 yrsTime interval = 2 min Hyd. volume = 3,996 cuftDrainage area Curve number = 1.320 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 4.47 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.020 x 85) + (1.300 x 58)] / 1.320



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4 $\,$

Hyd. No. 2POST UNDETAINED

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 4.89	+	0.00	+	0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	ed	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc						ı	6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

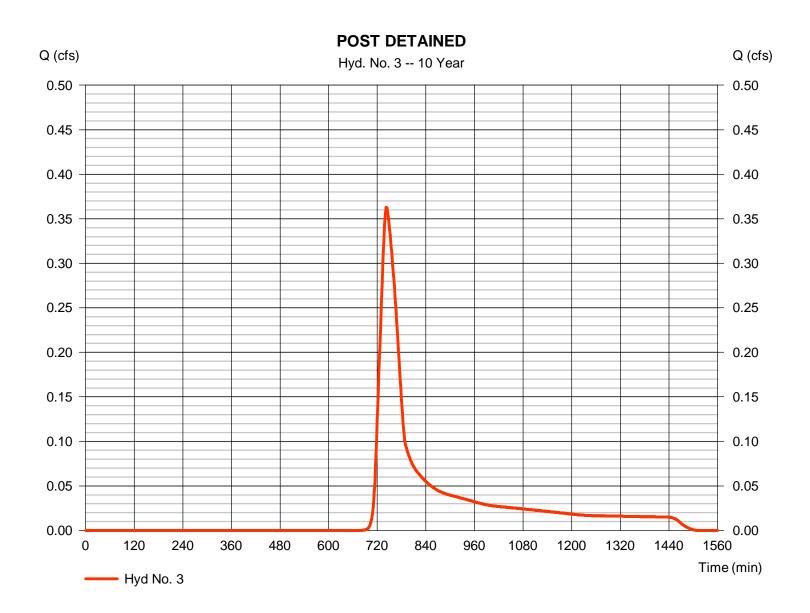
Monday, 01 / 23 / 2017

Hyd. No. 3

POST DETAINED

= SCS Runoff Hydrograph type Peak discharge = 0.363 cfsStorm frequency Time to peak = 742 min = 10 yrsTime interval = 2 min Hyd. volume = 2,199 cuftDrainage area Curve number = 0.490 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) = 46.22 min Total precip. = 4.47 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.110 \times 85) + (0.380 \times 58)] / 0.490$



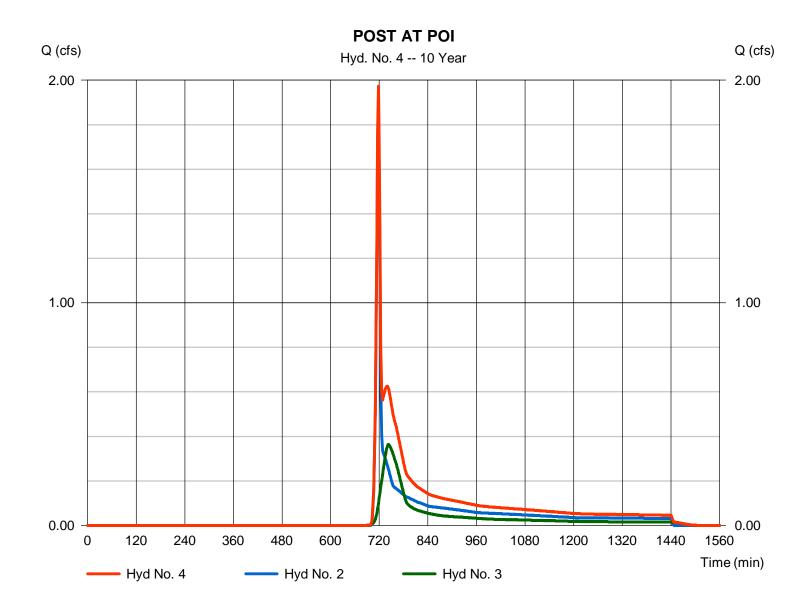
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Hyd. No. 4

POST AT POI

Hydrograph type = Combine Peak discharge = 1.973 cfsStorm frequency = 10 yrsTime to peak = 718 min = 2 min Time interval Hyd. volume = 6,195 cuftInflow hyds. = 2, 3Contrib. drain. area = 1.810 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	47.8383	12.2000	0.8815	
2	54.1927	12.1000	0.8666	
3	0.0000	0.0000	0.0000	
5	62.5798	12.9000	0.8462	
10	55.8141	11.6000	0.7914	
25	52.1635	10.8000	0.7411	
50	47.5325	9.8000	0.6975	
100	42.7369	8.7000	0.6521	

File name: Gates Road IDF.IDF

Intensity = $B/(Tc + D)^E$

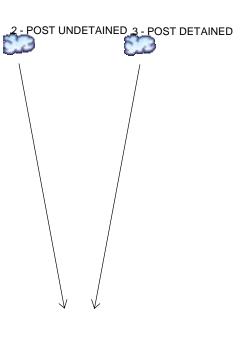
Return Period					Intens	ity Values	(in/hr)					
(Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	3.90	3.11	2.60	2.24	1.97	1.77	1.60	1.46	1.35	1.25	1.17	1.10
2	4.63	3.71	3.11	2.68	2.37	2.12	1.92	1.76	1.63	1.51	1.42	1.33
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5.45	4.42	3.74	3.25	2.89	2.60	2.37	2.18	2.02	1.88	1.76	1.66
10	6.04	4.91	4.16	3.63	3.23	2.92	2.67	2.46	2.29	2.14	2.01	1.90
25	6.75	5.50	4.69	4.11	3.68	3.34	3.07	2.84	2.65	2.49	2.34	2.22
50	7.26	5.92	5.06	4.45	4.00	3.64	3.35	3.11	2.91	2.74	2.59	2.46
100	7.75	6.33	5.42	4.79	4.31	3.94	3.64	3.39	3.18	3.00	2.85	2.71

Tc = time in minutes. Values may exceed 60.

\ESCGP-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Gates Road\Hydraflow Rev 1\Gates Road Precip.pc

		R	ainfall P	recipitat	ion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	2.46	2.97	0.00	3.77	4.47	5.55	6.50	7.58
SCS 6-Hr	1.74	2.11	0.00	2.65	3.10	3.76	4.32	4.92
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00







Legend

Hyd.OriginDescription1SCS RunoffPRE2SCS RunoffPOST UNDETAINED3SCS RunoffPOST DETAINED4CombinePOST AT POI

Project: 50-year.gpw

Monday, 01 / 23 / 2017

Hydrograph Return Period Recap

	Hydrograph	Inflow				Peak Ou	tflow (cfs)				Hydrograph	
) .	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description	
l	SCS Runoff								6.376		PRE	
2	SCS Runoff								4.650		POST UNDETAINED	
3	SCS Runoff								1.221		POST DETAINED	
ļ	Combine	2, 3							5.370		POST AT POI	

Proj. file: 50-year.gpw

Monday, 01 / 23 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	6.376	2	718	12,787				PRE
2	SCS Runoff	4.650	2	718	9,325				POST UNDETAINED
3	SCS Runoff	1.221	2	730	4,599				POST DETAINED
4	Combine	5.370	2	718	13,924	2, 3			POST AT POI
50-year.gpw					Return F	Period: 50 `	 /ear	Monday, 0	1 / 23 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

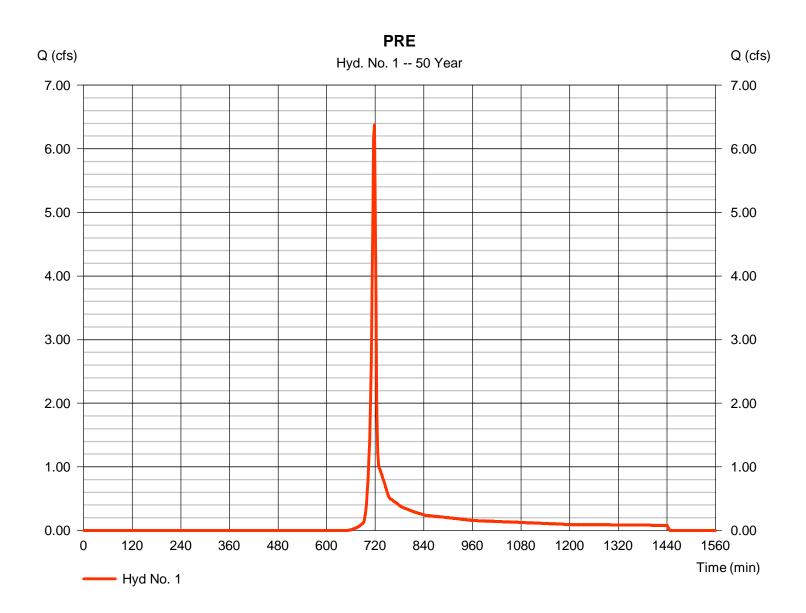
Monday, 01 / 23 / 2017

Hyd. No. 1

PRE

Hydrograph type = SCS Runoff Peak discharge = 6.376 cfsStorm frequency Time to peak = 718 min = 50 yrsTime interval = 2 min Hyd. volume = 12.787 cuftDrainage area Curve number = 1.810 ac= 58*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 6.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.810 x 58)] / 1.810



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 4.89	+	0.00	+	0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	ed	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

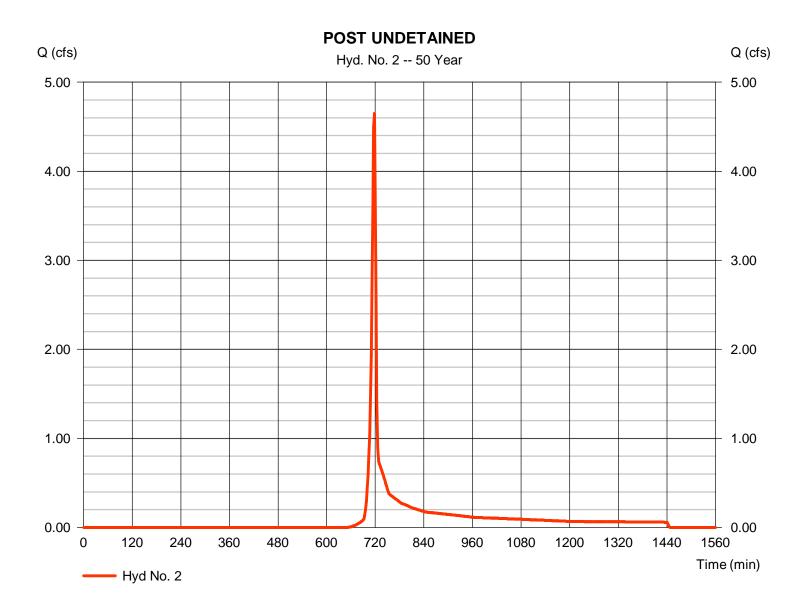
Monday, 01 / 23 / 2017

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 4.650 cfsStorm frequency Time to peak = 718 min = 50 yrsTime interval = 2 min Hyd. volume = 9.325 cuftCurve number Drainage area = 1.320 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 6.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.020 x 85) + (1.300 x 58)] / 1.320



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4 $\,$

Hyd. No. 2POST UNDETAINED

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 4.89	+	0.00	+	0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	ed	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc						ı	6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

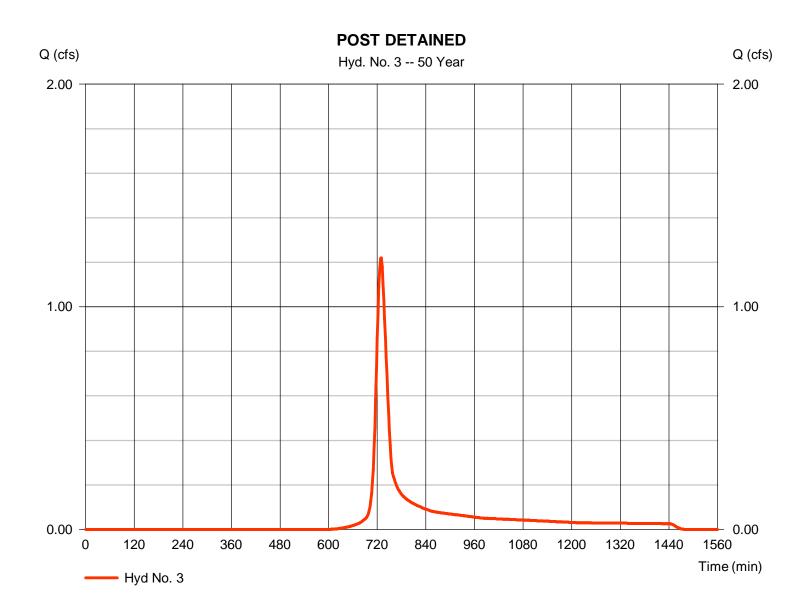
Monday, 01 / 23 / 2017

Hyd. No. 3

POST DETAINED

= SCS Runoff Hydrograph type Peak discharge = 1.221 cfsStorm frequency Time to peak = 730 min = 50 yrsTime interval = 2 min Hyd. volume = 4,599 cuftDrainage area Curve number = 0.490 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 26.13 \, \text{min}$ Total precip. = 6.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.110 \times 85) + (0.380 \times 58)] / 0.490$



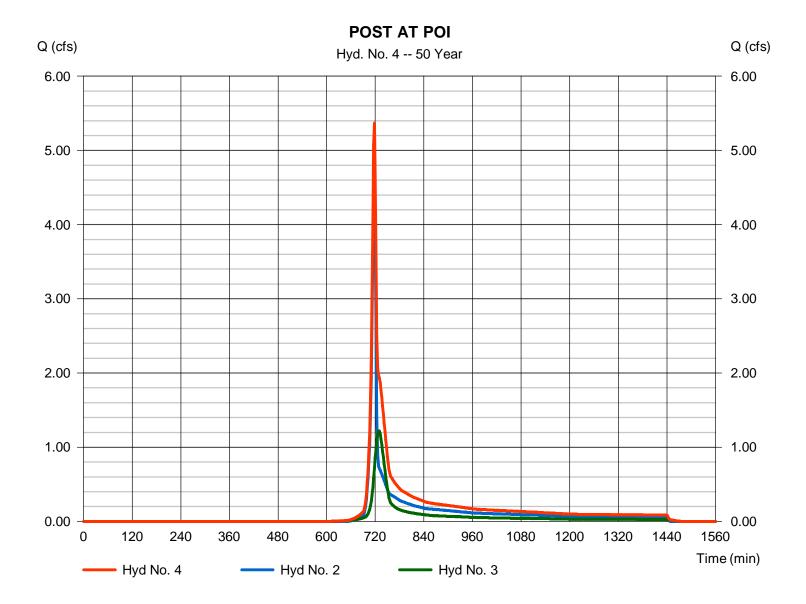
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Hyd. No. 4

POST AT POI

Hydrograph type = Combine Peak discharge = 5.370 cfsStorm frequency = 50 yrsTime to peak = 718 min = 2 min Time interval Hyd. volume = 13,924 cuftInflow hyds. Contrib. drain. area = 2, 3= 1.810 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	47.8383	12.2000	0.8815	
2	54.1927	12.1000	0.8666	
3	0.0000	0.0000	0.0000	
5	62.5798	12.9000	0.8462	
10	55.8141	11.6000	0.7914	
25	52.1635	10.8000	0.7411	
50	47.5325	9.8000	0.6975	
100	42.7369	8.7000	0.6521	

File name: Gates Road IDF.IDF

Intensity = $B/(Tc + D)^E$

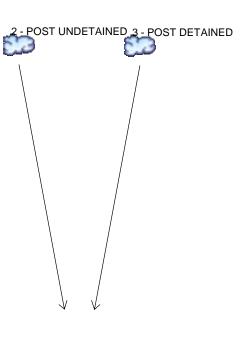
Return Period					Intens	ity Values	(in/hr)					
(Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	3.90	3.11	2.60	2.24	1.97	1.77	1.60	1.46	1.35	1.25	1.17	1.10
2	4.63	3.71	3.11	2.68	2.37	2.12	1.92	1.76	1.63	1.51	1.42	1.33
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5.45	4.42	3.74	3.25	2.89	2.60	2.37	2.18	2.02	1.88	1.76	1.66
10	6.04	4.91	4.16	3.63	3.23	2.92	2.67	2.46	2.29	2.14	2.01	1.90
25	6.75	5.50	4.69	4.11	3.68	3.34	3.07	2.84	2.65	2.49	2.34	2.22
50	7.26	5.92	5.06	4.45	4.00	3.64	3.35	3.11	2.91	2.74	2.59	2.46
100	7.75	6.33	5.42	4.79	4.31	3.94	3.64	3.39	3.18	3.00	2.85	2.71

Tc = time in minutes. Values may exceed 60.

\ESCGP-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Gates Road\Hydraflow Rev 1\Gates Road Precip.pc

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	2.46	2.97	0.00	3.77	4.47	5.55	6.50	7.58		
SCS 6-Hr	1.74	2.11	0.00	2.65	3.10	3.76	4.32	4.92		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		







Legend

Hyd.OriginDescription1SCS RunoffPRE2SCS RunoffPOST UNDETAINED3SCS RunoffPOST DETAINED4CombinePOST AT POI

Project: 100-year.gpw

Monday, 01 / 23 / 2017

Hydrograph Return Period Recap

	Hydrograph	Inflow	Peak Outflow (cfs)							Hydrograph	
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff									8.660	PRE
2	SCS Runoff									6.316	POST UNDETAINED
3	SCS Runoff									1.785	POST DETAINED
4	Combine	2, 3								7.514	POST AT POI

Proj. file: 100-year.gpw Monday, 01 / 23 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

					1	apris Extension for AutoCAD® Civil 3D® 2013 by AutoGesk, inc. v10.					
łyd. ło.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	8.660	2	718	17,318				PRE		
2	SCS Runoff	6.316	2	718	12,629				POST UNDETAINED		
3	SCS Runoff	1.785	2	726	6,245				POST DETAINED		
4	Combine	7.514	2	718	18,874	2, 3			POST AT POI		
100	year.gpw				Return F	Period: 100	Year	Monday, 01	/ 23 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

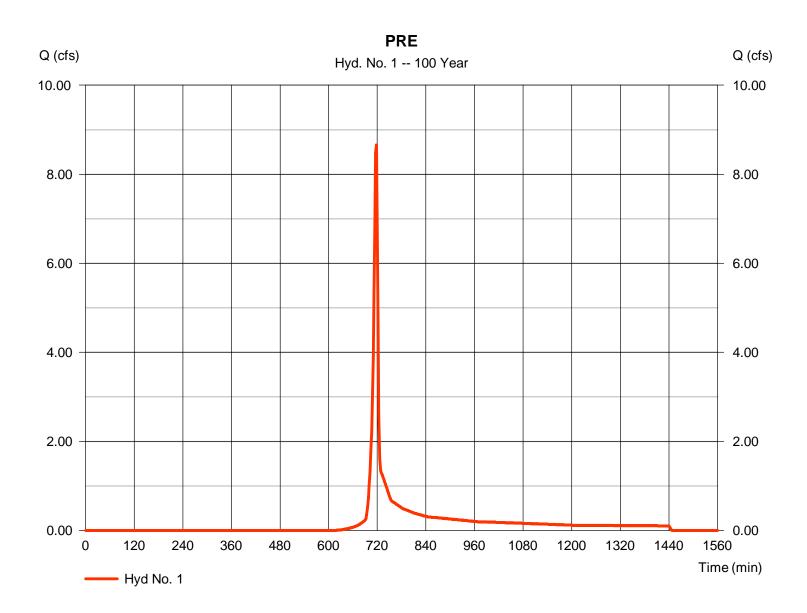
Monday, 01 / 23 / 2017

Hyd. No. 1

PRE

= SCS Runoff Hydrograph type Peak discharge = 8.660 cfsStorm frequency Time to peak = 718 min = 100 yrsTime interval = 2 min Hyd. volume = 17,318 cuftDrainage area Curve number = 1.810 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 7.58 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.810 x 58)] / 1.810



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Hyd. No. 1

PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 4.89	+	0.00	+	0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	ed	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

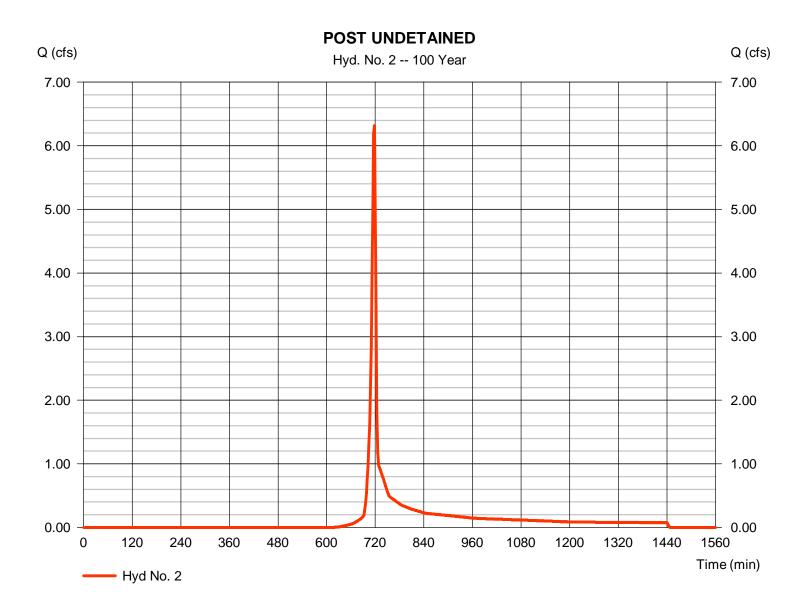
Monday, 01 / 23 / 2017

Hyd. No. 2

POST UNDETAINED

Hydrograph type = SCS Runoff Peak discharge = 6.316 cfsStorm frequency Time to peak = 718 min = 100 yrsTime interval = 2 min Hyd. volume = 12.629 cuftDrainage area Curve number = 1.320 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 6.30 \, \text{min}$ Total precip. = 7.58 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.020 x 85) + (1.300 x 58)] / 1.320



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4 $\,$

Hyd. No. 2POST UNDETAINED

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.240 = 50.0 = 2.97 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 4.89	+	0.00	+	0.00	=	4.89
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 173.00 = 9.10 = Unpave =4.87	d	22.00 1.60 Unpave 2.04	ed	123.00 4.50 Unpave 3.42	ed	
Travel Time (min)	= 0.59	+	0.18	+	0.60	=	1.37
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc						ı	6.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

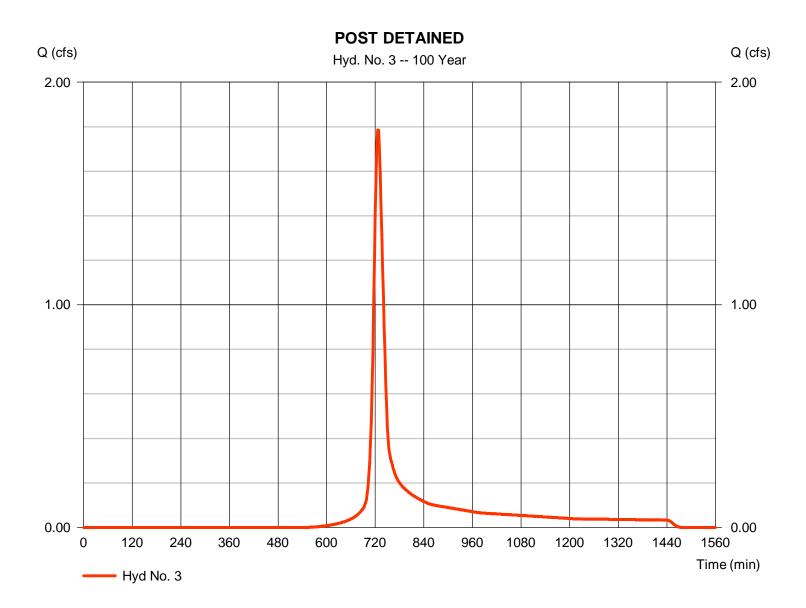
Monday, 01 / 23 / 2017

Hyd. No. 3

POST DETAINED

= SCS Runoff = 1.785 cfsHydrograph type Peak discharge = 726 min Storm frequency Time to peak = 100 yrsTime interval = 2 min Hyd. volume = 6,245 cuftDrainage area Curve number = 0.490 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) = 21.58 min Total precip. = 7.58 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.110 x 85) + (0.380 x 58)] / 0.490



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

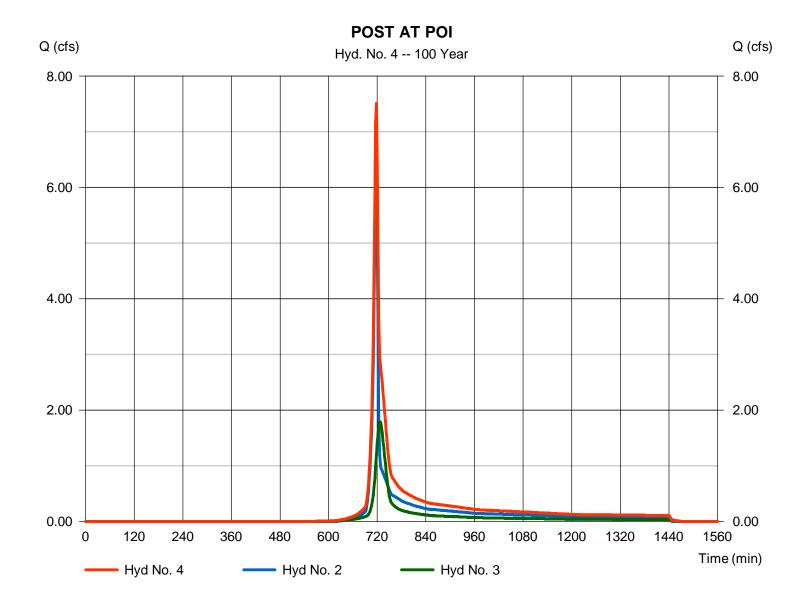
Monday, 01 / 23 / 2017

Hyd. No. 4

POST AT POI

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 2 min
Inflow hyds. = 2, 3

Peak discharge = 7.514 cfs
Time to peak = 718 min
Hyd. volume = 18,874 cuft
Contrib. drain. area = 1.810 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2015 by Autodesk, Inc. v10.4

Monday, 01 / 23 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)								
1	47.8383	12.2000	0.8815									
2	54.1927	12.1000	0.8666									
3	0.0000	0.0000	0.0000									
5	62.5798	12.9000	0.8462									
10	55.8141	11.6000	0.7914									
25	52.1635	10.8000	0.7411									
50	47.5325	9.8000	0.6975									
100	42.7369	8.7000	0.6521									

File name: Gates Road IDF.IDF

Intensity = $B/(Tc + D)^E$

Return	Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60		
1	3.90	3.11	2.60	2.24	1.97	1.77	1.60	1.46	1.35	1.25	1.17	1.10		
2	4.63	3.71	3.11	2.68	2.37	2.12	1.92	1.76	1.63	1.51	1.42	1.33		
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
5	5.45	4.42	3.74	3.25	2.89	2.60	2.37	2.18	2.02	1.88	1.76	1.66		
10	6.04	4.91	4.16	3.63	3.23	2.92	2.67	2.46	2.29	2.14	2.01	1.90		
25	6.75	5.50	4.69	4.11	3.68	3.34	3.07	2.84	2.65	2.49	2.34	2.22		
50	7.26	5.92	5.06	4.45	4.00	3.64	3.35	3.11	2.91	2.74	2.59	2.46		
100	7.75	6.33	5.42	4.79	4.31	3.94	3.64	3.39	3.18	3.00	2.85	2.71		

Tc = time in minutes. Values may exceed 60.

\ESCGP-2\PPP\02 SCRO\07 PCSM\Attach 4 Stormwater Calcs\Gates Road\Hydraflow Rev 1\Gates Road Precip.pc

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	2.46	2.97	0.00	3.77	4.47	5.55	6.50	7.58		
SCS 6-Hr	1.74	2.11	0.00	2.65	3.10	3.76	4.32	4.92		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		